

CAST-IN-PLACE CONCRETE:

TO BE MIXED AND PLACED IN ACCORDANCE WITH ACI 301, ACI 506 & 506.2. ALL REINFORCED CONCRETE TO HAVE 28 DAY COMPRESSIVE STRENGTHS AS FOLLOWS:

- FOUNDATIONS: $f_c = 3,000$ PSI
- SLAB ON GRADE: $f_c = 4,000$ PSI

ALL CONCRETE MIX DESIGN SUBMITTALS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

UNLESS NOTED OTHERWISE, NORMAL WEIGHT CONCRETE (145 PCF) SHALL BE USED WITH 3/4" MAX. (ELEVATED) OR 1" MAX. (SLAB-ON-GRADE) COARSE AGGREGATE CONFORMING TO ASTM C 33.

PROTECT FRESHLY POURED CONCRETE FROM PREMATURE DRYING AND EXCESSIVE COLD AND HOT TEMPERATURES. START CURING AS SOON AS FREE WATER HAS DISAPPEARED FROM THE CONCRETE SURFACE AFTER PLACING AND FINISHING. ALL CURING PROCEDURES TO FOLLOW ACI 308.

PROTECT CONCRETE FROM DAMAGE AND REDUCED STRENGTH CAUSED BY FROST, FREEZING ACTIONS AND LOW TEMPERATURES IN COMPLIANCE WITH ACI 306.

PROTECT CONCRETE FROM DAMAGE AND REDUCED STRENGTH CAUSED BY HIGH TEMPERATURES IN COMPLIANCE WITH ACI 305. UNIFORMLY COOL WATER AND AGGREGATES BEFORE MIXING TO OBTAIN A CONCRETE MIXTURE TEMPERATURE OF NOT GREATER THAN 90 DEGREES FAHRENHEIT AT POINT OF PLACEMENT.

WHERE NEW CONCRETE IS TO BE POURED ONTO EXISTING CONCRETE, ROUGHEN AND CLEAN SURFACE OF THE ADJOINING AREA AND COAT WITH SIKADUR 32 HI-MOD OR AN APPROVED BONDING AGENT.

NO ADDITIONAL WATER SHALL BE ADDED TO THE CONCRETE AT THE JOB SITE.

SEE ARCHITECTURAL, ELECTRICAL, MECHANICAL, FIRE PROTECTION AND PLUMBING DRAWINGS FOR DRIPS, CHAMFERS, REGLETS, SLOTS, SLEEVES, RUSTICATIONS, INSERTS ANCHORS AND OTHER EMBEDDED ITEMS NOT NOTED ON STRUCTURAL DRAWINGS. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING & PLACING ALL EMBEDDED ITEMS SHOWN ON DRAWINGS & ADDITIONAL ITEMS NOTED IN THIS NOTE, AS REQUIRED BY OTHER TRADES.

ALL OPENINGS REQUIRED BY OTHER TRADES ARE TO BE COORDINATED W/ARCH. & MECH. DWGS., AND ARE SUBJECT TO STRUCTURAL ENGINEERING APPROVAL.

ALL OPENINGS IN CONCRETE SLABS SHALL BE LOCATED, SIZED AND REINFORCED (WITH THE EXCEPTION OF SMALL OPENINGS AND/OR SLEEVES OF A SIZE THAT WILL NOT DISPLACE OR INTERRUPT THE CONTINUITY OF THE REINFORCING) AS SHOWN ON RESPECTIVE FLOOR PLANS AND DETAILS. ALL OPENINGS IN SLAB LESS THAN 10" SHALL BE COORDINATED WITH ARCHITECTURAL & MEP DRAWINGS AND ARE SUBJECT TO STRUCTURAL ENGINEER'S APPROVAL. ANY ALTERATIONS REQUIRE APPROVAL OF THE STRUCTURAL ENGINEER. (SEE TYPICAL SLAB OPENING DETAIL).

DEFECTIVE AREA IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.01 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE STRUCTURAL ENGINEER.

CONCRETE REINFORCING:

REINFORCING STEEL AND ACCESSORIES SHALL BE DETAILED IN ACCORDANCE WITH ACI 315-99 (MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES) AND CRSI MSP2-01 (MANUAL OF STANDARD PRACTICE). (IBC 2009; CRSI MANUAL OF STANDARD PRACTICE, 28TH EDITION)

REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 (UNLESS NOTED OTHERWISE).

