PAD FOOTING SCHEDULE

MARK	SIZE	REINF.
Ⓐ	3'–0" SQ. X 2'–0" DP. MIN.	(4) #4 EA. WAY BOTT.
Ⓑ	4'–8" SQ. X 2'–0" DP. MIN.	(5) #5 EA. WAY BOTT.
Ⓓ	4'–0" SQ. X 2'–0" DP. MIN.	(5) #4 EA. WAY BOTT.

CONT. FOOTING SCHEDULE

MARK	SIZE	REINF.
CF1	2'–6" WIDE X 2'–0" DP. MIN.	(3) #5 CONT. TOP & BOTT.
CF2	3'–0" WIDE X 2'–0" DP. MIN.	(3) #5 CONT. TOP & BOTT.
CF3	2'–6" WIDE X 1'–0" DP. MIN.	(3) #5 CONT. BOTT.
CF4	3'–0" WIDE X 1'–0" DP. MIN.	(3) #5 CONT. BOTT.

WOOD STAIR SCHEDULE

MARK	SIZE
W1	2X10 JST'S @ 16" O.C. (MAX.)
W2	(3) 2X12 STRINGERS (PER STAIR)
W3	(2) 1¾ X 11⅞ 1.55E LSL

MASONRY LINTEL SCHEDULE

MARK	SIZE W (WIDTH)	SIZE D (DEPTH)	REINFORCING BOTT. BARS	REINFORCING TOP BARS	REINFORCING TIES	MIN. BRG. EA. END	DETAIL
ML1	8"	8"	(2) #5	——	——	8"	1/S3.3
ML2	8	16"	(2) #5	(1) #5	#3 @ 16" O.C.	8"	2/S3.3
ML3	8"	24"	(2) #5	(2) #5	#3 @ 16" O.C.	8"	3/S3.3
ML4	8"	24"	(2) #5	(2) #5	#3 @ 8" O.C.	8"	3/S3.3

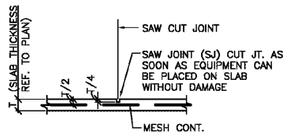
NOTE: ALL MASONRY DIMENSIONS ARE NOMINAL MEASUREMENTS.
EXAMPLE: 8" CMU = 7 5/8" WIDE.

HEADER/BEAM/LINTEL SCHEDULE

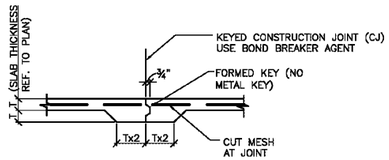
MARK	SIZE	BRG. STUDS UNDER EA. END	ALTERNATE
L1	(2) 2X8's W/ ½" PLYWOOD SPACERS	(2) 2X4	——
L2	(2) 2X12's W/ ½" PLYWOOD SPACERS	(2) 2X4	3½ X 7¼ 1.3E LSL
L3	(2) 1¾ X 9¼ 1.55E LSL	(3) 2X4	3½ X 9¼ 1.55E LSL
L4	(2) 1¾ X 11⅞ 1.55E LSL	(3) 2X4	3½ X 11⅞ 1.55E LSL
L5	(3) 1¾ X 11⅞ 1.55E LSL	4X6 POST W/ (1) 2X4 GLUED & NAILED	5¼ X 11⅞ 2.0E PSL

MASONRY WALL SCHEDULE

MARK	SIZE	VERTICAL REINFORCING	HORIZONTAL BOND BEAM SPACING	SECTIONS
MW1	8" CMU	#5 @ 32" O.C. (MAX.) w/ MATCHING DWL's INTO WALL.	•10'–0" O.C. (MAX.) •(2) 8" DP. BOND BM's @ EA. FLOOR •(1) 8" DP. BOND BM. @ ROOF	A/S3.1, B/S3.1 & A/S4.1
MW2	8" CMU	#5 @ 24" O.C. (MAX.) w/ MATCHING DWL's INTO FNDN.	•10'–0" O.C. (MAX.) •(2) 8" DP. BOND BM's @ EA. FLOOR •(1) 8" DP. BOND BM. @ ROOF	A/S3.1, B/S3.1 & A/S4.1
MW3	8" CMU	#5 @ 24" O.C. (MAX.) w/ MATCHING DWL's INTO FNDN.	•10'–0" O.C. (MAX.) •(1) 8" DP. BOND BM's @ EA. FLOOR •(1) 8" DP. BOND BM. @ ROOF	G/S3.1 & F/S4.1



