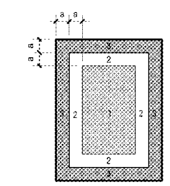


FLAT ROOF 7-27 DEG.

a = 6' - 0" MIN.

(150 MPH) ALLOWABLE (ASD) WIND PRESSURES FOR COMPONENTS & CLADDING (ASCE 7-10)						
EFFECTIVE AREA (SQ FT)	ZONE 1		ZONE 2		ZONE 3	
	PRESSURE	SUCTION	PRESSURE	SUCTION	PRESSURE	SUCTION
10	38 PSF	-36.6 PSF	57 PSF	-55.4 PSF	76.1 PSF	-90.4 PSF
20	38 PSF	-36.6 PSF	57 PSF	-55.4 PSF	76.1 PSF	-90.4 PSF
50	38 PSF	-36.6 PSF	57 PSF	-55.4 PSF	57 PSF	-55.4 PSF
100	38 PSF	-36.6 PSF	57 PSF	-55.4 PSF	57 PSF	-55.4 PSF
A ≥ 380	38 PSF	-36.6 PSF	38 PSF	-36.6 PSF	38 PSF	-36.6 PSF

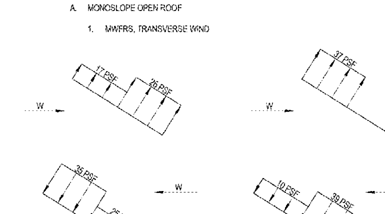
NOTES:
1. 'a' DENOTES EDGE STRIP = 6'-0"
2. POSITIVE & NEGATIVE SIGNS SIGNIFY PRESSURES ACTING TOWARDS AND AWAY FROM THE BUILDING SURFACES, RESPECTIVELY.
3. WIND PRESSURES INDICATED ABOVE ARE ALLOWABLE STRESS DESIGN WIND PRESSURES. DIVIDE THE PRESSURES BY A FACTOR OF 0.6 TO OBTAIN STRENGTH DESIGN WIND PRESSURES

COMPONENTS & CLADDING PRESSURES

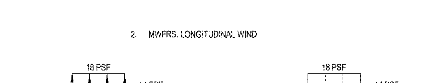
NON-BLDG STRUCTURE WIND LOAD

A. MONOSLOPE OPEN ROOF

1. MWFRS, TRANSVERSE WIND



2. MWFRS, LONGITUDINAL WIND



WIND PRESSURES & VELOCITIES GIVEN HEREIN ARE NOMINAL (V nom)

3. ROOF WL PRESSURE (MWFRS & C&C) ARE THE COMBINED EFFECT OF WIND IN BOTH SURFACES (TOP & BOTTOM)

B. 2D TRUSSES, 46 PSF ON SOLID AREA, EACH TRUSS SIMULTANEOUSLY

C. SIGN, 46 PSF. CASES A,B, AND C CONSIDERED

D. COLUMNS, SQUARE. 46 PSF ALL COLUMNS SIMULTANEOUSLY IN THE WIND DIRECTION BEING CONSIDERED

GENERAL

A. THESE GENERAL NOTES ARE NOT INTENDED TO REPLACE SPECIFICATIONS. SEE SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES.

B. NOT ALL EXISTING CONDITIONS, PROPOSED CONDITIONS, OR UTILITIES ARE SHOWN ON THE STRUCTURAL DRAWINGS. COORDINATE STRUCTURAL WORK WITH THE WORK OF OTHER TRADES. IN CASE OF CONFLICT, NOTIFY THE ENGINEER OF RECORD

C. THE STRUCTURE HAS BEEN DESIGNED TO RESIST DESIGN LOADS ONLY AS A COMPLETE STRUCTURE. APPLICATIONS OF CONSTRUCTION LOADS TO THE PARTIALLY COMPLETED STRUCTURE SHALL BE CONSIDERED AND SO INCLUDED IN THE DESIGN OF SHORING, BRACING, FORMWORK, AND STRUCTURE DURING ERECTION AND UNTIL ALL PERMANENT CONNECTIONS ARE MADE. TEMPORARY BRACING FOR THE STRUCTURE MUST BE PROVIDED IN ALL DIRECTIONS.