UNLESS NOTED OTHERWISE, CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

1. CONCRETE CAST AGAINST EARTH ----- 3"
2. FORMED CONCRETE EXPOSED TO EARTH OR WEATHER ---- 2"
3. WALLS ----- 3/4"
4. COLUMNS/BEAMS ----- 1 1/2"
5. CONVENTIONAL REINFORCING ELEVATED SLABS ----- 1"

SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS AND DETAILS IS NOT ACCEPTABLE.

ALL WELDED REINFORCING STEEL SHALL CONFORM TO ASTM A-706, GRADE 60, AND BE USED ONLY WITH PRIOR PERMISSION FROM THE STRUCTURAL ENGINEER.

ALL TENSION SPLICES, INCLUDING SPLICES FROM BARS LABELED CONTINUOUS, SHALL CONFORM TO ACI 318-05. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI 318, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. (FBC 2010; ACI 318-08)

WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A-185 AND BE LAPPED TWO FULL PANELS, TIED ON EACH SIDE AND SHALL BE SUPPLIED IN FLAT SHEETS.

LONGITUDINAL REINFORCING BARS IN FOOTINGS SHALL BE PLACED CONTINUOUS AT CORNERS AND INTERSECTIONS.

FOR EVERY VERTICAL OR HORIZONTAL BAR DISCONTINUED BY AN OPENING, ONE BAR (MIN. OF 2 BARS) SHALL BE ADDED @ SIDE OF OPENING (HALF TO EACH SIDE - TYPICAL).

PROVIDE CORNER BARS AT ALL CONTINUOUS REINFORCING BARS AT ALL MEMBERS (FOUNDATIONS, WALLS, SLABS, BEAMS AND OTHER MEMBERS).

PROVIDE DOWELS FROM FOUNDATIONS, THE SAME SIZE & NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING UNLESS NOTED OTHERWISE.

ALL DOWELS AND TERMINATING BARS SHALL HAVE A STANDARD 90 DEGREE HOOK.

ALL HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL JOINTS FOR CONSTRUCTION JOINTS AND AROUND CORNERS.

CONCRETE SLABS ON GRADE:

SHALL BE INSTALLED OVER MINIMUM 15 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL SOIL TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES.

SLAB CONSTRUCTION JOINTS SHALL BE USED IN PLACE OF CONTROL JOINTS WHERE NEEDED TO INTERRUPT A CONTINUOUS POUR. SLAB CONSTRUCTION JOINTS SHALL BE KEPT CLEAN.

REFER TO ARCHITECTURAL/MECHANICAL FOR SLAB FINISHES, DEPRESSIONS, THICKENED SLABS (IN ADDITION TO THICKENED SLABS NOTED ON STRUCTURAL DRAWINGS), ELEVATIONS, AND ENCASED OR EMBEDDED ITEMS.

PLUMBING AND ELECTRICAL CONDUITS SHALL BE PLACED BELOW THE SLAB AND NOT WITHIN THE SLAB. VERTICAL PENETRATIONS ARE ALLOWED.

COLUMN BOXES SHALL BE USED TO ISOLATE AN ADEQUATE AREA AROUND COLUMN BASE PLATES TO PROVIDE FOR COLUMN PLACEMENT AND LEVELING. BOXES ARE TO BE CLEAN AND FREE OF DEBRIS TO TOP OF FOOTING PRIOR TO FILLING WITH CONCRETE. COLUMN BOX-CUTS ARE NOT REQUIRED IF STEEL COLUMNS ARE PLUMB AND FULLY GROUTED PRIOR TO PLACEMENT OF SLAB.

C.J. NOTES CONCRETE SLAB "CONTROL JOINT" WHICH SHALL BE CUT INTO THE SLABS AT A DEPTH OF 1/4 TIMES THE THICKNESS OF THE SLAB WITHIN 12 HOURS OF PLACING THE CONCRETE. MAXIMUM SPACING OF INTERIOR SLAB CONTROL JOINTS, UNLESS NOTED OTHERWISE, SHALL BE 16'-0" (MAX.) IN EACH DIRECTION.

PLACEMENT OF WELDED WIRE REINFORCEMENT IN SLAB, WHERE SPECIFIED, SHALL BE AT CONSISTENT DEPTH OF 1"-2" FROM T/SLAB. WELDED WIRE REINFORCEMENT SHALL BE PROPERLY CHAIRED ABOVE GRADE. OVERLAP EACH REINFORCING SHEET TWO FULL PANELS AND TIE CROSS WIRES ON EACH SIDE. WELDED WIRE REINFORCEMENT SHALL BE SUPPLIED IN FLAT SHEETS.