1 S–0.1 TYP. SAWCUT JT.
SCALE: 1/2" = 1'–0"



2 S–0.1 TYP. KEYED CONST. JT.
SCALE: 1/2" = 1'–0"
NOTE: MAY BE USED IN LIEU OF SAWCUT JT.

"SIMPSON" CONNECTORS

PRODUCT	DESCRIPTION	FLORIDA NO.
A34	FRAMING ANCHOR	FL10446.1
ACE66	POST CAP	FL10860.5
BA 3.56/11.88	HANGER	FL10667.4
H2.5A	HURRICANE TIE	FL10456.3
H2.5T	HURRICANE TIE	FL10446.8
H3	HURRICANE TIE	FL10456.3
H8	HURRICANE TIE	FL10446.8
HB712/11.25	HANGER	FL10667.4
HDU8–SDS2.5	HOLDOWN	FL10447.4
HGLBB	BEAM SEAT	FL10866.6
HM9KT	HURRICANE TIE	*****
HTSM16	TWIST STRAP	FL11473.12
HU210–2F	HANGER	FL10856.1
HU210–3TF	HANGER	FL10856.1
HWU5.50/11.88	HOLDOWN	FL10667.54
LSSU28	HANGER	FL10447.5
LSSU210	HANGER	FL10447.5
LSTA24	STRAP TIE	FL10852.6
LU24	HANGER	FL10655.28
LU28	HANGER	FL10655.28
[illegible]	HANGER	FL10655.31
[illegible]S28	HANGER	FL10655.31
LTP4	FRAMING ANGLE	FL10446.13
MIT416	HANGER	FL10667.76
MIT3516	HANGER	FL10667.75
MTS12	TWIST STRAP	FL10456.8
MSTA24	STRAP TIES	FL10852.9
MSTAM24	STRAP TIES	FL11473.11
SP6	STUD PLATE TIE	FL10456.12
STC	ROOF TRUSS CLIP	*****
OH33	ORNAMENTAL ANGLE	*****

ALTERNATE "USP CONNECTORS"

PRODUCT	FLORIDA NO.
[illegible]34	FL17244.23
[illegible]E66	FL17239.15
BPH [illegible]	FL17241.2
RT7A	FL17236.11
RT7AT	FL17236.11
RT3A	FL17236.11
RT8A	FL17236.11
HBPW	******
PHD8	FL17219.4
KHGLBB	FL17241.11
RT16M	FL17236.10
HTWM16	FL6223.2
HD210–2TF	FL17232.3
HD210–3TF	FL17232.3
PHXUS5118	FL17241.20
LSSH15–TZ	******
LSSH210	FL17243.30
LSTA24	FL17244.18
JUS24	FL17232.11
JL28	FL17232.9
JUS26	FL17232.12
JUS28	FL17232.12
MP4F	FL17244.24
TFI416	FL17241.23
TFI3516	FL17241.23
MTW12	FL17244.8
MSTA24	FL17244.18
MSTAM24	FL17325.1
SPT6	FL17244.32
TR1	******
KHL33–0	******

STRUCTURAL GENERAL NOTES:

