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1746 S. SPRINGFIELD, SUITE 417
SPRINGFIELD, MO 65802

STRUCTURAL NOTES (REFER TO PROJECT MANUAL FOR ADDITIONAL INFORMATION)

- 1. FOOTINGS & FOUNDATION EXCAVATION:**
- A GEOTECHNICAL ANALYSIS HAS BEEN PERFORMED ON THIS SITE. SEE PROJECT MANUAL FOR GEOTECHNICAL INFORMATION. APPROPRIATE RECOMMENDATIONS STATED IN THE GEOTECHNICAL REPORT ISSUED BY ATC GROUP SERVICES LLC DATED JULY 11, 2016 SHALL BE FOLLOWED.
 - THESE FOUNDATIONS HAVE BEEN DESIGNED FOR A SOIL BEARING OF 2000 PSF FOR CONTINUOUS AND 2000 PSF FOR ISOLATED FOOTINGS.
 - FOUNDATIONS AND SLAB SHOULD BEAR ON DENSIFIED EXISTING SOILS OR ON APPROVED IMPORTED STRUCTURAL FILL. SEE GEOTECHNICAL REPORT FOR SPECIFIC REQUIREMENTS REGARDING EXCAVATION AND PREPARATION OF SUBGRADE. A GEOTECHNICAL ENGINEER SHOULD BE PRESENT TO DIRECT THE REMOVAL OF UNSUITABLE SOILS AND TO DETERMINE THE ADEQUACY OF THE BEARING SURFACE PRIOR TO PLACEMENT OF THE REINFORCEMENT AND CONCRETE.
 - FOOTING WIDTHS TO BE AS SHOWN ON PLANS AND DETAILS. BOTTOM OF FOOTING IS TO BE EXCAVATED SQUARE AND TRUE.
 - WHERE ANY OPEN TRENCH HAS BEEN EXPOSED TO RAIN, SNOW OR ICE PRIOR TO POURING CONCRETE, ALL REINFORCING IN THAT TRENCH SHALL BE REMOVED AND THE BOTTOM OF THE TRENCH SHALL BE DRAINED OF ALL WATER AND CLEANED OF MUD, SNOW OR ICE. A GEOTECHNICAL ENGINEER OR HIS TECHNICAL REPRESENTATIVE SHALL INSPECT THE BOTTOM OF THE TRENCH AND OBSERVE THE RE-COMPACTION OF SOILS PRIOR TO PLACING REINFORCEMENT AND POURING OF CONCRETE.
 - ALL STRIP FOOTINGS SHALL BE CENTERED UNDER WALLS BEING SUPPORTED AND ALL ISOLATED FOOTINGS SHALL BE CENTERED UNDER COLUMNS, UNLESS NOTED OTHERWISE.
 - MINIMUM EXTERIOR FOOTING DEPTH SHALL BE AS INDICATED ON THE FOUNDATION PLAN SHEET S2.
 - IN THE EVENT THAT ORGANIC SOIL OR UNCOMPACTED FILL IS FOUND BELOW FOOTINGS OR FLOOR SLABS, IT SHALL BE REMOVED AND REPLACED WITH SELECT FILL, PLACED AND COMPACTED AS DESCRIBED IN THE GEOTECHNICAL REPORT.
 - STRUCTURAL FILL SHOULD BE PLACED AND COMPACTED AS INDICATED IN THE GEOTECHNICAL REPORT. ADEQUATE DENSITY AND MOISTURE CONTENT TESTS SHOULD BE PERFORMED TO INSURE COMPLIANCE WITH PROJECT SPECIFICATIONS. SUBGRADE INSPECTION AND FILL TESTING UNDER CONTROLLED CONDITIONS IS CONSIDERED ESSENTIAL IF THE FOOTINGS ARE TO BE FOUND IN A FILL. A TESTING FREQUENCY OF AT LEAST ONE FIELD DENSITY TEST FOR EACH 2500 SQUARE FEET OF LIFT, BUT NOT LESS THAN 3 TESTS PER LIFT IS RECOMMENDED WITHIN THE BUILDING AREAS.