D. ONLY USE DIMENSIONS INDICATED ON THE DRAWINGS. DO NOT SCALE DRAWINGS.

E. ELEVATIONS INDICATED ON THE STRUCTURAL DRAWINGS ARE BASED ON A PROJECT DATUM INDICATED ON THE ARCHITECTURAL DRAWINGS.

F. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ARCHITECT/ENGINEER OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH WORK.

G. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETE DESIGN OF THE STRUCTURE. THEY DO NOT INDICATE THE MEANS AND METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKERS, OR THEIR PERSONS DURING CONSTRUCTION.

H. OBSERVATION VISITS TO THE SITE BY ENGINEER OF RECORD OR REPRESENTATIVES OF THE ENGINEER OF RECORD MAY BE MADE DURING CONSTRUCTION. ANY SUPPORT SERVICES PERFORMED HEREIN SHALL BE DISTINGUISHED FROM INSPECTION AND / OR TESTING SERVICES PERFORMED BY OTHERS, AND ARE NOT TO BE CONSTRUED AS SUPERVISION AND / OR MANAGEMENT OF CONSTRUCTION.

I. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL MEMBERS AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT / ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS WITHIN THE STRUCTURE.

J. CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED IN THE GENERAL NOTES. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.

K. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO THE DETAILS RESENTED,SIMILAR DETAILS SHALL BE USED SUBJECT TO THE REVIEW OF ENGINEER OF RECORD.

L. SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS, AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR. SUBSEQUENT DOCUMENTATION REQUESTS TO BUILDING ENGINEER OF RECORD FROM GENERAL CONTRACTOR SHALL INCLUDE EVALUATION OF DEVIATIONS BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.

DESIGN CRITERIA

A. FLORIDA BUILDING CODE 6TH EDITION 2017, AS ADOPTED AND SUPPLEMENTED BY LOCAL REGULATIONS

B. ASCE 7-10, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES

C. ACI 318-14, STRUCTURAL CONCRETE BUILDING CODE

D. AISC MANUAL OF STEEL CONSTRUCTION, 14TH EDITION

E. NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, 2012 (AISI S100-12)

F. NDS-15, NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION

G. SERVICEABILITY AND DEFLECTION CRITERIA PER GOVERNING COMPONENT DESIGN STANDARD

DESIGN LOADING

A. ROOF LIVE LOADS — 20 PSF

B. DEAD LOADS (D)
1. ROOF: — 16 PSF, INCLUDING STRUCTURE
2. SIGN: — 20 PSF, INCLUDING STRUCTURE
3. COLUMNS & TRUSSES — STEEL SELF WEIGHT (490 PCF)

C. UPLIFT LOAD COMBINATIONS (D)
1. ROOF: — 10 PSF, INCLUDING STRUCTURE
2. SIGN: — 10 PSF, INCLUDING STRUCTURE
3. COLUMNS & TRUSSES: — STEEL SELF WEIGHT (490 PCF)

D. WIND LOADING
1. WIND DESIGN: — (ASCE 7-10)
2. ULTIMATE DESIGN WIND SPEED, VULT: — 150 MPH
3. NOMINAL DESIGN WIND SPEED, VNOM: — 117 MPH
4. MIN SITE SPECIFIC WIND SPEED, VULT: — 141 MPH
5. RISK CATEGORY: — II
6. WIND EXPOSURE: — C
7. ENCLOSURE CLASSIFICATION: — OPEN
8. INTERNAL PRESSURE COEFF: +/- — +/- 0.0