1. DESIGN AND CONSTRUCTION SHALL CONFORM TO THE "FLORIDA BUILDING CODE, 2017 EDITION". REFER TO THE THRESHOLD INSPECTION NOTES ON SHEET S0.2 FOR ADDITIONAL REQUIREMENTS.
2. DESIGN CRITERIA AND LOADS: PITCHED ROOF LIVE LOAD – 20 PSF: FLAT ROOF LIVE LOAD = 30 PSF: SECOND, THIRD AND FOURTH FLOOR PRIVATE ROOMS LIVE LOAD = 40 PSF: UTILITY ROOM LIVE LOAD = 50 PSF: STAIR AND CORRIDOR LIVE LOAD = 100 PSF: SLAB ON GRADE LIVE LOAD = 150 PSF: ULTIMATE WIND SPEED 140 MPH (3 SECOND GUST), EXPOSURE C, WIND/SEISMIC RISK CATEGORY II: : SEISMIC DESIGN CATEGORY–A: SEISMIC SITE CLASS–D.
3. THE NET ALLOWABLE TOTAL LOAD SOIL BEARING PRESSURE DOES NOT EXCEED 2500 PSF FOR ALL WALL AND COLUMN FOOTINGS RESTING ON THE LIMESTONE OR ON COMPACTED SOIL, REFER TO GEOTECHNICAL REPORT BY MORTENSEN ENGINEERING, INC. PROJECT NO. 18–10–08737, DATED JULY 17, 2018. ALL FOOTINGS SHALL EXTEND AT LEAST 2'–2" BELOW FINAL ADJACENT GRADE. FLOOR SLAB SUBGRADE PREPARATION SHALL BE PREPARED AS OUTLINED IN THE GEOTECHNICAL REPORT. IF ACTUAL SITE CONDITIONS DO NOT SATISFY THESE REQUIREMENTS COORDINATE ADJUSTMENTS WITH THE ARCHITECT/ENGINEER/SOILS ENGINEER/OWNERS REPRESENTATIVE.
4. FOOTINGS MAY BE POURED TO NEAT LINES OF EXCAVATIONS PROVIDING VERTICAL LINES OF EXCAVATIONS CAN BE MAINTAINED DURING CONCRETE PLACEMENT.
5. BEAMS, COLUMNS, WALLS AND FOOTINGS CENTER SHALL BE CENTERED UNDER SUPPORTING MEMBERS (TYPICAL UNLESS NOTED).
6. CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" (VERIFY WITH ARCHITECT).
7. NO ALUMINUM SHALL BE IMBEDDED IN ANY CONCRETE.
8. ALL STRUCTURAL REGULAR WEIGHT CAST–IN–PLACE CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS (TYPICAL UNLESS NOTED): ALL SLABS ON GRADE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3500 PSI AT 28 DAYS. ALL CONCRETE SHALL BE IN CONFORMANCE WITH THE LATEST A.C.I. 301 STANDARDS PUBLICATION.
9. ALL REINFORCING BARS SHALL MEET ASTM A 615 GRADE 60.
10. REINFORCING BARS QUANTITIES SHOWN ARE FOR ESTIMATING PURPOSES ONLY.
11. PROVIDE 500 LBS. OF EXTRA BARS OF VARIOUS SIZES USED ON THIS PROJECT TO BE USED AS DIRECTED: INCLUDE LABOR FOR PLACEMENT.
12. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE 1 1/4" CLEAR FOR SLABS, 2" CLEAR FOR FORMED SURFACES AND 3" CLEAR FOR FOOTINGS (TYPICAL UNLESS NOTED).
13. REINFORCEMENT SHALL BE CONTINUOUS AND LAPPED 40 BAR DIAMETERS (2' – 0" MIN.) EXCEPT AS NOTED AND PROVIDE CORNER BARS OF SAME SIZE AND SPACING.
14. REINFORCEMENT SHALL BE DETAILED IN ACCORDANCE WITH THE LATEST A.C.I. DETAILING MANUAL BY QUALIFIED AND EXPERIENCED PERSON AND FIRM. PLACE AND SUPPORT REINFORCING WITH ACCESSORIES: MAXIMUM SPACING – 48" CENTERS. USE 3" SBC SUPPORTS AT ALL FOOTINGS.
