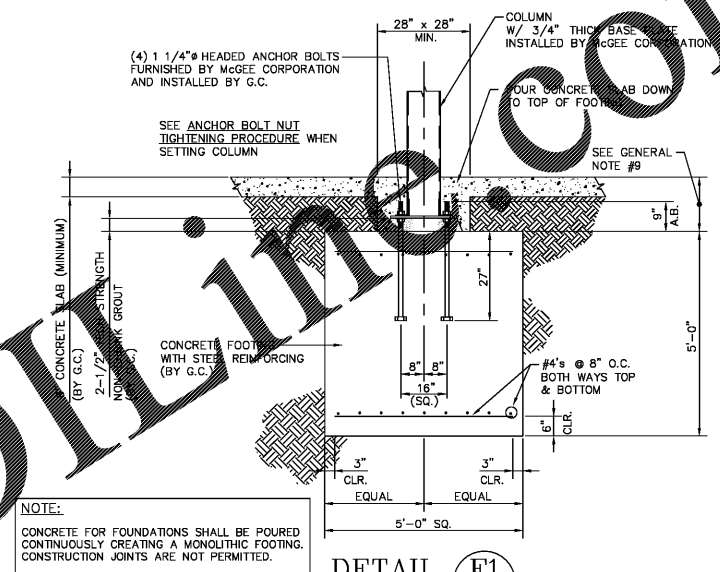
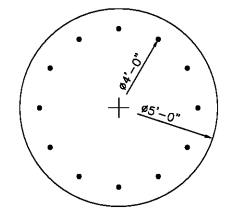


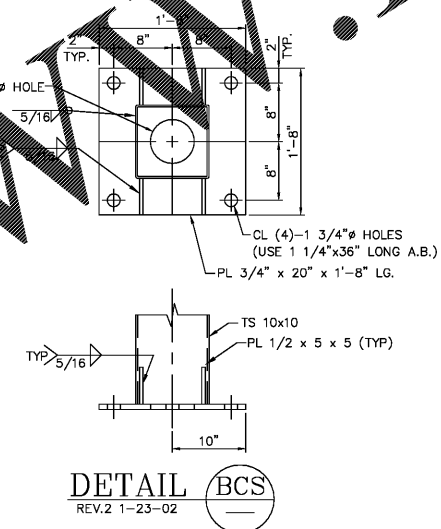
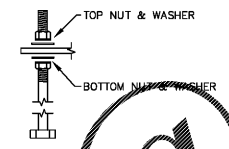
**FOUNDATION PLAN**  
ALL DIAGONAL DIMENSIONS SHOWN ARE GIVEN TO CL OF COLUMN



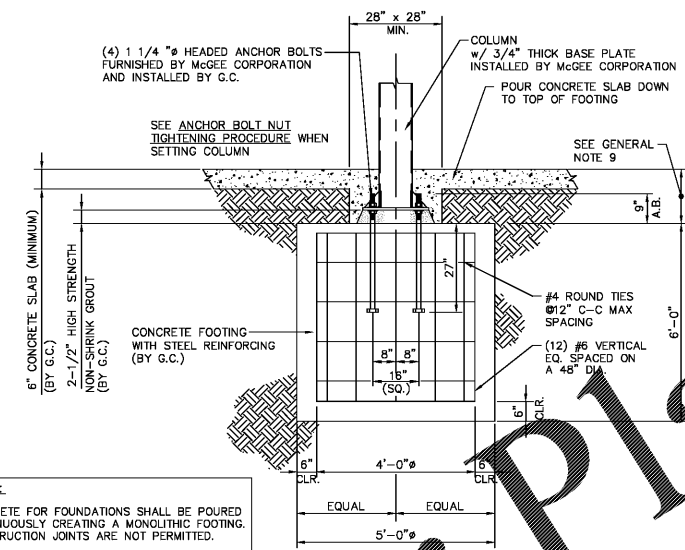
**DETAIL F1**  
REV. 01/22/03



**ANCHOR BOLT NUT TIGHTENING PROCEDURE:**  
SET AND PLUMB THE COLUMN, PER AISC ERECTION PROVISIONS, WITH DOUBLE NUTS. THE REQUIRED NUMBER OF ANCHOR BOLTS, THE BOTTOM NUT SHALL HAVE A FLAT WASHER BETWEEN THE BOTTOM OF BASEPLATE AND THE TOP OF THE NUT. THE TOP NUT SHALL HAVE A WASHER BETWEEN THE TOP OF BASEPLATE AND THE BOTTOM OF THE NUT. AFTER ALL COLUMN IS SET AND PLUMB, TIGHTEN THE TOP NUT TO A SNUG TIGHT CONDITION. TOP OF THE BASEPLATE (FULL EFFORT OF A MAN ON A WRENCH).