E. DESIGN LOAD-BEARING VALUES OF SOIL — 2000 PSF

01003 SUBMITTALS

A. SHOP DRAWINGS / DATA TO BE SUBMITTED FOR APPROVAL.

1. STRUCTURAL STEEL SHOP DRAWINGS (INCLUDING STEEL JOISTS AND GIRDERS)
   a. ERECTION DRAWINGS
   b. PIECE DRAWINGS
   c. METAL DECK DRAWINGS
   d. PREP PROCEDURE AND PRIMING MATERIAL
   e. CURRENT WELDERS CERTIFICATIONS
2. CONCRETE
   a. MIX DESIGN
   b. MATERIAL CERTIFICATES
   c. HISTORICAL TESTING BACKGROUND
3. CONCRETE REINFORCING SHOP DRAWINGS
   a. PLACEMENT DRAWINGS
   b. FABRICATION AND BENDING DETAILS

B. SEE SECTION 01003 IN THE PROJECT MANUAL AND SPECIFICATIONS FOR THE LIST OF REQUIRED STRUCTURAL SUBMITTALS / SHOP DRAWINGS.

C. SEE SECTION 01002-14 "SHOP DRAWINGS SUBMITTAL" IN THE PROJECT MANUAL AND SPECIFICATIONS FOR SUBMITTAL RESPONSIBILITIES AND REQUIREMENTS.

D. SHOP DRAWING REVIEW IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT. CORRECTIONS OR COMMENTS MADE ON THIS REVIEW DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR ERRORS AND OMISSIONS, AND FROM COMPLIANCE WITH THE PLANS AND SPECIFICATIONS. CORRECTIONS OR COMMENTS DO NOT AUTHORIZE AN INCREASE IN THE CONSTRUCTION BUDGET.

E. APPROVAL OF SHOP DRAWINGS DOES NOT INDICATE ACCEPTANCE OF DEVIATIONS FROM CONTRACT DOCUMENTS OR PREVIOUS SHOP DRAWING REVIEW, UNLESS SPECIFICALLY NOTED THEREON BY ENGINEER OF RECORD.

F. ANY CHANGES TO THE DESIGN CONCEPT SHOWN IN CONTRACT DOCUMENTS SHALL BE SUBMITTED IN WRITING AND APPROVED BY THE ARCHITECT AND ENGINEER PRIOR TO SUBMITTING SHOP DRAWINGS. ALL SUCH CHANGES SHALL BE "BUBBLED" ON THE SHOP DRAWINGS AND REFERENCED TO THE PROPER RFI.

G. SUBMITTALS SHALL CONFORM TO THE REQUIREMENTS OF THE CONTRACT DRAWINGS (SEE ITEM F ABOVE FOR EXCEPTION). NON-CONFORMING OR NON-REVIEWED SUBMITTALS WILL BE RETURNED WITHOUT [illegible]

1. SHOP DRAWINGS SHALL BE "APPROVED", SIGNED AND DATED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO ENGINEER AND ARCHITECT OF RECORD.
2. SHOP DRAWINGS SHALL NOT CONTAIN REPRODUCTIONS OF THE CONTRACT DRAWINGS.

H. PROVIDE COPIES OF MANUFACTURERS LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT

02001 EARTHWORK / FOUNDATION

A. FOUNDATION DESIGN IS BASED UPON THE FOLLOWING:

NO REPORT PROVIDED:
PRIOR TO CONSTRUCTION, CONTRACTOR SHALL RETAIN THE SERVICES OF A QUALIFIED GEOTECHNICAL ENGINEER. THE GENERAL CONTRACTOR HEREIN IS RESPONSIBLE FOR PERFORMING ALL EARTHWORK OPERATIONS IN STRICT ACCORDANCE WITH GEOTECHNICAL ENGINEERING REQUIREMENTS.

IF THE FOUNDATION RECOMMENDATIONS AND / OR DESIGN VALUES DIFFER FROM THAT ASSUMED, MODIFICATIONS TO THE DRAWINGS MAY BE REQUIRED.

DO NOT PLACE CONCRETE PRIOR TO GEOTECHNICAL APPROVAL.

THE DESIGN ALLOWABLE SOIL BEARING PRESSURE IS LISTED IN THE DESIGN LOADING CRITERIA. NET ALLOWABLE SOIL PRESSURE ASSUMED TO BE AT 6'-0" DEPTH. SHALLOW FOUNDATION SYSTEM IS ASSUMED TO BE ADEQUATE. ESTIMATED SETTLEMENT ASSUMED TO BE NO GREATER THAN 1" OVERALL AND 1/2" DIFFERENTIAL SETTLEMENT. IT IS ASSUMED THAT ALL PROBLEMATIC SOILS (ORGANIC, EXPANSIVE, ETC.) WILL BE REMOVED AND REPLACED WITH SUITABLE STRUCTURAL FILL.