15. CONTRACTOR SHALL VERIFY THAT ALL REINFORCEMENTS, INSERTS, SLEEVES AND EMBEDDED ITEMS ARE PROPERLY LOCATED AND RIGIDLY SECURED PRIOR TO CONCRETE PLACEMENT.
16. ALL ANCHORS WHERE NOTED SHALL BE MANUFACTURED BY HILTI, INC. AND INSTALLED PER HILTI SPECIFICATIONS. SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL WITH APPROPRIATE ICC EVALUATION REPORTS.
17. ALL MESH SHALL MEET ASTM A1064: LAP A MINIMUM OF 8" OR ONE FULL MESH, WHICHEVER IS GREATER.
18. SLABS ON GRADE SHALL BE 4" MINIMUM THICK WITH 6X6–W1.4 X W1.4 W.W.F AT CENTER OF SLAB THICKNESS, UNLESS NOTED.
19. HOLLOW LOAD BEARING CONCRETE MASONRY UNITS SHALL MEET ASTM C90 GRADE N SPECIFICATIONS (F'm= 2000 PSI) UNLESS NOTED: MORTAR TYPE– M OR S. REFER TO THE REINFORCED CONCRETE MASONRY NOTES.
20. PROVIDE STANDARD LADDER TYPE HORIZONTAL MASONRY REINFORCEMENT AT EVERY OTHER COURSE AT ALL MASONRY WALLS.
21. STRUCTURAL STEEL BEAMS SHALL MEET ASTM A992 (Fy = 50 KSI MIN.), STRUCTURAL STEEL PLATES, ANGLES AND CHANNELS SHALL MEET ASTM A36 (Fy = 36 KSI MIN.), STRUCTURAL TUBING (HSS) – ASTM A500 GRADE B (Fy = 46 KSI), AND ANCHOR BOLTS – ASTM F–1554, GRADE 36, TYPICAL UNLESS NOTED.
22. STRUCTURAL STEEL SHALL BE NEW AND MEET THE FOURTEENTH EDITION A.I.S.C. "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS AND BRIDGES" AND THE "CODE OF STANDARD PRACTICES FOR STEEL BUILDINGS AND BRIDGES", EXCLUDING SECTION 4.4.1.b
23. THE STRUCTURAL STEEL FABRICATOR SHALL BE AN A.I.S.C. QUALITY CERTIFIED COMPANY FOR THE CATEGORY OF WORK IN THIS PROJECT OR PROVIDE A QUALITY ASSURANCE PLAN AND SPECIAL INSPECTIONS AS OUTLINED IN THE CODE.
24. ANY SHOP AND FIELD SPLICES WITHIN THE STEEL MEMBERS DESIGN LENGTH MUST BE MADE WITH A FULL PENETRATION WELD AND PROVED ADEQUATE BY NON–DESTRUCTIVE TESTING AT THE CONTRACTOR'S EXPENSE.
25. WELDING SHALL CONFORM TO CURRENT A.W.S. "STRUCTURAL WELDING CODE – STEEL" AND "STRUCTURAL WELDING CODE – SHEET STEEL" SPECIFICATIONS AND BE COMPLETED BY AN A.W.S. CERTIFIED WELDER.
26. ALL WALL STUDS AND LOAD BEARING EXPOSED TOP PLATES AND BOTTOM PLATES SHALL BE No. 1/No. 2 OR BETTER S–P–F (SPRUCE–PINE–FIR WITH MINIMUM Fb = 875 PSI (SINGLE USE) AND MINIMUM E = 1,400,000 PSI (TYPICAL UNLESS NOTED OR REVIEWED EQUIVALENT).
27. ALL NOMINAL STRUCTURAL LUMBER 2X8 OR LARGER (U.N.) SHALL BE No. 2 OR BETTER HEM–FIR WITH MINIMUM Fb = 850 PSI (SINGLE USE) AND MINIMUM E = 1,300,000 PSI (TYPICAL UNLESS NOTED OR REVIEWED EQUIVALENT).