**DETAIL BCS**  
REV.2 1-23-02



**DETAIL F2**  
REV. 01/22/03

**SITE CONDITIONS / REQUIREMENTS**

- 1.) PROVIDE A DRIVE ACCESSIBLE AREA TO WITHIN 15'-0" FROM THE EDGE OF CANOPY FASCIA IN ORDER TO UNLOAD MATERIALS AND PERFORM WORK.
- 2.) FILL ALL OPEN TANK HOLES AND TRENCHES WITHIN 15'-0" FROM THE EDGE OF CANOPY FASCIA FROM THE TIME THAT THE STRUCTURE ARRIVES AND UNTIL ERECTION IS COMPLETE.
- 3.) THE JOB SITE MUST BE GRADED LEVEL WITH NO SWELLS, DITCHES, OR TOPOGRAPHICAL IRREGULARITIES WITHIN 15'-0" FROM THE EDGE OF CANOPY FASCIA. ANY CONCRETE POURED PRIOR TO McGEE'S ARRIVAL MUST HAVE HAD AMPLE TIME TO CURE AND BE ABLE TO SUPPORT THE WEIGHT OF McGEE'S TRAILERS AND CRANES.
- 4.) THE JOB SITE MUST BE DRY ENOUGH FOR McGEE'S VEHICLES AND PERSONNEL TO PERFORM WORK. IF NECESSARY THE GENERAL CONTRACTOR SHOULD LAY GRAVEL IN EXCESSIVELY MUDDY AREAS TO ENSURE ADEQUATE WORK CONDITIONS.
- 5.) POURED CONCRETE PAVING UNDER THE CANOPY TO BE EXCLUSIVELY FOR WORK SPACE AND STORAGE OF MATERIALS.
- 6.) REMOVE ALL OVERHEAD OBSTRUCTIONS.
- 7.) FORM, SET, AND POUR FOUNDATIONS PER McGEE'S SITE SPECIFIC APPROVED FOUNDATION PLAN. ALL FORMS SHALL BE REMOVED PRIOR TO McGEE'S ARRIVAL. ALL THREADS SHALL BE FREE FROM DEBRIS AND DUST AND SHALL BE ACCESSIBLE.
- 8.) INSTALL ALL ANCHOR BOLTS W/ NUTS. SET AT PROPER ELEVATIONS WITH NO MORE THAN 1/4" TOLERANCE.
- 9.) PROVIDE TEMPORARY POWER SOURCE (110 VOLTS) WITHIN 100 FEET OF THE STRUCTURE FOR INSTALLERS USE.
- 10.) OBTAIN ALL REQUIRED PERMITS FROM LOCAL AUTHORITIES AND ARRANGE ALL LOCAL INSPECTIONS.
- 11.) VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS. ANY DEVIATIONS FROM THESE DRAWINGS DUE TO FIELD CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE PROJECT ENGINEER FOR MODIFICATIONS.