- 2. CONCRETE:**
- ALL READY MIX CONCRETE SHALL BE 4000 PSI FOR ALL CONCRETE PLACEMENT. DO NOT ADD WATER TO THE MIX DESIGN AFTER DELIVERY TO THE PROJECT SITE.
 - EXPOSED EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED (TOTAL AIR CONTENT = 5%). INTERIOR CONCRETE SHALL NOT BE AIR-ENTRAINED.
 - UNLESS NOTED OTHERWISE, CONCRETE COVER OVER STEEL REINFORCEMENT SHALL CONFORM TO THE MINIMUM REQUIREMENTS OF ACI 318.
 - REINFORCEMENT DETAILING AND PLACEMENT SHALL CONFORM TO ACI 318 AND ACI 315, EXCEPT WHERE OTHERWISE INDICATED.
 - HOT OR COLD WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 305-89 AND ACI 306-1-90, RESPECTIVELY.
 - ANY CONCRETE PLACED BY MEANS OF PUMPING SHALL BE DONE IN ACCORDANCE WITH ACI 304.2R (02).
 - CEMENT SHALL CONFORM TO A.S.T.M. C-150 TYPE I.
 - AGGREGATES SHALL CONFORM TO A.S.T.M. C-33 FOR NORMAL WEIGHT CONCRETE & A.S.T.M. C-330 FOR LIGHTWEIGHT CONCRETE.
 - READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH A.S.T.M. C-94.
 - ADMIXTURES MAY BE USED WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. ADMIXTURES SHOULD INCREASE DURABILITY OF THE CONCRETE SHALL NOT BE CONSIDERED TO REDUCE THE CEMENT CONTENT. NO CEMENT CHLORIDES ARE TO BE USED.

- 3. SLABS ON GRADE:**
- FLOOR SLABS ARE TO BE PLACED AND FINISHED IN ACCORDANCE WITH ACI 302-0R PROJECT MANUAL FOR ADDITIONAL INFORMATION.
 - THICKNESS TOLERANCE FOR ALL SLABS IS TO BE PER ACI 117 AND IS TO BE NO MORE THAN +3/8" (THICKNESS) AND NO MORE THAN -1/4" (THINNESS) FROM THE DESIGN THICKNESS.
 - CONCRETE USED FOR FLOOR SLABS SHALL INCLUDE SUPERPLASTICIZER. SEE PROJECT MANUAL FOR ADDITIONAL INFORMATION.

- 4. REINFORCING:**
- REINFORCING BARS SHALL BE BILLET STEEL, ASTM A601, GRADE 60. PROVIDE CONTINUOUS BENT BARS AT FOOTING STEPS AND 90 DEGREE BENT TIES AT CORNERS, UNLESS OTHERWISE NOTED. LAP SPICES OR EMBEDMENT LENGTHS SHALL CONFORM TO CLASS B SPLICE (SEE SPLICE TABLE). ADJACENT BAR SPLICES IN WALLS AND FOOTINGS TO BE ALTERNATED. ALL FOOTINGS SHALL REQUIRED HOOKED REINFORCING PROJECTED INTO WALLS, PLASTERS OR GROUTINGS. THE SIZE AND SPACING OF DONNELS ARE TO MATCH VERTICAL REINFORCING.
 - WELDED WIRE FABRIC (WFF) SHALL CONFORM TO THE CURRENT A.S.T.M. SPECIFICATION FOR COLD DRAWN STEEL REINFORCEMENT WIRE. LAP END AND EDGES MINIMUM 6".
 - REINFORCING DETAILING, BENDING, AND PLACING SHALL CONFORM TO ACI 318.
 - MINIMUM CONCRETE COVER: THE MINIMUM CLEAR DISTANCES BETWEEN REINFORCING STEEL AND FACE OF CONCRETE SHALL BE MAINTAINED UNLESS NOTED OTHERWISE:
 ON WALLS: 2"
 ON SLABS: 1"
 BELOW GRADE: 3"
 CONCRETE BELOW GRADE: UNLESS NOTED AND POURED AGAINST EARTH: 3"