28. ALL NOMINAL STRUCTURAL EXPOSED FRAMING LUMBER OR LUMBER IN DIRECT CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED NO. 2 OR BETTER SYP (SO. PINE) WITH TREATED VALUES MINIMUM Fb = 1000 PSI (SINGLE USE) AND MINIMUM E = 1,300,000 PSI (TYPICAL UNLESS NOTED OR REVIEWED EQUIVALENT).
29. SILL PLATES ON CONCRETE FOUNDATIONS OR SLABS SHALL BE PRESSURE TREATED, AND BOLTED TO THE FOUNDATION WITH NOT LESS THAN 5/8" DIA. x 11" GALVANIZED STEEL BOLTS WITH A 2" 90 DEG. HOOK EMBEDDED NOT LESS THAN 7" AND SPACED NOT FURTHER THAN 4'–0" O.C. BOLT LAYOUT SHALL BE SUCH THAT EACH PIECE RECEIVES AT LEAST (2) BOLTS AND BOLTS SHALL BE LOCATED WITHIN 12 INCHES OF EACH END (TYPICAL UNLESS NOTED). CENTER BOLTS IN PLATE WIDTH (TYP.) 5/8" DIA. HOT DIP GALVANIZED HILTI KWIK BOLT TZ EXPANSION ANCHORS MAY BE SUBSTITUTED FOR EMBEDDED ANCHORS AT INTERIOR WALLS NOT OTHERWISE INDICATED AS SHEARWALLS, AT THE CONTRACTORS OPTION.
30. PARALLAM PSL (PARALLEL STRAND LUMBER) BEAMS SHALL MEET TRUS JOIST SPECIFICATIONS: MINIMUM Fb = 2900 PSI AND MINIMUM E = 2,000,000 PSI (REFER TO PLANS). SUBSTITUTIONS SHALL NOT BE MADE WITHOUT REVIEW OF THE ARCHITECT/ENGINEER/OWNERS REPRESENTATIVE.
31. MICROLLAM LVL (LAMINATED VENEER LUMBER) BEAMS SHALL MEET TRUS JOIST SPECIFICATIONS: MINIMUM Fb = 2,600 PSI AND MINIMUM E = 2,000,000 PSI (REFER TO PLANS). SUBSTITUTIONS SHALL NOT BE MADE WITHOUT REVIEW OF THE ARCHITECT/ENGINEER/OWNERS REPRESENTATIVE.
32. TIMBERSTRAND LSL (LAMINATED STRAND LUMBER) BEAMS SHALL MEET TRUS JOIST SPECIFICATIONS: MINIMUM Fb = 2,325 PSI AND MINIMUM E = 1,550,000 PSI (REFER TO PLANS). SUBSTITUTIONS SHALL NOT BE MADE WITHOUT REVIEW OF THE ARCHITECT/ENGINEER/OWNERS REPRESENTATIVE.
33. ALL HEADERS IN EXTERIOR OR INTERIOR BEARING WALLS SPANNING MORE THAN 3'–8" SHALL BE SUPPORTED ON DOUBLE STUDS UNLESS NOTED.
34. ALL EXTERIOR PLYWOOD WALL SHEATHING EXCEPT WHERE NOTED SHALL BE 15/32" C–D EXT. – APA MINIMUM. ALL PANEL EDGES SHALL BE BACKED WITH 2 INCH NOMINAL OR WIDER FRAMING. ATTACH WITH 8d COMMON NAILS AT 6" O.C. MAXIMUM AT ALL TOP PLATES, BLOCKING, BOUNDARIES AND 12" O.C. MAXIMUM IN THE FIELD. REFER TO PLANS. APA RATED O.S.B. IS NOT ALLOWED.
35. MINIMUM NAILING SHALL CONFORM TO F.B.C. TABLE NO. 2304.10.1: USE COMMON NAILS EXCEPT WHERE NOTED.
36. THE PREFABRICATED WOOD TRUSS MANUFACTURER/SUPPLIER SHALL BE A CURRENT MEMBER OF THE TRUSS PLATE INSTITUTE (TPI) AND PARTICIPATE IN THEIR CERTIFICATION PROCESS.
37. FOR FLOOR WOOD TRUSS NOTES–REFER TO STRUCTURAL FLOOR NOTES, FLOOR DEAD LOAD = 20 PSF.