**GENERAL NOTES:**

- 1) ERECTION OF STEEL STRUCTURE SHALL BE PERFORMED PER ALL AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) ERECTION PROVISIONS.
- 2) ALL CONCRETE WORK SHALL BE PERFORMED IN ACCORDANCE WITH ACI BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, (ACI 318-11). ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 PSI AND A MINIMUM UNIT WEIGHT OF 145 PCF. REINFORCING STEEL SHALL BE NEW BILLET STEEL DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60.
- 3) STRUCTURAL STEEL SHALL CONFORM TO: Wide Flange Beams - ASTM A992, Grade 50, Fy = 50 KSI Structural Angle and Channel - ASTM A36, Fy = 36 KSI Structural Plate - ASTM A572, Grade 50, Fy = 50 KSI Structural Tubing - ASTM A500, Grade B, Fy = 46 KSI Structural Pipe - ASTM A500, Grade B, Fy = 42 KSI
- 4) LIGHT GAUGE COLD FORMED SHAPES SHALL CONFORM TO ASTM A653 AND ASTM C-955. ALL MEMBERS SHALL BE FORMED FROM MATERIAL HAVING A 50 KSI MINIMUM YIELD STRENGTH.
- 5) BOLTS SHALL CONFORM TO ASTM A325 FOR STRUCTURAL STEEL CONNECTIONS. BOLTS SHALL BE TIGHTENED TO SNUG TIGHT PER AISC 4 RCSC SPECIFICATIONS.
- 6) MINIMUM REQUIRED SOIL BEARING PRESSURE OF 3000 PSF PER EC3 GEOTECH REPORT SHALL BE PROVIDED BY THE OWNER.
- 7) DESIGN CRITERIA: 2012 INTERNATIONAL BUILDING CODE W/ GA AMENDMENTS  
Roof Live Load = 20 PSF  
Roof Snow Load (ASCE 7-10):  
Ground Snow Load - Pg = 1.0 PSF  
Flat Roof Snow Load - Pf = 1.0 PSF  
Snow Exposure Factor - Ce = 1.0  
Snow Importance Factor - Is = 1.0 (Risk Category II)  
Thermal Factor - Ct = 1.2  
Wind Load (ASCE 7-10):  
Ultimate Wind Speed (3-Sec. Gust) - V = 115 MPH  
Lateral = 25 PSF (0.6 W FOR ASD)  
Uplift = 20 PSF (0.6 W FOR ASD)  
Wind Importance Factor - Iw = 1.0 (Risk Category II)  
Wind Exposure - B  
Internal Pressure Coefficients - GCp1 = 0.00 (Open Bldg.)  
SEISMIC LOAD: (ASCE 7-10)  
Seismic Importance Factor - Is = 1.00 (Risk Category II)  
Risk Category - II  
Mapped MCEer Response Accelerations At Short Periods - Ss = 0.3 g - Fa = 1.2  
Mapped MCEer Response Accelerations At 1-Sec. Period - S1 = 0.11 g - Fv = 1.69  
Site Class - C PER EC3 GEOTECH REPORT  
Design Spectral Response Acceleration At Short Periods - Sps = 0.24 g  
Design Spectral Response Acceleration At 1-Sec. Period - Sp1 = 0.124g  
SEISMIC DESIGN CATEGORY - B
- 8) CANOPY FOUNDATION INSTALLATION: CONTRACTOR SHALL DETERMINE WHICH FINISHED GRADE ELEVATION AT EACH CANOPY COLUMN IS THE LOWEST AND ESTABLISH ALL FOUNDATION LOCATIONS IN RELATION TO THAT ELEVATION. CONTRACTOR MUST VERIFY FUEL CONTAINMENT BOX SIZE AND LOCATION TO ENSURE FOUNDATION DOES NOT INTERFERE WITH BOX INSTALLATION. TOP OF FOUNDATION DEPTH MAY BE GREATER THAN BUT NOT LESS THAN 12" BELOW THE PREVIOUSLY DETERMINED LOWEST FINISHED GRADE ELEVATION.
- 9) STRUCTURAL AND MISCELLANEOUS STEEL SUBJECTED TO EXTERIOR EXPOSURE HAS BEEN PRIME COATED ONLY. FIELD TOUCH-UP, FINISH PAINTING AND MAINTENANCE ARE THE RESPONSIBILITY OF THE OWNER. THEIR DESIGN ASSUMES THAT THERE ARE NO BURIED TANKS OR OTHER NEARBY OBSTRUCTIONS THAT WOULD BE DETRIMENTAL TO THEIR PROPER FUNCTION. THE ENGINEER OF RECORD SHALL BE NOTIFIED PRIOR TO CONSTRUCTION OF FOUNDATIONS FOR THE RESOLUTION OF ANY CONFLICT. WHERE FOUNDATION DETAIL IS NOT SHOWN McGEE CORPORATION AND THEIR ENGINEERS TAKE NO RESPONSIBILITY FOR FOUNDATION DESIGN.
- 10) ALL WELDED CONNECTIONS SHALL BE IN ACCORDANCE WITH LATEST AWS SPECIFICATIONS, USING E70XX ELECTRODES. ALL WELDING SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER.
- 11) USE GROUP M / CONSTRUCTION TYPE II-B