- 5. LUMBER:**
- TREATED LUMBER: IN SPECIFICATIONS WHERE TREATED LUMBER IS SHOWN ON DRAWINGS, THE APPROVED PRESSURE TREATED WOODS ARE ACQ-(D) CARBONATE OR MICROTREATED WOODS WITHOUT AMMONIA CARRIERS. THE CHEMICAL RETENTION LEVELS ARE TO BE NO GREATER THAN 0.4 PCF FOR ACQ-(D), 0.21 PCF FOR CA-B. ALL METAL CONNECTORS ARE TO HAVE A GALVANIZED COATING OF NO LESS THAN 1.85 OUNCES OF ZINC PER SQUARE FOOT PER ASTM A653. ALL BOLTS, SCREWS NAILS AND OTHER FASTENERS ARE TO BE GALVANIZED PER ASTM A153. WHERE TREATED LUMBER IS SHOWN IN EXTERIOR INSTALLATIONS WITH NO ROOF COVERINGS TO PREVENT EXPOSURE TO RAIN, USE HOT DIP GALVANIZED CONNECTORS PER ASTM A123.

- 6. STRUCTURAL STEEL:**
- FABRICATOR QUALIFICATIONS: FABRICATOR MUST PARTICIPATE IN THE AISC QUALITY CONTROL PROGRAM AND BE DESIGNATED AN AISC-CERTIFIED PLANT.
 - MATERIALS:
 STRUCTURAL STEEL ASTM A992, GRADE 50 UNLESS NOTED
 PLATES, ANGLES, CHANNELS, AND MISCELLANEOUS STEEL ASTM A36
 ANCHOR RODS ASTM F1554, GRADE 36
 HIGH STRENGTH BOLTS ASTM A325 (3/4") UNLESS NOTED (OF NORTH AMERICAN MANUFACTURE)
 WELDING ELECTRODES AWS A5.1 (E70XX)
 PIPE ASTM A53, GRADE B
 SQUARE AND RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS) ASTM A500, GRADE B

- 7. LIGHT GAUGE STEEL: SHALL CONFORM TO AISI (LATEST EDITION) AND THE FOLLOWING:**
- ALL LIGHT GAUGE METAL STUDS, JOISTS AND HEADERS ARE TO MEET OR EXCEED INDUSTRY STANDARDS AS SET FORTH BY THE STEEL STUDS MANUFACTURER'S ASSOCIATION (SSMA).
 - LIGHT GAUGE STEEL MEMBER DESIGNATIONS SHOWN ON THE CONSTRUCTION DOCUMENTS ARE SSMA STANDARD DESIGNATIONS.
 - ALL LIGHT GAUGE STEEL WALL SHALL BE LATERALLY BRIDGED USING 1/2" COLD FORM CHANNELS SPACED AT 48" O.C. MAXIMUM VERTICALLY. BRIDGING CHANNEL IS TO BE POSITIVELY CONNECTED AT EACH END BY AN APPROVED METHOD.
 - BOTTOM TRACK FASTENERS TO BE SPACED AT 48" O.C. MAXIMUM AND WITHIN 6" OF DOOR / WINDOW OPENINGS AND ENDS OF WALLS.
 - POWDER ACTUATED FASTENERS SHALL BE OF HEAVY DUTY 0.1774" (1/8") LONG MANUFACTURED FROM MODIFIED AISI 1081 STEEL AUTEMPERED TO A HARDNESS OF 52-56 HRC AND ZINC PLATED IN ACCORDANCE WITH ASTM B633, SCS1, TYPE II. FASTENERS SHALL BE INSTALLED BY A QUALIFIED OPERATOR IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. POWDER ACTUATED FASTENERS SHALL BE AS MANUFACTURED BY "HILTI FASTENING SYSTEMS" OR EQUIVALENT.
 - ALL LOAD BEARING STUDS TO BE SEATED SQUARELY INTO TOP AND BOTTOM WALL TRACKS WITH NO MORE THAN A 1/4" GAP.
 - THE DESIGN OF SLIP TRACKS SHALL CONFORM TO THE GUIDELINES ESTABLISHED IN SSMA TECHNICAL NOTE NO. 1 PUBLISHED JANUARY, 2001.

