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**HARMONY LELAND CLAY
REPLACEMENT ELEMENTARY SCHOOL**
6528 FACTORY SHOALS RD, SW
FOR COBB COUNTY SCHOOL DISTRICT
COBB COUNTY, GA
CCSD PROJECT NO. A044 / DDE FACILITY CODE

DATE: JULY 18, 2019
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ABBREVIATIONS

ACI	AMERICAN CONCRETE INSTITUTE	K	KIPS (KILOPOUNDS)
ADDL	ADDITIONAL	KLF	KIPS PER LINEAL FOOT
AESS	ARCHITECTURAL EXPOSED	KSI	KIPS PER SQUARE INCH
AFF	ARCHITECT	KSF	KIPS PER SQUARE FOOT
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	L	LENGTH
AISI	AMERICAN IRON AND STEEL INSTITUTE	LH	LONG FACE HORIZONTAL
ALTN	ALTERNATE	LFV	LONG FACE VERTICAL
ANCH	ANCHOR ROD	LG	LONG
ARCH	ARCHITECT	LL	LIVE LOAD
ASD	ALLOWABLE STRESS DESIGN	LLH	LONG LEG HORIZONTAL
ASTM	AMERICAN SOCIETY OF TESTING AND MATERIALS	LLV	LONG LEG VERTICAL
AWS	AMERICAN WELDING SOCIETY	LO	LOADS
B/	BOTTOM	LRFD	LOAD RESISTANCE FACTORED DESIGN
BD	BOARD	L	LONG SIDE HORIZONTAL
BETW	BETWEEN	LV	LONG SIDE VERTICAL
BLDG	BUILDING	M	MINIMUM
BM	BENCH MARK	MEP	MECHANICAL, ELECTRICAL & PLUMBING
BOT	BOTTOM	MEZZ	MEZZANINE
BP	BRAKE	MFR	MANUFACTURER
BRDG	BRIDGE	MIN	MINIMUM
BRG	BEARING	MISC	MISCELLANEOUS
C/C	CENTER TO CENTER	MPII	MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS
CFSF	COLD FORMED STEEL FRAMING	MTL	METAL
CJ	CONTROL JOINT	NIC	NOT IN CONTRACT
CL	CENTERLINE	NS	NEAR SIDE
CLR	CLEAR	NTS	NOT TO SCALE
CMU	CONCRETE MASONRY UNIT	OC	ON CENTER
CONC	CONCRETE	OD	OUTSIDE DIAMETER
CONN	CONNECTION	OH	OPPOSITE HAND
CONT	CONTINUOUS	OPNG	OPENING
CTR	CENTER	P/AF	POWDER ACTUATED FASTENERS
D	DEEP	PEMB	PRE-ENGINEERED METAL BUILDING
DBA	DEFORMED BAR ANCHOR	PJF	PREFORMED JOINT FILLER
DBL	DOUBLE	PL	PLATE
DEP	DEPRESSED	PLF	POUNDS PER LINEAL FOOT
DIA	DIAMETER	PPHCC	PRESTRESSED PRECAST HOLLOW CORE CONCRETE
DIAG	DIAGONAL	PREFAB	PREFABRICATED
DIAW	DEAD LOAD	PSI	POUNDS PER SQUARE INCH
DN	DOWN	PSF	POUNDS PER SQUARE FOOT
EA	EACH	PT	POST TENSIONED
EF	EACH FACE	P.T.	PRESSURE TREATED
EFAS	EXPANSION JOINT	QTY	QUANTITY
ELEV	ELEVATION	R/AF	RAMMED AGGREGATE PIER
ENG	ENGINEER OR ENGINEERING	RD	ROOF DRAIN
EOS	EDGE OF SLAB	REF	REFERENCE
EQ	EQUAL	REINF	REINFORCING
EW	EACH WAY	REQD	REQUIRED
EXIS	EXISTING	REV	REVISION
EXT	EXTERIOR	RTU	ROOF TOP UNIT
F	FACE OF	SCHED	SCHEDULE
FD	FLOOR DRAIN	SER	STRUCTURAL ENGINEER
FDN	FOUNDATION	SF	SQUARE FOOT
FF	FRESH FLOOR	SHTHG	SHEATHING
FLR	FLOOR	SM	SIMILAR
FRT	FIRE RETARDANT TIMBER	SLH	SHORT LEG HORIZONTAL
FS	FAR SIDE	SLV	SHORT LEG VERTICAL
FTG	FOOTING	SPA	SPACES
FV	FIELD VERIFY	SPEC	SPECIFICATION
GA	GALVE GAGE	SS	STAINLESS STEEL
GALV	GALVANIZED	STD	STANDARD
GC	GENERAL CONTRACTOR	STIFF	STIFFENER
GDR	GRIDDER	STL	STEEL
GENL	GENERAL	STR	STRUT
GYP	GYP-SUM	SYM	SYMMETRICAL
HCA	HEADED CONCRETE ANCHORS	T	TOP OF
HDR	HEADER	T&B	TOP & BOTTOM
HG	HP GIRDER	T&G	TONGUE & GROOVE
HGR	HANGER	TEMP	TEMPORARY
HK	HIGH	TG	TRUSS GIRDER
HKD	HOOKED	THK	THICKENED OR THICK
HORIZ	HORIZONTAL	THRU	THROUGH
HSS	HOLLOW STRUCTURAL SECTION	TYP	TYPICAL
H.T.	HEAVY TIMBER	UNO	UNLESS NOTED OTHERWISE
ID	INSIDE DIAMETER	VERT	VERTICAL
IE	INVERT ELEVATION	W	WIDE
INSUL	INSULATION OR INSULATING	W/	WITH
INT	INTERIOR	W/O	WITHOUT
JST	JOINT	WD	WOOD
JT	JOINT	WP	WORK POINT
		WWR	WELDED WIRE REINFORCEMENT