BASIC SEISMIC - FORCE - RESISTING SYSTEM - INVERTED PENDULUM SYSTEM  
CANTILEVERED COLUMN SYSTEM  
Response Modification Coefficient - R = 2  
System Overstrength Factor - do = 2  
Deflection Amplification Factor - Cd = 2  
SEISMIC RESPONSE COEFFICIENT - Cs = 0.134  
SEISMIC BASE SHEAR - V = 1.2 KIPS / COL  
ANALYSIS - EQUIVALENT LATERAL FORCE PROCEDURE  
ASTM F1554 GR. 36 OR A36 (fy = 36 KSI) HEADED ANCHOR RODS 4 WOOD TEMPLETES SHALL BE FURNISHED BY McGEE CORP.

PLEASE REVIEW ALL DRAWINGS, SIGN AND RETURN FOR FABRICATION OF CANOPY

<b>CANOPY SIZE</b>	<input type="checkbox"/> APPROVED AS SUBMITTED
	<input type="checkbox"/> APPROVED WITH NOTED CHANGES
<b>COLUMN SPACING</b>	<input type="checkbox"/> APPROVED AS SUBMITTED
	<input type="checkbox"/> APPROVED WITH NOTED CHANGES
<b>CLEARANCE</b>	<input type="checkbox"/> APPROVED AS SUBMITTED
	<input type="checkbox"/> APPROVED WITH NOTED CHANGES
<b>SIGNAGE</b>	<input type="checkbox"/> NUMBER APPROVED AS SUBMITTED
	<input type="checkbox"/> LAYOUT APPROVED AS SUBMITTED
	<input type="checkbox"/> APPROVED WITH NOTED CHANGES
<b>DECALS</b>	<input type="checkbox"/> APPROVED AS SUBMITTED
	<input type="checkbox"/> APPROVED WITH NOTED CHANGES
<b>LIGHTS</b>	<input type="checkbox"/> NUMBER APPROVED AS SUBMITTED
	<input type="checkbox"/> LAYOUT APPROVED AS SUBMITTED
	<input type="checkbox"/> APPROVED WITH NOTED CHANGES

ELEVATION FORMS FORWARDED TO GENERAL CONTRACTOR

APPROVED BY: \_\_\_\_\_  
DATE: \_\_\_\_\_

NOTE: SIGNED SALES ORDER, APPROVAL DRAWINGS, AND A COMPLETED ELEVATION FORM MUST BE RECEIVED AT LEAST 3 WEEKS PRIOR TO DELIVERY OF ANY CANOPY MATERIALS. REQUESTED DELIVERY DATE: \_\_\_\_\_

**McGEE CORPORATION**  
12701 East Independence Blvd. P.O. Box 1375 Matthews, NC 28106-1375  
Phone: (704) 882-1900  
Fax: (800) 528-5589

PRJ. JOB NO. 5644B  
FINAL JOB NO. 5644B  
DRAWING NO. P05644B  
**RACETRAC STATION #1310**  
1325 E MAIN ST  
CARTERSVILLE, GA 30120 (BARTOW)  
SCALE: 1/8"=1'-0" IN ACCORDANCE WITH REV. LETTER:  
DATE: 7/31/18  
DRAWN BY: LLE  
CHKD BY:  
**METAL CANOPY 54'-0" x 126'-4"**  
FOUNDATION PLAN  
SHEET NO. 1 OF 3

**LAWRENCE R. PILON / PROFESSIONAL ENGINEER**  
51 MAPLEVIEW DRIVE, PENNELLVILLE, NY 13132  
(315) 668-0039

REGISTERED PROFESSIONAL ENGINEER  
STATE OF GEORGIA  
No. 24363  
LAWRENCE R. PILON