- 8. GENERAL:**
- CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
 - THE STRUCTURAL DESIGN OF THE BUILDING IS BASED UPON THE FULL INTERACTION OF ALL ITS COMPONENT PARTS, WITH NO PROVISION MADE FOR CONDITIONS OCCURRING DURING CONSTRUCTION. THE STRUCTURE IS STABLE IN ITS COMPLETED FORM. THE CONTRACTOR SHALL PROVIDE ADEQUATE BRACING DURING CONSTRUCTION. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED AND INSTALLED BY THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTION ANALYSIS AND ERECTION PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC. THE STRUCTURAL ENGINEER ASSUMES NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.
 - CORRECTIONS DUE TO UNFORESEEN FIELD CONDITIONS OR DIMENSIONAL DISCREPANCIES ON CONSTRUCTION DOCUMENTS MUST BE BROUGHT TO THE ATTENTION OF THE PROJECT ARCHITECT FOR REVIEW AND AUTHORIZATION PRIOR TO CORRECTIVE MEASURES BEING IMPLEMENTED.
 - STRUCTURAL DRAWINGS ARE TO BE USED IN CONJUNCTION WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.
 - NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES.
 - ALL SHOP DRAWINGS SHALL BE REVIEWED AND APPROVED BY THE PROJECT ARCHITECT/ENGINEER PRIOR TO SUBMITTING TO THE BUILDING DEPARTMENT FOR REVIEW AND APPROVAL.

- 9. SIMPSON "AT-XP" ADHESIVE SYSTEM INTO CONCRETE (APMO UES-ER-263):**
- SUBSTITUTIONS FOR SIMPSON "AT-XP" ANCHORING ADHESIVE SHALL BE ONLY UPON THE APPROVAL OF THE PROJECT ENGINEER OF RECORD.
 - DRILL PROPER SIZE HOLE IN FULLY GROUTED MASONRY CELLS OR CONCRETE FOR THE DESIGNATED BAR AS SHOWN ON THE PLANS. CLEAN OUT HOLE WITH OIL-FREE COMPRESSED AIR. COMPLETE PREPARATION WITH USE OF A NYLON BRUSH (DO NOT USE WIRE BRUSH). BLOW OUT DUST OR FRAGMENTS.
 - PRIOR TO INJECTION, DISCHARGE AND DISPOSE OF APPROXIMATELY ONE FLUID OUNCE OF ADHESIVE. ADHESIVE MUST BE UNIFORM IN COLOR. INSERT NOZZLE INTO THE BOTTOM OF THE HOLE AND FILL ONE HALF THE HOLE DEPTH.
 - MARK SIDE OF REINFORCEMENT DONNEL OR ANCHOR PRIOR TO PLACEMENT TO INSURE THE ROD IS PLACED TO THE REQUIRED DEPTH.
 - INSERT SELECTED ROD SLOWLY BY HAND INTO THE BOTTOM OF THE HOLE USING A SLOW TWISTING MOTION.
 - LOAD ONLY AFTER RECOMMENDED CURE TIME (SEE MANUFACTURER'S RECOMMENDATIONS).

- 10. SPECIAL INSPECTIONS REQUIREMENTS:**
- OWNER SHALL ENGAGE ONE OR MORE QUALIFIED SPECIAL INSPECTORS AND/OR TESTING AGENCIES TO CONDUCT STRUCTURAL TESTS, CONSTRUCTION MATERIAL TESTING, AND SPECIAL INSPECTIONS SPECIFIED IN THE "STATEMENT OF SPECIAL INSPECTIONS".
 - FOR THE SPECIFIC RESPONSIBILITY OF THE OWNER, CONTRACTOR, AND SPECIAL INSPECTOR REFER TO SECTION 01 45 16 OF THE PROJECT MANUAL.

DESIGN CRITERIA IS BASED ON 2014 FLORIDA BUILDING CODE (2012 IBC)

ROOF DEAD LOAD:	5.5 psf	ROOF LIVE LOADS:	20.0 psf
ROOF COLLATERAL:	2.5 psf	SPRINKLER LOADS:	
CEILING SYSTEM COLLATERAL AT PARTS AREA "A":	2.0 psf	UNIFORM BRANCH PIPE LOAD	0.0 psf
		LINEAL LOOP/TEE MAIN PIPE LOAD	0.0 psf

SNOW LOAD: $P_g = 0$ psf
 $C_e = 1.0$ $C_t = 1.0$ IMPORTANCE FACTOR = 1.0
 $P_f = 0$ psf