38. PLYWOOD FLOOR DECK TO BE 23/32" OR 3/4" APA RATED T&G LONG SIDES SHEATHING, EXPOSURE 1, 24" SPAN RATING, 4' X 8' SHEET (MINIMUM 2 SPAN) STAGGERED WITH APPROVED ADHESIVE AND 10d RING SHANK NAILS AT 6" CENTERS AT PANEL ENDS/SIDES AND 12" CENTER AT INTERIOR IN THE FIELD (INTERMEDIATE) SUPPORTS (TYPICAL UNLESS NOTED). APA RATED O.S.B. IS NOT ALLOWED.
39. FOR PLYWOOD ROOF SHEATHING AND WOOD TRUSS NOTES – REFER TO STRUCTURAL ROOF NOTES.
40. PROVIDE 1/8" GAP AT ALL PLYWOOD PANEL EDGES AND END JOINTS UNLESS OTHERWISE SPECIFIED BY THE MANUFACTURER. DUE TO CONSTRUCTION CONDITIONS, TEMPORARY EXPANSION JOINTS MAY BE REQUIRED IN FLOORS/ROOF SHEATHING.
41. LIGHT GAGE WOOD FRAMING CONNECTORS AS NOTED ON THE PLANS FOR WOOD JOISTS, COLUMNS, BEAMS AND TRUSSES SHALL BE "STRONG – TIE" CONNECTORS BY THE SIMPSON CO. OR REVIEWED EQUIVALENT. U.S.P. IS AN ACCEPTABLE ALTERNATE.
42. ALL BOLTS, NAILS, SCREWS, ETC. IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE HOT DIP GALVANIZED PER ASTM A123 OR ASTM A153.
43. HOLES, PIPES, SLEEVES, ETC. NOT SHOWN ON THE DRAWINGS MUST BE REVIEWED BY THE ARCHITECT BEFORE PLACEMENT THROUGH STRUCTURAL MEMBERS.
44. IF MECHANICAL AND ELECTRICAL EQUIPMENT SIZES, WEIGHTS, OR LOCATIONS DO NOT COINCIDE WITH EQUIPMENT SHOWN ON THE PLANS, COORDINATE ADJUSTMENTS WITH THE ARCHITECT/OWNERS REPRESENTATIVE.
45. NO AREA OF THE STRUCTURAL SHALL BE LOADED WITH CONSTRUCTION MATERIALS OR EQUIPMENT THAT EXCEEDS FINAL DESIGN CRITERIA.
46. THE STRUCTURE IS DESIGNED TO BE SELF–SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTORS RESPONSIBILITY TO EXECUTE AND DETERMINE FINAL ERECTION PROCEDURES, SEQUENCE AND TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES WHATEVER SHORING, SHEETING, TEMPORARY BRACING, GUYING OR TIE DOWNS WHICH MIGHT BE NECESSARY.
47. FABRICATORS AND SUPPLIERS SHALL CLEARLY NOTE AND HIGHLIGHT CHANGES MADE IN SHOP DRAWINGS WHICH DO NOT COMPLY WITH THE CONTRACT DOCUMENTS.
48. SURFACE WATER SHALL NOT BE ALLOWED TO STAND ADJACENT TO OR DRAIN TOWARDS THE FOUNDATION UNDER ANY CIRCUMSTANCES. PAVEMENTS OR GRADED SOIL AT THE PERIMETER OF THE BUILDING, EXCEPT AS REQUIRED AT EXITS OR AS NOTED, SHALL BE SLOPED AWAY AT 1/2 INCHES/FT. MINIMUM FOR THE FIRST 10 FEET.
49. IF DISCREPANCIES EXIST BETWEEN STRUCTURAL PLANS, ARCHITECTURAL PLANS, OTHER PLANS, OR SPECIFICATIONS, THE CONTRACTOR OR SUB–CONTRACTOR SHALL PROVIDE A WRITTEN REQUEST FOR CLARIFICATION FROM THE ARCHITECT AND/OR ENGINEER BEFORE PROCEEDING WITH THE WORK.
50. THE STRUCTURE IS NOT DESIGNED FOR FUTURE EXPANSION.
51. CONTRACTOR TO VERIFY ALL DIMENSIONS, ELEVATIONS AND EXISTING CONDITIONS AND REPORT ANY DISCREPANCIES TO THE ARCHITECT/OWNERS REPRESENTATIVE IMMEDIATELY.