CONCRETE TESTING:

OWNER WILL EMPLOY AN INDEPENDENT TESTING LABORATORY TO PERFORM THE FOLLOWING TESTS AND SUBMIT TEST REPORTS ON CAST-IN-PLACE CONCRETE:

•ASTM C143 "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." SLUMP SHALL NOT EXCEED LIMIT INDICATED ON APPROVED MIX DESIGN, OR 6" (WHICHEVER IS SMALLER)

•ASTM C39 "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." CYLINDERS SHALL BE TAKEN FOR EACH MIX DESIGN USED, AND FOR EVERY 50 CUBIC YARDS OF CONCRETE PLACED. TEST CYLINDERS AT THE FOLLOWING AGES:

CYLINDER AT 3 DAYS, 1 CYLINDER AT 7 DAYS, 2 CYLINDERS AT 28 DAYS --OR-- HOLD ONE RESERVE CYLINDER TO BE TESTED AS REQUESTED BY THE ENGINEER. IF REQUIRED 28 DAY STRENGTH IS ACHIEVED, THE RESERVE CYLINDER MAY BE DISCARDED.

ALL CONCRETE TESTS INCLUDING AIR CONTENT, SLUMP, AND TEST CYLINDERS SHALL BE TAKEN AT THE POINT OF DISCHARGE AND FROM THE DISCHARGE END OF PUMP HOSE WHEN CONCRETE IS PUMPED.

POST-INSTALLED ANCHORS IN CONCRETE:

EXPANSION ANCHORS AS SHOWN ON CONTRACT DOCUMENTS SHALL BE HILTI KWIK BOLT 3 ANCHORS MANUFACTURED BY HILTI FASTENING SYSTEMS, WEDGE-ALL ANCHORS MANUFACTURED BY SIMPSON STRONGTIE COMPANY, POWER-STUD ANCHORS MANUFACTURED BY POWERS FASTENERS, TRUBOLT WEDGE ANCHORS MANUFACTURED BY ITW RAMSET/RED HEAD OR APPROVED EQUAL. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE PER MANUFACTURER'S REQUIREMENTS UNLESS NOTED OTHERWISE.

ADHESIVE ANCHORS AS SHOWN ON CONTRACT DOCUMENTS SHALL CONSIST OF AN ALL-THREAD GRADE 36 STEEL ANCHOR WITH HY150 MAX INJECTION ADHESIVE (HIT HY20 INJECTION ADHESIVE FOR MASONRY CONSTRUCTION WITH VOIDS) SUPPLIED BY HILTI FASTENING SYSTEMS, EPOXY-TIE SET EPOXY ADHESIVE SUPPLIED BY SIMPSON STRONGTIE COMPANY, EPCON G5 EPOXY ADHESIVE SUPPLIED BY ITW RAMSET/RED HEAD OR APPROVED EQUAL. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE PER MANUFACTURER'S REQUIREMENTS UNLESS NOTED OTHERWISE.

POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REBAR. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS. ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACING INDICATED IN THE MANUFACTURER'S LITERATURE. UNLESS SPECIFIED OTHERWISE, ANCHORS SHALL BE EMBEDDED IN THE APPROPRIATE SUBSTRATE WITH A MINIMUM EMBEDMENT OF 8 TIMES THE NOMINAL ANCHOR DIAMETER OR THE EMBEDMENT REQUIRED TO SUPPORT THE INTENDED LOAD. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED, SHALL BE SUBMITTED TO THE ENGINEER WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE BUILDING CODE.

ADHESIVE FOR REINFORCING DOWELS IN EXISTING CONCRETE SHALL BE EITHER THE HIT HY150 MAX INJECTION ADHESIVE SUPPLIED BY HILTI FASTENING SYSTEMS, EPOXY-TIE SET EPOXY ADHESIVE SUPPLIED BY SIMPSON STRONGTIE COMPANY, POWER-FAST EPOXY INJECTION GEL SUPPLIED BY POWERS FASTENERS, EPCON G5 EPOXY ADHESIVE SUPPLIED BY ITW RAMSET/RED HEAD OR APPROVED EQUAL. MIN. EMBEDMENT LENGTH SHALL BE 12 BAR DIAMETERS, UNLESS NOTED OTHERWISE.

STEEL JOIST:

ALL OPEN WEB JOIST AND JOIST GIRDERS SHALL BE FABRICATED, FURNISHED, AND ERECTED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE'S STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS, LATEST EDITION.

THE DESIGN OF ALL STEEL JOIST AND JOIST GIRDERS SHALL BE BY THE MANUFACTURER'S ENGINEER. ALL JOIST AND JOIST GIRDERS ALONG WITH THE BRIDGING SHALL BE DESIGNED FOR THE GRAVITY LOADS OF THE PROJECT IN ADDITION TO THE WIND LOADS SHOWN ON THE STRUCTURAL DRAWINGS AND THE MECHANICAL LOADS SHOWN ON THE MECHANICAL DRAWINGS.