B. SEE SECTION 02001-04 IN THE PROJECT MANUAL AND SPECIFICATIONS FOR EARTHWORKS TO INCLUDE BUT NOT LIMITED TO:

1. PREPARING AND GRADING SUBGRADES FOR SLABS-ON-GRADE
2. EXCAVATING AND BACKFILLING FOR BUILDINGS AND STRUCTURES
3. DRAINAGE AND MOISTURE - CONTROL FILL COARSE FOR SLABS-ON-GRADE
4. EXCAVATING AND BACKFILLING TRENCHES WITHIN BUILDING LINES
5. MATERIAL, INSPECTION, AND TESTING REQUIREMENTS

C. ANY FILL REQUIRED TO BACKFILL EXCAVATED AREA OR ACHIEVE FINISHED GRADE IN STRUCTURAL AREAS SHALL BE AS INDICATED BY GEOTECHNICAL ENGINEER. THE FILL SHALL BE PLACED IN LEVEL LIFTS NOT EXCEED 12" LOOSE THICKNESS AND COMPACTED TO A MINIMUM OF 95% OF THE SOIL'S MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM SPECIFICATION D-1557.

D. IN-PLACE DENSITY TESTS SHALL BE PERFORMED BY AN EXPERIENCED ENGINEERING TECHNICIAN. TESTS SHALL BE PERFORMED FOR EACH 2,000 SQUARE FEET, IN EVERY COLUMN FOOTING LOCATION AND EACH 50'-0" ALONG WALL FOOTINGS. COPIES OF THE TEST REPORTS SHALL BE FURNISHED TO THE STRUCTURAL ENGINEER.

E. REMOVE FREE WATER FROM EXCAVATIONS BEFORE PLACING CONCRETE. WATER TABLE ASSUMED BELOW 6'-0" PRECLUDING BUOYANCY CONDITIONS ON FOOTINGS.

F. CAUTION SHOULD BE USED WHEN OPERATING VIBRATORY COMPACTING EQUIPMENT NEAR THE EXISTING STRUCTURE TO AVOID THE RISK OF DAMAGE TO THE STRUCTURE.

G. REFER TO ARCHITECTURE DRAWINGS FOR ANY NECESSARY WATERPROOFING REQUIREMENTS.

03301 CAST-IN-PLACE CONCRETE

A. SEE SECTION 03301 IN THE PROJECT MANUAL AND SPECIFICATIONS TO INCLUDE BUT NOT LIMITED TO:

1. GENERAL REQUIREMENTS
   a. SUBMITTALS
   b. QUALITY ASSURANCE / CODE REQUIREMENTS
2. PRODUCT / MATERIAL REQUIREMENTS
3. EXECUTION OF WORK REQUIREMENTS
   a. SHORING
   b. VAPOR RETARDER
   c. JOINTS
   d. PLACEMENT / FINISHING
   e. CURING
4. QUALITY CONTROL – TESTING REQUIREMENTS

B. SUMMARY OF PROJECT MANUAL AND SPECIFICATIONS SECTION 03301-02 PRODUCTS:

1. CONCRETE STRENGTH: — 4,000 PSI
2. STEEL REINFORCEMENT: — 60 KSI, (ASTM A615
3. PLAIN-STEEL WIRE FABRIC: — ASTM A185, FLAT SHEETS

C. LAP SPLICE REINFORCEMENT LENGTH

LAP SPLICES (INCHES)

REINF SIZE	4000 PSI CONCRETE STREGTH
#3	16
#4	20
#5	24
#6	30
#7	48
#8	60

REF: CRSI, CLASS B, FY=60 KSI, UNCOATED BARS, NORMAL WEIGHT CONC.