WIND CALCULATION METHOD: ENVELOPE
 WIND SPEED: $V_{33} = 136$ mph ($V_{50} = 105$ mph)
 EXPOSURE = "C" RISK CATEGORY = "II"
 BUILDING, ENCLOSED; $G_{cp} = 0.18$

MAIN WIND DIRECTION RESISTING SYSTEM:
 WALL AND ROOF $q = 20.5$ psf
 PARAPETS $q = 21.5$ psf

WIND COMPONENTS & CLADDING:
 WALL AREA 10 SQUARE FEET OR LESS = 29.5 psf
 WALL EXTERIOR ZONES = 18.0 psf & -20.2 psf
 WALL END ZONES = 18.0 psf & -21.1 psf
 PARAPETS AT INTERIOR ZONES = 16.3 psf & -25.9 psf
 PARAPETS AT END ZONES = 16.3 psf & -29.3 psf
 ROOF INTERIOR ZONES = 7.8 psf & -22.1 psf
 ROOF EDGE ZONES = 7.8 psf & -26.2 psf
 ROOF CORNER ZONES = 7.8 psf & -26.2 psf

ALL WIND PRESSURES AND FORCES IN "ASD" UNLESS NOTED OTHERWISE

SEISMIC=5.0 K
 WIND=28.3 K

DESIGN BASE SHEAR
 SEISMIC=6.0 K
 WIND=28.9 K

EQUIVALENT FORCE PROCEDURE

SEISMIC LOAD: $S_s = 0.082$
 $S_1 = 0.052$
 MRS $S_{ds} = 0.087$
 ALL OTHERS $S_{ds} = 0.087$
 $S_{d1} = 0.083$
 SITE CLASS = D
 SEISMIC DESIGN CATEGORY = B
 $I_e = 1.0$

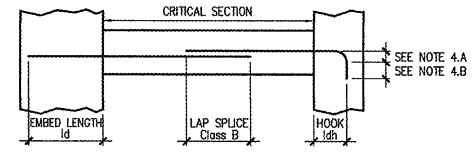
STEEL NOT SPECIFICALLY DETAILED FOR SEISMIC
 $R = 3.00$ (Perp. to Frame) $C_s = 0.029$
 $R = 3.00$ (Parallel to Frame) $C_d = 3.00$

STEEL NOT SPECIFICALLY DETAILED FOR SEISMIC
 $R = 3.00$ (Perp. to Frame) $C_s = 0.029$
 $R = 3.00$ (Parallel to Frame) $C_d = 3.00$

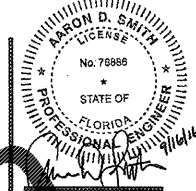
SPLICE TABLE¹
(UNLESS NOTED OTHERWISE)

BAR SIZE	LAP SPLICES (in.) ²		EMBED LENGTH (in.)		
	TOP BARS ³ Class B	OTHERS Class B	TOP BARS ³ Id	Id	HOOKS ⁵ Idh
#3	25	19	19	15	8
#4	33	25	25	19	10
#5	41	31	31	24	12
#6	49	37	37	29	15
#7	71	54	54	42	17

- SPLICE TABLE IS BASED ON THE FOLLOWING:
 A. CONCRETE $f'_c = 4000$ psi
 B. GRADE 60 REBAR
 C. BAR SPACING NOT LESS THAN 2 BAR DIAMETERS OR 1"
 D. CONCRETE COVER NOT LESS THAN ONE BAR DIAMETER
- LAP LENGTHS SHOWN ARE FOR CLASS "B" TENSION SPLICES PER ACI 318-11 CHAPTER 12.
- TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT PLACED SO THAT MORE THAN 12" OF CONCRETE IS CAST BELOW THE REINFORCEMENT IN THAT MEMBER.
- STANDARD 90° HOOKS:
 A. RADIUS = 4 BAR DIAMETERS FOR #3 THRU #8
 RADIUS = 5 BAR DIAMETERS FOR #9 THRU #11
 B. LENGTH = 12 BAR DIAMETERS
- HOOK LENGTH MAY BE REDUCED IN ACCORDANCE WITH ACI 318-11 CHAPTER 12.5



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