K-SERIES JOISTS SHALL BEAR A MINIMUM OF 2 1/2" OVER STEEL SUPPORTS. K-SERIES JOISTS AND JOIST GIRDERS SHALL BEAR A MINIMUM OF 4" OVER STEEL SUPPORTS. BEARING PLATES AND ANGLES SHALL BE ADJUSTED TO PROVIDE LEVEL BEARING ON SUPPORT SURFACE.

EXTEND BOTTOM CHORDS OF JOISTS AND JOIST GIRDERS AT COLUMNS.

SHOP DRAWINGS SHALL BE SUBMITTED COMPLETELY DETAILING THE JOISTS FOR ERECTION. INCLUDE ALL BRIDGING AND CONNECTIONS, CALCULATIONS AND SHOP DRAWINGS SHALL BE SIGNED, SEALED, AND DATED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.

PROVIDE A SHOP COAT OF STANDARD PRIMER. PAINT COLOR TO BE SPECIFIED BY THE ARCHITECT OR OWNER. PRIMER TO BE COMPATIBLE WITH FINISH COAT WHERE APPLICABLE. TOUCH UP DAMAGED AREAS WITH SAME PAINT USED FOR SHOP COAT.

METAL DECKING:

STEEL FLOOR DECK SHALL BE INTERMEDIATE RIB CONFIGURATION, DESIGNED IN ACCORDANCE WITH LATEST EDITION OF STEEL DECK INSTITUTE SPECIFICATIONS (SEE SPECIFICATIONS)

STEEL FLOOR DECK SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST EDITION STEEL DECK MANUAL AS ADOPTED BY THE STEEL DECK INSTITUTE AS WELL AS ALL SDI SPECIFICATION AND RECOMMENDATIONS.

EXTEND FLOOR AND ROOF METAL DECK PAST CENTER LINE OF SUPPORTING MEMBERS TO EDGE OF POUR STOPS. TYPICAL AT ALL OPENINGS AND SPANDREL CONDITIONS. THE GAUGE OF THE METAL FLOOR DECK SHOWN IS THE MINIMUM ALLOWED. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING AN UNSHORED DECK SYSTEM OR PROVIDING DECK SHORING AS REQUIRED TO AVOID OVER STRESSING THE DECK DURING PLACEMENT OF CONCRETE. FLOOR DECK SHALL HAVE 3 SPANS UNLESS INDICATED ON SHOP DRAWINGS AND APPROVED BY THE ENGINEER OF RECORD.

ROOF EDGE ANGLE SHALL RUN CONTINUOUS. INSTALL ROOF EDGE ANGLE CONTINUOUS BY WELDING OR BOLTING EDGE ANGLES TOGETHER. WELD ANGLE TO EACH JOIST OR BEAM. ALL ROOF DECK SHALL BE WELDED TO ALL EDGE ANGLES, EDGE BEAMS AND FRAMED OPENING SUPPORTS WITH SAME ATTACHMENT PATTERN AS TYPICAL SUPPORT BEAMS, AS CALLED OUT IN THE ROOF DECK ATTACHMENT DETAIL.

SUBMIT DECK SHOP DRAWING INDICATING DECK MANUFACTURER, ALL STRUCTURAL PROPERTIES, FASTENER TYPE AND SPACING. STEEL ROOF DECK SHALL BE APPROVED BY FACTORY MUTUAL AND SHALL BE INSTALLED IN ACCORDANCE WITH THE F.M. REQUIREMENTS.

STRUCTURAL STEEL:

MATERIALS:

STRUCTURAL STEEL	A572 or A992 (50 KSI)
CHANNELS, ANGLES, PLATES	A36 (36 KSI)
RECTANGULAR STEEL TUBES	A500 Gr. B. (46 KSI)
PIPES	A53 Gr. B
HIGH STRENGTH BOLTS	A325N
ANCHOR BOLTS	A307 or ASTM F1554 (36 KSI)
WELDING ELECTRODES	E70XX

DETAIL, FABRICATE, AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.

ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE AISC SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS - ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN, 14TH EDITION.