D. CONCRETE COVER FOR REINFORCEMENT:

1. CAST AGAINST AND PERMANENTLY IN CONTACT WITH THE GROUND: 3"
2. EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND: 2"
3. NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND: [illegible]"

E. THERE SHALL BE NO HORIZONTAL JOINTS IN ANY CONCRETE POURS UNLESS SHOWN ON THE STRUCTURAL DRAWINGS. THE ENGINEER OF RECORD SHALL APPROVE ALL DEVIATIONS OR ADDITIONAL JOINTS IN WRITING.

05120 STRUCTURAL STEEL

A. SEE SECTION 05120 IN THE PROJECT MANUAL AND SPECIFICATIONS TO INCLUDE BUT NOT LIMITED TO:

1. GENERAL REQUIREMENTS
   a. FABRICATOR REQUIREMENTS
   b. SUBMITTALS
   c. CODE REQUIREMENTS
2. PRODUCT / MATERIAL REQUIREMENTS
3. PAINTING REQUIREMENTS
4. EXECUTION OF WORK REQUIREMENTS
   a. FABRICATION
   b. ERECTION
5. QUALITY ASSURANCE REQUIREMENTS (INSPECTIONS & TESTING)

B. SUMMARY OF PROJECT MANUAL AND SPECIFICATIONS 05120 PRODUCTS:

1. MATERIALS:
   a. W SHAPES: — ASTM A992 (50 KSI)
   b. TUBE SHAPES: — ASTM A500 GR B (46 KSI)
   c. PIPE SHAPES: — ASTM A53, GR B
   d. CHANNELS AND ANGLES: — ASTM A36 (36 KSI)
   e. PLATES: — ASTM A36 (36 KSI)
   f. BOLTS: — ASTM A 325
   g. ANCHOR BOLTS / RODS: — ASTM F 1554, GRADE 55 (S1)
   h. ANCHOR BOLT NUT: — HEAVY HEX HEAD ASTM A563
   i. ANCHOR BOLT WASHERS: — ASTM F436
   j. ELECTRODES FOR WELDING: — AWS CODE E70XX

C. TEMPLATES SHALL BE PROVIDED FOR ALL ANCHOR BOLTS / RODS CAST IN CONCRETE.

D. GALVANIZE STEEL MEMBERS INDICATED ON PLANS:

1. GALVANIZE STEEL MEMBERS, FABRICATIONS AND ASSEMBLIES AFTER FABRICATION BY THE HOT-DIP PROCESS IN ACCORDANCE WITH ASTM A 123.
2. COATING REQUIREMENTS:
   a. CONFORM TO PARAGRAPH 6.1 OF ASTM A 123, TABLE 1 OF ASTM A 153, OR TABLE 2 OF ASTM A 767 AS APPROPRIATE.
   b. SURFACE FINISH: CONTINUOUS, ADHERENT, AS SMOOTH AND EVENLY DISTRIBUTED AS POSSIBLE AND FREE FROM ANY DEFECT DETRIMENTAL TO THE END USE OF THE COATED ARTICLE.
   c. ADHESION: WITHSTAND NORMAL HANDLING CONSISTENT WITH THE NATURE AND THICKNESS OF THE COATING AND NORMAL USE OF THE ARTICLE.
3. REPAIR OF DAMAGED COATING: REPAIR DAMAGED AREAS BY WELDING, FLAME CUTTING OR DURING HANDLING, TRANSPORT OR ERECTION BY ONE OF THE APPROVED METHODS IN ACCORDANCE WITH ASTM A 780 WHENEVER DAMAGE EXCEEDS 3/16" IN WIDTH. MINIMUM THICKNESS REQUIREMENTS FOR THE REPAIR ARE THOSE DESCRIBED IN ASTM A 123 SECTION 6.2, CURRENT EDITION.