STRUCTURAL FLOOR NOTES:

1. DESIGN WOOD FLOOR TRUSSES FOR LIVE LOADS AS STATED IN THE STRUCTURAL GENERAL NOTES PLUS DEAD LOADS AS FOLLOWS (TRUSS WEIGHT NOT INCLUDED): TOP CHORD – 10 PSF AND BOTTOM CHORD 10 PSF. LIVE LOAD DEFLECTION SHALL NOT EXCEED L/480. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TOTAL DESIGN OF TRUSSES INCLUDING LATERAL STABILITY BRACING. TRUSSES TO BE SPACED AS SHOWN ON PLANS (24" CENTERS MAXIMUM).
2. ALL PREFABRICATED WOOD TRUSSES SHALL BE ATTACHED TO BEARING PLATES WITH "SIMPSON" STRONG–TIE H2.5A FRAMING CONNECTORS AS REQUIRED OR REVIEWED EQUIVALENT.
3. WEB MEMBERS OF ADJACENT TRUSSES SHALL ALIGN WHERE POSSIBLE.
4. REFER TO STRUCTURAL GENERAL NOTES FOR THE FLOOR SHEATHING REQUIREMENTS.

REINFORCED CONCRETE MASONRY NOTES:

1. ALL CONCRETE HOLLOW MASONRY (CMU) SHALL BE MODULAR TWO HOLE (NOMINAL 8 HIGH X 16 LONG TYPICALLY) UNITS, LIGHT WEIGHT UNLESS NOTED, PLACED IN RUNNING BOND PATTERN. PROBLOCK IS STRUCTURALLY ACCEPTABLE.
2. USE LOW LIFT GROUTING PROCEDURES AND PROVIDE GROUT CONFORMING TO ASTM C476 WITH MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS.
3. MASONRY WALLS SHALL BE REINFORCED WITH NOT LESS THAN 1 – #5 VERTICAL AT 32 INCHES O.C. MAXIMUM, AT CORNERS, AT EACH SIDE OF OPENINGS AND EACH SIDE OF CONTROL JOINTS (TYPICAL UNLESS NOTED).
4. VERTICAL REINFORCEMENT SHALL BE LAPPED, 32 INCHES FOR #5 BARS, 58 INCHES (MIN.) FOR A #6 BAR, OR USE AN APPROVED MECHANICAL COUPLER.
5. VERTICAL REINFORCEMENT AT JAMBS OF OPENINGS SHALL EXTEND THE FULL HEIGHT OF WALL, UNLESS NOTED.
6. PROVIDE DOWELS FROM CONCRETE THAT ARE THE SAME SIZE AND SPACING AS MASONRY WALL VERTICALS, UNLESS NOTED.
7. HORIZONTAL REINFORCEMENT IN GROUTED CORE FILL BOND BEAMS SHALL BE CONTINUOUS AND LAPPED 30 BAR DIAMETERS (2'–0" MIN.). PROVIDE CORNER BARS OF SAME SIZE. PROVIDE STANDARD JOINT REINFORCING AT 16" O.C. (MAX.)
8. ALL REINFORCING SHALL BE PROPERLY POSITIONED WITH CENTERING AND CAGING DEVICES WITH A MINIMUM GROUT COVERAGE OF 3/4".