WELDING FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS CODE D1.1 AND VISUALLY CONFORM TO AWS SECTION 6 AND TABLE 6.1. FABRICATION/ERECTION INSPECTIONS BY THE CONTRACTOR PER AWS SECTION 6, SHALL BE BY ASSOCIATE/CERTIFIED INSPECTORS (ACI/CW) PER AWS QC1 OR AWS B5.1. VERIFICATION INSPECTIONS SHALL BE BY A CERTIFIED WELDING INSPECTOR (WI) OR SENIOR WELDING INSPECTOR (SWI) PER AWS B5.1.

WELDS SHALL BE MADE ONLY BY OPERATORS EXPERIENCED IN PERFORMING THE TYPE OF WORK INDICATED. WELDING SHALL BE DONE IN ACCORDANCE WITH APPROPRIATE WELD PROCEDURE SPECIFICATIONS (WPS). WELDERS SHALL BE FAMILIAR WITH THE APPLICABLE WPS'S. WELDS NORMALLY EXPOSED TO VIEW IN THE FINISHED WORK SHALL BE UNIFORMLY MADE AND GROUND SMOOTH. WHERE WELDING IS DONE IN PROXIMITY TO GLASS OR FINISHED SURFACES, SUCH SURFACES SHALL BE PROTECTED FROM DAMAGE DUE TO WELD SPARKS, SPATTER OR TRAMP METAL.

WELDER QUALIFICATIONS AND (WPS) SHALL BE MAINTAINED AT THE JOB SITE THROUGHOUT WORK AND SHALL BE READILY AVAILABLE FOR INSPECTION UPON REQUEST, BOTH IN THE SHOP AND IN THE FIELD.

A CERTIFIED WELD INSPECTOR (CWI) SHALL INSPECT AND DOCUMENT COMPLIANCE WITH THE FOLLOWING:

- CONFIRM WELDER QUALIFICATIONS PRIOR TO START OF WORK.
- REVIEW ALL (WPS) PRIOR TO START OF WORK.
- CONFIRM MATERIALS AND FABRICATIONS CONFORM TO SPECIFICATIONS.
- OBSERVE JOINT PREPARATION, FIT UP AND WELDER TECHNIQUE.
- IDENTIFY ALL MULTI-PASS FILLET WELDS, SINGLE PASS FILLET WELDS GREATER THAN 5/16" AND COMPLETE / PARTIAL JOINT PENETRATION GROOVES WELDED BUTT JOINTS THAT REQUIRED CONTINUOUS INSPECTION.
- QUALITY INSPECT ALL WELDS PER AWS SECTION 6.5 AND TABLE 6.1

ALL WELDS SHALL BE 1/8" MINIMUM FILLET WELDS U.O.N. OR AS REQUIRED BY DESIGN.

STRUCTURAL GROUT SHALL BE SHRINKAGE RESISTANT NON-EXPANSIVE, NON-METALLIC GROUT WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 5000PSI WHEN TESTED IN ACCORDANCE WITH ASTM C109. FORMS SHALL BE PLACED AROUND BASE PLATE AND THE STRUCTURAL GROUT SHALL BE POURED. NO DRY-DAMP PACKING.

PRIME: ALL STEEL SURFACES, BOTH EXPOSED AND CONCEALED ARE TO RECEIVE ONE SHOP COAT OF PAINT AND FIELD SPOT PRIME ALL WELDS, SCARS, CONN., ETC., WHERE PAINT IS DAMAGED. ALL GALVANIZED STEEL SHALL HAVE ALL WELDS, SCARS, ETC., RE-GALVANIZED IF DAMAGED.

DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER. STRUCTURAL ENGINEER SHALL BE COMPENSATED BY THE CONTRACTOR FOR THE COST INVOLVED IN THE REDESIGN OF CONNECTIONS FOR THE CONVENIENCE OF THE CONTRACTOR.



10.01.18
Christopher J Sabourin PE
FL PE#71461

PLAN NAME	BRANAN FIELD WALK
DATE	10.01.18
SSE No.	RW11-18-0133

ISSUE	DATE
PERMIT	10.01.18
REVISIONS	DATE

STRUCTURAL ENGINEERING FOR
BUILDING 1
BRANAN FIELD WALK
MIDDLEBERG, FL

FIELD ALTERATION
CONTRACTOR SHALL CONTACT CHRISTOPHER SABOURIN PE PRIOR TO MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINAL CONTRACTOR DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER SABOURIN MAY RESULT IN ADDITIONAL ENGINEERING OR INSPECTION FEES.

SCALING
DO NOT SCALE DIMENSIONS FROM THESE DRAWINGS. IF A DIMENSION IS UNCLEAR REFER TO THE ARCHITECTURAL DRAWINGS OR CONTACT THE EDR.

GENERAL NOTES

SHEET
SO.1
SHEET 2 OF 15