CONCRETE MIXTURES

APPLICATION	EXPOSURE	F _c (AT 28-DAYS UNO)	MAXIMUM W/C	AIR CONTENT	NOMINAL MAXIMUM AGGREGATE SIZE (NOTE 4)	MAXIMUM CONCRETE WEIGHT
FOOTINGS	F0, S0, P0, C0	3000 PSI	SEE NOTE 2	SEE NOTE 3	1"	150 PCF
EXTERIOR SLAB-ON-GRADE	F1, S0, P0, C1	4000 PSI @ 28-DAYS 4500 PSI @ 56-DAYS	0.45	4.5% ± 1.5%	1"	150 PCF
INTERIOR SLAB-ON-GRADE	F0, S0, P0, C0	4000 PSI	SEE NOTE 2	SEE NOTE 3	1"	150 PCF
WALLS	F0, S0, P0, C0	4000 PSI	SEE NOTE 2	SEE NOTE 3	3/4"	150 PCF
LIGHTWEIGHT ELEVATED SLAB ON STEEL DECK	F0, S0, P0, C0	4000 PSI	SEE NOTE 2	5.5% ± 1.5%	3/4"	120 PCF

NOTES:

- EXPOSURE CATEGORIES AND CLASSES FOR SULFATES, PERMEABILITY, AND CORROSION PROTECTION OF REINFORCEMENT IS CLASS ZERO UNLESS NOTED OTHERWISE.
- WHERE NO MAXIMUM WATER CEMENT RATIO IS NOTED FOR DURABILITY, PROPORTIONING OF WATER CEMENT RATIO SHALL BE AS REQUIRED FOR SPECIFIED CONCRETE MIX DESIGN. WATER CEMENT RATIO IS NOT APPLICABLE FOR DURABILITY REQUIREMENTS IN LIGHTWEIGHT CONCRETE.
- WHERE AIR ENTRAINMENT IS NOT REQUIRED BY DESIGN, THE CONTRACTOR, INSTALLER, AND SUPPLIER MAY CHOOSE TO INCLUDE AIR ENTRAINMENT TO IMPROVE PLACEMENT AND FINISHING CHARACTERISTICS. AIR ENTRAINMENT IS NOT PERMITTED IN NORMAL WEIGHT CONCRETE TO RECEIVE A HARD TROWEL FINISH AND ENTRAPPED AIR SHALL NOT EXCEED 3%. AIR ENTRAINMENT IN LIGHTWEIGHT CONCRETE SLABS IS REQUIRED TO MEET FIRE RATING REQUIREMENTS. SLABS SHALL BE PROPERLY FINISHED TO AVOID SURFACE IMPERFECTIONS, SUCH AS BLISTERS OR DELAMINATION.
- COARSE AGGREGATE SHALL BE ASTM C 33, GRADED. SELECT GRADING CLASS PER TYPE OF CONSTRUCTION OR LOCATION USED, AND IN RELATION TO SPECIFIC WEATHERING REGION. AGGREGATE SHALL BE FROM A SINGLE SOURCE. #57 GRADING SHALL BE USED FOR CONCRETE WITH 3/4" INCH MAXIMUM; #57 GRADING SHALL BE USED FOR CONCRETE WITH 1" INCH MAXIMUM.

