GENERAL STRUCTURAL NOTES

A. CONTRACTOR NOTES

1. THE CONTRACT DRAWINGS REPRESENT THE DESIRED RESULT OF CONSTRUCTION. THE METHODS OF CONSTRUCTION & THE RISKS INVOLVED DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL MAINTAIN THE STRUCTURAL INTEGRITY OF THE BUILDING AT ALL STAGES OF CONSTRUCTION.
2. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS & ELEVATIONS PRIOR TO CONSTRUCTION AND SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT/ENGINEER BEFORE COMMENCING ANY WORK.
3. THE CONTRACT'S PROPOSED SUBSTITUTIONS SHALL BE APPROVED BY THE ARCHITECT/ENGINEER PRIOR TO COMMENCING ANY PERTINENT WORK.
4. CONTRACTOR & ALL SUBCONTRACTORS SHALL COORDINATE ALL DRAWINGS DURING BIDDING & SHALL REPORT ANY DISCREPANCIES. IF DISCREPANCIES ARE FOUND DURING CONSTRUCTION THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL NECESSARY MEANS (MATERIAL & LABOR) TO RESOLVE THE DISCREPANCIES.

B. DESIGN CRITERIA

THE NEW PARAPET AND TOWER STRUCTURE HAS BEEN CHECKED ACCORDING TO THE 2012 EDITION OF THE INTERNATIONAL BUILDING CODE AND FOR SPECIFIC LOADS AS LISTED BELOW ON CONTRACT DOCUMENT:

1. NEW ROOF LOADS
   a) D.L. = 10 P.S.F.
   b) L.L. = 20 P.S.F.
   c) ROOF SNOW LOAD
      i. GROUND SNOW LOAD Pg = 10 P.S.F.
      ii. FLAT ROOF SNOW LOAD Pf = 10 P.S.F.
      iii. SNOW EXPOSURE FACTOR Ce = 1.0
      iv. SNOW IMPORTANCE FACTOR Is = 1.0
      v. THERMAL FACTOR Ct = 1.0
2. NEW WALL LOADS
   a) D.L. = 10 P.S.F.
3. LATERAL LOADS (WIND)
   a) ULTIMATE DESIGN WIND SPEED, V-ULT = 115 MPH
   b) NOMINAL DESIGN WIND SPEED V-ASD = 90 MPH
   c) WIND EXPOSURE = C
   d) RISK CATEGORY = II
   e) THE APPLICATE INTERNAL PRESSURE COEFFICIENT = +/- 0.18
   f) COMPONENTS AND CLADDING DESIGN LOADS, SEE COMPONENTS AND CLADDING EXTERNAL PRESSURE LOADS SCHEDULE ON THIS SHEET.

C. STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A36, Fy = 36 K.S.I., UNLESS NOTED OTHERWISE.
2. ALL WELDS SHALL BE MADE WITH E70XX RODS AND WELDING SHALL CONFORM TO THE LATEST AWS CODE.

D. FRAMING LUMBER (AND PLYWOOD)

1. ALL LUMBER SHALL BE GRADED IN ACCORDANCE WITH NFPA STANDARDS.
   a) 2X12 STUD SHALL BE SOUTHERN PINE, GRADE NO. 2 AT 19% MOISTURE CONTENT WITH ALLOWABLE SINGLE MEMBER BENDING STRESS, Fb= OF 750 P.S.I. AND A MODULUS OF ELASTICITY, E= OF 1,400,000 P.S.I.
   b) 2X10 RAFTERS SHALL BE SOUTHERN PINE, GRADE NO. 2 AT 19% MOISTURE CONTENT WITH ALLOWABLE SINGLE MEMBER BENDING STRESS, Fb OF 800 P.S.I. AND A MODULUS OF ELASTICITY, E, OF 1,400,000 P.S.I.
   c) GLUED-LUMINATED MEMBERS SHALL BE DOUGLAS FIR INDUSTRIAL GRADE (24F-V8) OR APPROVED EQUAL U.N.O., MEMBER SIZES ARE BASED ON CURRENT NDS WESTERN SPECIES. GRADE 24F-V4 MAY BE SUBSTITUTED FOR SIMPLY SUPPORTED SPANS ONLY
2. WOOD CONNECTORS SHOWN ON THE DRAWINGS SHALL BE SIMPSON STRONG-TIE CONNECTORS AS MANUFACTURED BY THE SIMPSON CO. OR APPROVED EQUAL.
   a) ALL JOIST HANGERS SHALL BE TYPE U AND ALL WIND ANCHORS SHALL BE TYPE H2.5 OR H3.
   b) USE LARGEST SIZE HANGER OR ANCHORS AS POSSIBLE WITH MAXIMUM NUMBER OF NAILS.
3. PROVIDE X-BRACING OR SOLID BLOCKING AT END SPAN OF JOIST/RAFTER AND EQUALLY SPACED AT 4'-0" O.C., MAXIMUM ALONG THE JOIST/RAFTER.
4. ALL PLYWOOD SHALL BE STRESS RATED AT (32/16) EXTERIOR GRADE, (C-DX), WITH THICKNESS AS INDICATED ON THE DRAWINGS.
5. PLYWOOD ON ROOF SHALL BE PLACED WITH FACE GRAIN ACROSS TRUSSES AND STAGGERED SO CONTINUOUS PANEL JOINTS OCCUR ONLY IN ONE DIRECTION, PERPENDICULAR TO THE SPAN OF THE TRUSSES. NAIL PLYWOOD WITH 10d COMMON NAILS. NAILS SHALL BE SPACED AT 6" O.C. AT SUPPORTED EDGES OF PANEL AND AT 12" O.C. AT INTERMEDIATE SUPPORT MEMBERS, UNLESS NOTED OTHERWISE.
6. PLYWOOD SHALL BE NAILED TO STUD WITH 8d COMMON NAIL @ 6" O.C. @ PANEL EDGES (BLOCK ALL EDGES) & 12" O.C. @ ALL INTERMEDIATE SUPPORTS, UNLESS NOTED OTHERWISE