REINFORCED UNIT MASONRY DESIGNED PER FBC 2122 AND PROVISIONS OF TMS 402/ACI 530/ASCE 5 AND TMS 602–ACI 530.1–ASCE 6, IN ADDITION REINFORCED UNIT MASONRY STRUCTURE SHALL COMPLY WITH SECTIONS 2121

STRUCTURAL ROOF NOTES:

1. ROOF DECKING SHALL BE 19/32" OR 5/8" APA RATED SHEATHING 40/20, EXPOSURE 1, MINIMUM 2 SPAN, STAGGERED 4' X 8' SHEETS WITH 10d COMMON NAILS AT 6" CENTERS AT ALL SHEET ENDS/DISCONTINUOUS SIDES/BOUNDARIES AND 6" CENTERS MAXIMUM AT INTERMEDIATE FRAMING MEMBERS (IN THE FIELD). GABLE ENDS SHALL HAVE 10d COMMON NAILS AT 4" O.C. AT ALL PANEL EDGES AND INTERMEDIATE SUPPORTS. (SEE SHEET S1.3 FOR ADDITIONAL FASTENING LOCATIONS) USE PLYCLIPS AT MIDSPAN. APA RATED O.S.B. IS NOT ALLOWED.
2. THE WOOD PREFABRICATED TRUSS MANUFACTURER SHALL FURNISH SHOP DRAWINGS ALONG WITH SUPPORTING CALCULATIONS SEALED BY A LICENSED STRUCTURAL ENGINEER OF THE PROJECT STATE FOR ARCHITECTS REVIEW.
3. DESIGN WOOD TRUSSES FOR THE LIVE LOADS AS STATED IN THE STRUCTURAL GENERAL NOTES PLUS A 28 PSF DEAD LOAD (TOP CHORD – 14 PSF AND BOTTOM CHORD – 14 PSF: TRUSS WEIGHT NOT INCLUDED). TRUSSES TO BE SPACED AT MAXIMUM 2'–0" CENTERS.
4. ALL PREFABRICATED WOOD TRUSSES SHALL BE ATTACHED TO BEARING PLATES WITH (1) SIMPSON "STRONG–TIE" MTS12 OR HTSM18 FRAMING CONNECTOR AS REQUIRED OR REVIEWED EQUIVALENT.

TRUSS SHOP DRAWINGS NOTE:

THE WOOD ROOF AND FLOOR TRUSS MANUFACTURER SHALL SUBMIT 5 (FIVE) SETS MINIMUM OF SHOP DRAWINGS FOR REVIEW. THE SHOP DRAWINGS SHALL INCLUDE PLACING PLANS OF ALL TRUSSES CLEARLY LABELED, DETAILS OF TRUSS CONNECTIONS AND ANCHORAGES DETAILS OF METAL CONNECTORS USED AT JOINTS AND ENGINEERING DESIGN DATA. THE ENGINEERING DESIGN FOR EACH TYPE OF TRUSS SHALL INCLUDE: TRUSS LOCATION IDENTIFICATION, ALL LOADINGS AND REACTIONS, WOOD SPECIES AND STRESS GRADES, MEMBER STRESSES, JOINT CONNECTIONS, CONFIGURATION, TRUSS TO TRUSS CONNECTIONS, BRACING FOR LATERAL STABILITY OF THE COMPLETED FRAMING SYSTEM AND THE PROJECT STATE PROFESSIONAL ENGINEERS SEAL OF THE PERSON RESPONSIBLE FOR THE DESIGN OF THE TRUSSES/TRUSS SYSTEM.

THE TRUSS MANUFACTURER/SUPPLIER SHALL SUPPLY DESIGN AND CONNECTION HARDWARE FOR ALL TRUSS TO TRUSS CONNECTIONS.

WOOLPERT
115 Northridge Road
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WOODSPRING SUITES

CYPRESS CREEK, FLORIDA

Pasco County
Cypress Creek, FL

Issues & Revisions:

Proj. No.: 078761
Date: 10/02/2018

Sheet Name:

STRUCTURAL GENERAL NOTES, SCHEDULES & DETAILS

S0.1