WATER PIPING SUPPORT SCHEDULE

PIPE DIA. (IN.)	PIPE WEIGHT (LB./FT.)	PIPE SUPPORT SPACING (MAX.) (FT.)
2 1/2	6.0	12
3	8.5	12
4	12.0	12
5	17.5	12
6	23.5	6
8	37.5	6

NOTES:

- PIPES IN TABLE ARE SCHEDULE 10 TYPE. ESFR MANUFACTURER TO SUBMIT DOCUMENTATION FOR ALTERNATIVE PIPING NOT NOTED.
- PIPE WEIGHT INCLUDES: PIPE + INSULATION + WATER.
- EXACT PIPE LOCATIONS TO BE COORDINATED WITH MECHANICAL DRAWINGS.
- PIPES RUNNING PARALLEL TO JOISTS W/ DIA. GREATER THAN 4" OR RUNNING IN COMBINATION W/ OTHER PIPES SHALL BE DISTRIBUTED TO A MINIMUM OF 2 JOISTS.
- MEMBER SIZES ON PLANS HAVE BEEN ADJUSTED TO SUPPORT WATER PIPING LOADS IN THIS TABLE.
- ANY PIPE OR COMBINATION OF PIPES WITH TOTAL DIAMETERS GREATER THAN 8" SHALL BE HUNG PER THE DETAILS OF THE ARCH. - NOTIFY ARCH. PRIOR TO PROCEEDING W/WORK.
- NO PIPING SHALL RUN BELOW THE BOTTOM CHORD OF THE BAR JOIST.

BRICK/STONE VENEER LINTEL SCHEDULE

SPAN (OPENING)	LINTEL SIZE
0'-0" - 4'-0"	L7 x 4 x 3/8 (LLH)
4'-0" - 6'-0"	BENT PLATE 7 x 6 x 3/8 (LLH)
6'-0" - 8'-0"	BENT PLATE 7 x 6 x 3/8 (LLH)
8'-0" - 12'-0"	BENT PLATE 7 x 6 x 3/8 (LLH)
12'-0" - 16'-0"	BENT PLATE 7 x 6 x 3/8 (LLH)

DETAILS:

NOTES:

- PROVIDE 4" BEARING (MIN) AT EACH END OF LINTEL ANGLES.
- * DENOTES TO BOLT VENEER LINTEL @ 3RD POINTS WITH 3/4" DIA EXP ANCHORS x 6" LG OR CONNECT TO STEEL HANGERS PER SECTIONS.