E. CONNECTIONS, UNLESS OTHERWISE NOTED:

1. TIGHTEN BOLTS BY THE "SNUG-TIGHT" METHOD
2. FIELD CONNECTIONS SHALL BE MADE WITH 3/4" DIA. MINIMUM, HIGH-STRENGTH BEARING TYPE BOLTS (A325N) WITH THREADS ASSUMED TO BE INCLUDED IN SHEAR PLANES

F. GROUT UNDER BEARING PLATES SHALL BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 6000 PSI WHEN BEARING ON 3000 PSI CONCRETE AND 8000 PSI WHEN BEARING ON 4000 PSI CONCRETE.

G. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED OR OTHERWISE PROTECTED.

H. ALL STEEL EXPOSED TO SOIL SHALL BE ENCASED IN CONCRETE OR COATED PER ASPHALTIC BASED CORROSION NOTE.

09960 COATINGS FOR STEEL

A. SEE SPECIFICATIONS FOR GENERAL REQUIREMENTS, PRODUCTS, AND EXECUTION OF WORK.

B. ASPHALTIC BASED CORROSION RESISTANCE COATING SHALL BE MADE WITH AN ASPHALT-BASE EMULSION COATING SYSTEM PER ASTM D 1187.

C. APPLY A MINIMUM 1/16" THICK COATING IN TWO APPLICATIONS.

D. COATING TO BE APPLIED ON ALL SIDES OF MEMBERS – TO INCLUDE STEEL TO BE ADJACENT TO CAST-IN-PLACE CONCRETE.

E. COAT STRUCTURAL ANCHOR BOLTS, WELDS, AND ALL COMPONENTS IN THE AFFECTED AND DEFINED AREA.

F. ALL STEEL AND STEEL COMPONENTS (I.E., BASE PLATES AND ANCHOR BOLTS) EXPOSED TOP SOIL SHALL BE COATED FROM THE FOUNDATION (LOWEST LEVEL) UP TO 6" ABOVE THE SLAB OR FINAL GRADE – WHICHEVER IS GREATER.

STRUCTURAL INSPECTION

A. THE CONTRACTOR OR OWNER SHALL EMPLOY A QUALIFIED INDEPENDENT INSPECTION AGENCY. THIS AGENCY (REFERRED TO AS INSPECTOR HEREIN) SHALL BE RESPONSIBLE FOR VERIFYING THAT PROJECT STRUCTURAL WORK IS ACCOMPLISHED IN CONFORMANCE WITH THE CONTACT DOCUMENTS.

B. DURATION AND FREQUENCY OF JOB VISITS SHALL BE SUFFICIENT FOR THE INSPECTOR TO STATE AT THE COMPLETION OF THE PROJECT THAT THE STRUCTURAL WORK IS ACCOMPLISHED, AND ITS RELATED ELEMENTS HAVE BEEN ERECTED IN ACCORDANCE WITH THE CONTACT DOCUMENTS. SPECIFIC SYSTEMS TO BE INSPECTED INCLUDE:

FOUNDATION REINFORCING & CONCRETE
STRUCTURAL STEEL
PRE-ENGINEERED METAL PANEL DECKING
PRE-ENGINEERED LAM. PANEL DECKING

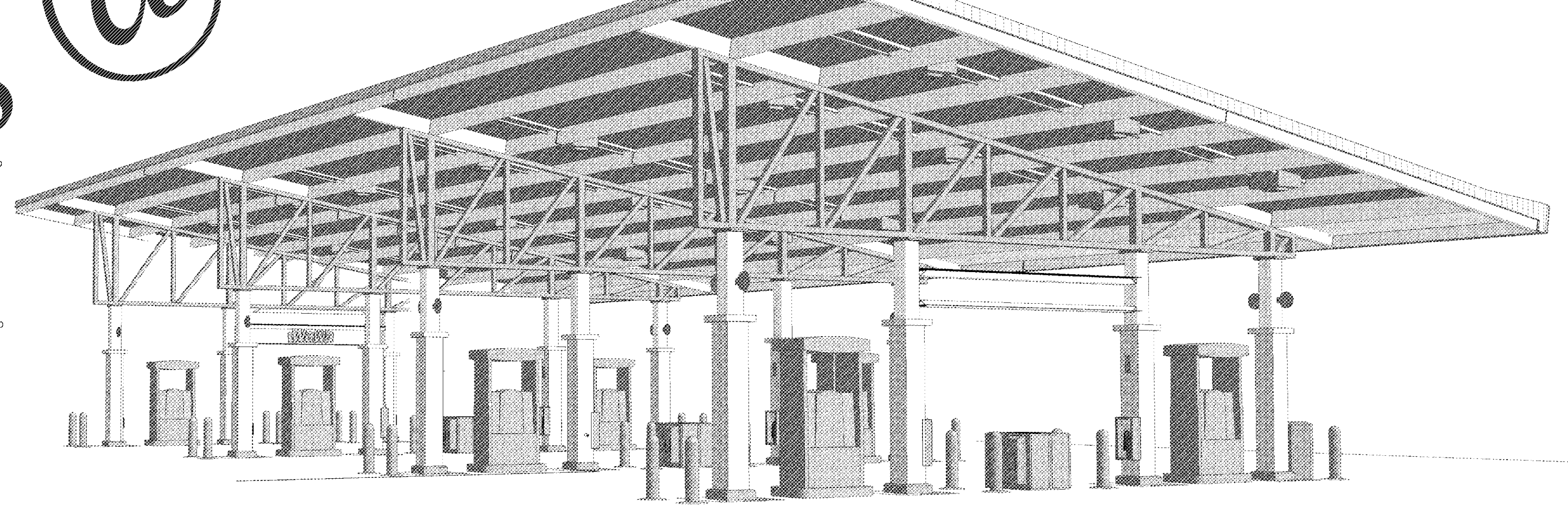
C. THE FOREGOING LIST IS NOT INTENDED TO BE ALL INCLUSIVE. THE INSPECTOR SHALL USE HIS PROFESSIONAL JUDGEMENT AND HIS KNOWLEDGE OF THE JOB SITE CONDITIONS AND THE OFFICIAL CONTRACT DOCUMENTS. THE INSPECTOR WILL NOT REPLACE THE QUALITY CONTROL PERSONNEL OF THE CONTRACTOR.

D. THE INSPECTOR DOES NOT RELIEVE THE CONTRACTORS CONTRACTUAL OR STATUTORY OBLIGATIONS. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE OFFICIAL CONTRACT DOCUMENTS. THE INSPECTOR WILL NOT REPLACE THE QUALITY CONTROL PERSONNEL OF THE CONTRACTOR.