CONCRETE MASONRY UNIT LINTEL & JAMB SCHEDULE

SPAN	LINTEL DEPTH "D"	LINTEL REINFORCING			JAMB REINFORCING (SEE NOTE 5)
		6" WALL	8" WALL	12" WALL	
≤ 4'-0"	8"	N/A	(2) - #5	N/A	(1) - #5
> 4'-0" TO ≤ 8'-0"	8"	N/A	(2) - #5	N/A	(2) - #5
> 8'-0" TO ≤ 12'-0"	16"	N/A	(2) - #5	N/A	(2) - #5
> 12'-0" TO ≤ 16'-0"	24"	N/A	(2) - #5	N/A	(3) - #5

NOTES:

- LINTELS SHALL BE CONSTRUCTED WITH U-BLOCKS AT THE BOTTOM AND DEPRESSED WEB BLOCKS ABOVE AND SHALL BE FULLY GROUTED.
- SILL REINFORCING REQUIREMENTS:
 - OPENING WIDTH ≤ 6'-0" AND SILL HEIGHT ≤ 3'-0": SILL REINFORCING NOT REQUIRED.
 - OPENING WIDTH > 6'-0" OR SILL HEIGHT > 3'-0": PROVIDE REINFORCED LINTEL ACCORDING TO SCHEDULE.
- NOTIFY STRUCTURAL ENGINEER OF RECORD IF OPENING WIDTH EXCEEDS SCHEDULED WIDTHS.
- SEE CONCRETE MASONRY GENERAL NOTES AND RELEVANT SCHEDULES, SECTIONS, AND DETAILS FOR ADDITIONAL REINFORCING AND REQUIREMENTS NOT SHOWN IN THIS SCHEDULE.
- PROVIDE (1) BAR PER CELL IN JAMBS, TYPICAL. WHERE SCHEDULED JAMB REINFORCING EXCEEDS THE NUMBER OF CELLS, PROVIDE (2) BARS PER CELL LOCATED 3/4" CLEAR FROM EACH FACE SHELL.

TENSION DEVELOPMENT LENGTHS (ACI 318, SECTION 12.2.2)

BAR SIZE	F _c = 3000 PSI		F _c = 4000 PSI	
	CASE 1	CASE 2	CASE 1	CASE 2
#3	21	32	16	25
#4	27	41	22	33
#5	33	50	27	41
#6	43	66	33	49
#7	56	86	43	62
#8	71	107	55	82

NOTES:

- TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE PLACED BELOW THE DEVELOPMENT OR SPLICE.
- REBAR IS ASSUMED TO BE UNCOATED (NO EPOXY COATING). INCREASE DEVELOPMENT LENGTHS DOWN BY 1.3 FOR TOP, AND 1.5 FOR OTHER EPOXY COATED BARS.
- FOR LIGHTWEIGHT CONCRETE, MULTIPLY TABULATED VALUES BY 1.3.
- LAP SPLICE LENGTHS SHALL BE AS SHOWN IN THE TABLE ABOVE, UNLESS NOTED OTHERWISE.
- WHERE BARS OF DIFFERENT SIZES ARE LAP SPICED, THE LAP SPLICE LENGTH SHALL BE THE LARGER OF THE TENSION DEVELOPMENT LENGTH OF THE LARGER BAR AND TENSION LAP SPLICE LENGTH OF THE SMALLER BAR.

CONCRETE MASONRY UNIT REINFORCEMENT PLACEMENT TOLERANCE

NOTES:

- UNLESS NOTED OTHERWISE, LOCATE REINFORCING BARS AT THE CENTER OF CELLS WITHIN THE VERTICAL AND HORIZONTAL TOLERANCES NOTED.

CAST-IN-PLACE CONCRETE CLEAR COVER SCHEDULE

CONCRETE COVER	CONCRETE COVER
CONCRETE CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND	3 IN
CONCRETE IN CONTACT WITH GROUND OR WEATHER: #6 THROUGH #18 BARS	2 IN
#6 BAR, W31 OR D31 WIRE, AND SMALLER	1 1/2 IN
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, WALLS, JOISTS: #14 AND #18 BARS	1 1/2 IN
#11 BAR AND SMALLER	3/4 IN
BEAMS, COLUMNS: PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1 1/2 IN

POST-INSTALLED ANCHORS SPECIFIED PRODUCTS BY APPLICATION

ANCHOR TYPE	CONCRETE	CONCRETE MASONRY
EXPANSIVE ANCHORS	HILTI KWIK-BOLT T2 SIMPSON HANCOCK-BOLT 2 WALTON POWER-STUD+ SD2	HILTI KWIK-BOLT 3 SIMPSON STRONG-BOLT 2 DEWALT POWER-POWER-STUD+ SD1
SCREW ANCHORS	HILTI HUS-EZ SIMPSON TITEN HD DEWALT POWER SCREW-BOLT+	HILTI HUS-EZ SIMPSON TITEN HD DEWALT POWER SCREW-BOLT+
ADHESIVE ANCHORS (EPOXY ANCHORS) W/ REBAR	HILTI HIT-HY20 SIMPSON SET-3G DEWALT POWER PURE110+	HILTI HIT-HY70 SIMPSON SET-3P DEWALT POWER AC108+ GOLD
ADHESIVE ANCHORS (EPOXY ANCHORS) W/ REBAR	HILTI HIT-HY20 SIMPSON SET-3G DEWALT POWER PURE110+	

NOTES:

- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USING POST INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REINFORCING. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.
- SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED, SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD WITH CALCULATIONS THAT ARE PREPARED BY A REGISTERED DESIGN PROFESSIONAL IN THE STATE IN WHICH THE PROJECT IS LOCATED SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE REFERENCED BUILDING CODE.
- ALTERNATE PRODUCTS SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL SHALL HAVE A VALID RESEARCH REPORT, ALSO KNOWN AS EVALUATION REPORT, INDICATING COMPLIANCE WITH APPROPRIATE ACCEPTANCE CRITERIA REQUIRED BY THE BUILDING CODE FOR THE INTENDED LOAD TYPE AND USE (E.G. WIND, SEISMIC, SUSTAINED TENSION, ETC). RESEARCH REPORTS SHALL BE ISSUED BY A SOURCE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
- ADHESIVE ANCHOR DESIGN TEMPERATURE RANGE IS 110°F (LONG TERM) AND 130°F (SHORT TERM).
- IN ADDITION TO THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, THE FOLLOWING GUIDELINES SHALL BE FOLLOWED FOR INSTALLATION OF ADHESIVE ANCHORS:
 - ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION.
 - ADHESIVE ANCHORS SHALL BE INSTALLED IN DRY CONCRETE, AND DURING DRY CONDITIONS.
 - ADHESIVE ANCHORS SHALL BE INSTALLED IN HOLES PREDRILLED WITH A CARBIDE TIPPED DRILL BIT.
 - ADHESIVE ANCHORS SHALL BE INSTALLED WITHIN THE TEMPERATURE RANGE SPECIFIED IN THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, BUT NOT OUTSIDE OF THE DESIGN TEMPERATURE RANGE. LOADS SHALL NOT BE APPLIED TO ADHESIVE ANCHORS UNTIL THE FULL CURING TIME ASSOCIATED WITH THE INSTALLATION TEMPERATURE HAS ELAPSED.
 - INSTALLATION OF ADHESIVE ANCHORS SHALL BE PERFORMED BY CERTIFIED PERSONNEL. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ANCHOR ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM OR EQUIVALENT.
 - SPECIAL INSPECTIONS SHALL BE PROVIDED FOR POST-INSTALLED ANCHORS IN ACCORDANCE WITH THE ANCHOR MPII AND/OR EVALUATION REPORT, UNLESS MORE SPECIFIC REQUIREMENTS ARE SPECIFIED IN THE CONSTRUCTION DOCUMENTS.
 - WHEN ANCHORING TO CONCRETE MASONRY WITH VOIDS, THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER.

COMPONENTS & CLADDING EXTERNAL PRESSURE LOADS (PSF)

IBC 2012 LOCATION PER ASCE 7-10: FIGURE 30.4-2A, 30.4-1

EFFECTIVE WIND AREA (FT ²)	1					2					3					4					5									
	<10	16	16	16	16	34	34	34	34	34	34	-37	-42	-42	-37	-45	16	16	16	16	33	33	33	33	33	33	-36	-46	-46	-36
20	16	16	16	16	31	31	31	31	31	31	16	16	16	16	31	16	16	16	16	29	29	29	29	29	29	-34	-40	-40	-32	-35
50	16	16	16	16	29	29	29	29	29	29	16	16	16	16	29	16	16	16	16	27	27	27	27	27	27	-34	-40	-40	-32	-35
>100	16	16	16	16	26	26	26	26	26	26	16	16	16	16	26	16	16	16	16	26	26	26	26	26	26	26	26	26	26	26
>500	16	16	16	16	26	26	26	26	26	26	16	16	16	16	26	16	16	16	16	26	26	26	26	26	26	26	26	26	26	26