E. ALL INSPECTION REPORTS SHALL BE FORWARD BY THE INSPECTOR TO THE ENGINEER AND ARCHITECT OF RECORD.

LIST OF STRUCTURAL ABBREVIATIONS

AB	ANCHOR BOLT	MATL	MATERIAL
ADDL	ADDITIONAL	MAX	MAXIMUM
AFF	ABOVE FINISHED FLOOR	MC	MOMENT CONNECTION
AHR	ANCHOR	MECH	MECHANICAL
ALT	ALTERNATE	MFR	MANUFACTURER
APPROX	APPROXIMATE	MFR REC	MANUFACTURER'S RECOMMENDATION
ARCH	ARCHITECT	MIN	MINIMUM
B PL	BASE PLATE	MISC	MISCELLANEOUS
BC	BOTTOM CHORD	MTL	METAL
BLDG	BUILDING	NIC	NOT IN CONTRACT
BM	BEAM	NOM	NOMINAL
BOS	BOTTOM OF STEEL	NS	NEAR SIDE
BOT	BOTTOM	NTS	NOT TO SCALE
BRDG	BRIDGING	OC	ON CENTER
BRG PL	BEARING PLATE	OPNG	OPENING
BRG	BEARING	OPP	OPPOSITE
BTWN	BETWEEN	PCC	PRECAST CONCRETE
CANTIL	CANTILEVER	PLYWD	PLYWOOD
CFMF	COLD FORMED METAL FRAMING	PREFAB	PREFABRICATE
CIP	CAST IN PLACE	PRELIM	PRELIMINARY
CJ	CONTROL JOINT	PSF	POUNDS PER SQUARE FOOT
CL	CENTERLINE	PSI	POUNDS PER SQUARE INCH
CLR	CLEAR	PT	PRESSURE TREATED
COL	COLUMN	PT	POST TENSIONED
CONC	CONCRETE	R	RADIUS
CONN	CONNECT	RCP	REINFORCED CONCRETE PIPE
CONST JT	CONSTRUCTION JT	RD	ROOF DRAIN
CONSTR	CONSTRUCTION	REBAR	REINFORCING STEEL BARS
CONT	CONTINUOUS	REF	REFERENCE
CMU	CONCRETE MASONRY UNIT	REINF	REINFORCE
CTR	CENTER	REQ	REQUIRED
DET	DETAIL	REV	REVISION
DIA	DIAMETER	RST	REINFORCING STEEL
DIM	DIMENSION	RTU	ROOF TOP UNIT
DIST	DISTANCE	SCHED	SCHEDULE
DL	DEVELOPMENT LENGTH	SE	STRUCTURAL ENGINEER
DR	DRAIN	SECT	SECTION
DWG	DRAWING	SIM	SIMILAR
EA	EACH	SPEL	SPOT ELEVATION
EE	EACH END	SPEC	SPECIFICATION
EF	EACH FACE	SQ	SQUARE
EJ	EXPANSION JOINT	SST	STAINLESS STEEL
EL	ELEVATION	STD	STANDARD
ENGR	ENGINEER	STL	STEEL
EQ	EQUAL	STRUCT	STRUCTURAL
EQUIP	EQUIPMENT	SYMM	SYMMETRICAL
EQUIV	EQUIVALENT	T&B	TOP AND BOTTOM
EXIST	EXISTING	THK	THICKNESS
EXP	EXPANSION	THRD	THREADED
EXT	EXTERIOR	TOC	TOP OF CONCRETE
EW	EACH WAY	TOF	TOP OF FLOOR / FOOTING
FD	FLOOR DRAIN	TOJ	TOP OF JOIST
FDTN	FOUNDATION	TOM	TOP OF MASONRY
FF EL	FINISHED FLOOR ELEVATION	TOS	TOP OF STEEL / SLAB
FIN	FINISH	TS	TUBE STEEL
FLR	FLOOR	TYP	TYPICAL
FS	FAR SIDE	UNO	UNLESS NOTED OTHERWISE
FTG	FOOTING	UON	UNLESS OTHERWISE NOTED
GA	GAGE / GAUGE	VERT	VERTICAL
GALV	GALVANIZED	VIF	VERIFY IN FIELD
GALV STL	GALVANIZED STEEL	WF	WIDE FLANGE
GC	GENERAL CONTRACTOR	W/	WITH
H	HIGH	W/O	WITHOUT
HDR	HEADER	WD	WOOD
HORIZ	HORIZONTAL	WT	WEIGHT
HS	HIGH STRENGTH	WH	WEEPHOLE
HT	HEIGHT	WP	WORKING POINT
INT	INTERIOR	WWF	WELDED WIRE FABRIC
JT	JOINT	XBRA	CROSSBRACING
K	KIPS	@	AT
KO	KNOCKOUT	&	AND
KSF	KIPS PER SQUARE FOOT		
KSI	KIPS PER SQUARE INCH		
L	ANGLE		
LBS	POUND		
LDH	LONG DIMENSION HORIZONTAL		
LDV	LONG DIMENSION VERTICAL		
LLH	LONG LEG HORIZONTAL		
LLV	LONG LEG VERTICAL		



1 CS1 3D View - Perspective

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WAWA, PENNSYLVANIA 19063

PROJECT NAME
WAWA STACKED 8 GAS CANOPY 2018.01 STORE #5275-LIT
9021 STATE ROAD 52
HUDSON, FL 34669

SHEET TITLE
DESIGN CRITERIA AND NOTES

Revision Schedule

No.	Description	Date
	PERMIT SET	03/21/2017
	PRE-BID REVIEW SET	08/29/2018
A	REV 2018.01	09/18/2018
	BID SET	09/18/2018

PROJECT NO. 2161082
DATE 03-21-2017
DRAWN RC
CHECKED JJ

CS1 (A)

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