NOTES:

- a = 15'-0". SEE ROOF PLAN MAP BELOW FOR LOCATION OF a-ZONES. WALL a-ZONE LOCATIONS TO MATCH ROOF a-ZONES.
- POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING OR COMPONENT FACE. NEGATIVE PRESSURE VALUES REFER TO FORCES ACTING AWAY FROM BUILDING OR COMPONENT FACE.
- EACH COMPONENT AND ITS CONNECTION SHALL BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE PRESSURES.
- FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES, DESIGN LOADS MAY BE INTERPOLATED. OTHERWISE DESIGN LOAD SHALL BE TAKEN FROM THE NEXT LOWEST TABULATED EFFECTIVE AREA.
- DESIGN VALUES SHOWN IN THIS TABLE ARE ULTIMATE VALUES FOR USE WITH LRFD DESIGN. VALUES MAY BE MULTIPLIED BY 0.6 FOR USE WITH SERVICE LEVEL OR ASD DESIGN. REFER TO THE BUILDING CODE FOR APPLICABLE LOAD COMBINATIONS.

COMPONENTS & CLADDING @ BUS CANOPIES

EFFECTIVE WIND AREA (FT ²)	EXTERNAL PRESSURE (PSF)	
	POSITIVE	NEGATIVE
≤ 9	54	-75
9' - < 36	41	-59
> 36	27	-39

NOTES:

- PARAPET COMPONENTS AND CLADDING ARE THOSE ELEMENTS WHICH EXIST ABOVE THE HORIZONTAL PLANE OF THE ROOF AND SHALL BE DESIGNED FOR:
 - POSITIVE AND NEGATIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON OUTSIDE FACE.
 - POSITIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF FACE.
 - NEGATIVE PRESSURES 2 OR 3 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - P4S SHALL BE APPLIED TO THE DESIGN OF THE STRUCTURAL ELEMENT OF THE PARAPET AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE PARAPET.
- A DESIGN WIND PRESSURE HORIZONTAL VALUE OF 60 PSF AND VERTICAL VALUE OF 47 PSF SHALL BE APPLIED TO COMPONENTS WHICH ARE EITHER ROOFTOP STRUCTURES OR ROOFTOP APPURTENANCES AND THEIR CONNECTION. EXAMPLES OF THIS ARE RTUs, AHUs, AND SCREEN WALLS.
- ROOF DENOTES DESIGN WIND PRESSURE VALUES WHICH SHALL BE APPLIED AT ROOF OVERHANGS TO TOP SURFACE CLADDING OR SHEATHING AND ITS CONNECTION. SOFFIT CLADDING OR SHEATHING SHALL BE DESIGNED FOR SIMILAR PRESSURE TO THE ADJACENT WALL PRESSURE. A COMBINATION OF THESE FORCES SHALL BE APPLIED TO THE STRUCTURAL ELEMENT OF THE OVERHANG AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE OVERHANG.

CONCRETE MASONRY UNITS REINFORCING LAP SPLICE LENGTHS

SIZE	BAR SIZE					
	#3	#4	#5	#6	#7	#8
9" CMU	16"	21"	26"	31"	36"	M
12" CMU	16"	21"	26"	31"	36"	74"

NOTES:

- F_m = 1500 psi.
- REINFORCING IS ASSUMED TO BE UNCOATED (NO EPOXY COATING).
- REBAR LOCATED AT CENTER OF CELL.
- * DENOTES MECHANICAL BAR SPLICE IS REQUIRED. SPLICE SHALL BE TOP 12" FOR SPECIFIED YIELD STRENGTH OF THE BAR. TENSION COMPRESSION.

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