E. CAST IN PLACE CONCRETE

1. ALL CONCRETE SHALL DEVELOP 4,000 P.S.I. COMPRESSIVE STRENGTH IN 28 DAYS.
2. REINFORCING BARS SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A615, Fy = 60 K.S.I. REINFORCING BARS INDICATED TO BE WELDED SHALL CONFORM TO ASTM A706. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
3. SPLICES IN CONTINUOUS VERTICAL OR HORIZONTAL REINFORCING BARS SHALL BE PER LATEST ACI 318-14 OR (40) BAR DIAMETERS LAP SPLICE, WHICHEVER IS GREATER, UNLESS NOTED OTHERWISE AND SHALL BE EITHER CONTINUOUS OR SPLICED WITH DOWELS AT CORNERS.
4. CLEARANCES BETWEEN REINFORCING BARS AND CONCRETE SURFACES SHALL BE AS FOLLOWS:

	COVER (INCHES)
a) CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3
b) CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THROUGH #18 BARS	2
#5 BAR AND SMALLER	1 1/2
c) CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS: #11 BAR AND SMALLER	3/4
BEAMS, COLUMNS; PRIMARY REINFORCEMENT, TIES AND STIRRUPS	1 1/2

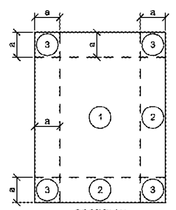
F. FOUNDATION, FILLING AND EXCAVATION

1. SOIL BEARING CAPACITY = [illegible] P.S.F. (PER EXISTING DRAWING)
2. FOR INFORMATION ON FILLING, EXCAVATION, BACKFILLING AND SPECIAL SOIL INFORMATION SEE REPORT OF GEOTECHNICAL EXPLORATION DATED JAN. [illegible], 1998 & DEC. 16, 1988 AS PREPARED BY GILES ENGINEERING ASSOCIATES INC. OF WAUKSHA, WI PROJECT NO. 190112 & 881[illegible]

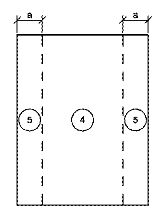
COMPONENTS AND CLADDING ULTIMATE WIND LOAD DESIGN EXTERNAL PRESSURE SCHED. (P.S.F.)

Effective Wind Area (SF.)	INTERNATIONAL BUILDING CODE 2012 Location per ASCE 7-10				
	1	2	3	4	5
10-20	17.7 -28.1	17.7 -48.9	17.7 -72.2	30.7 -33.3	30.7 -41.2
20-50	16.1 -27.3	16.1 -45.0	16.1 -67.6	29.3 -31.9	29.3 -38.3
50-100	16.0 -26.3	16.0 -39.9	16.0 -61.4	27.5 -30.1	27.5 -34.6

1. a = NA [0.1 X (THE LEAST HORIZONTAL DIMENSION) OR 0.4 x h (h = MEAN ROOF HEIGHT) WHICHEVER IS SMALLER.]
2. POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING, NEGATIVE PRESSURE VALUES REFER TO FORCES ACTING AWAY FROM BUILDING.
3. EACH COMPONENT MUST BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE FORCES.
4. FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES, DESIGN LOADS MAY BE INTERPOLATED. OTHERWISE DESIGN LOAD MUST BE TAKEN FROM THE NEXT LOWEST EFFECTIVE AREA.



ROOF PLAN



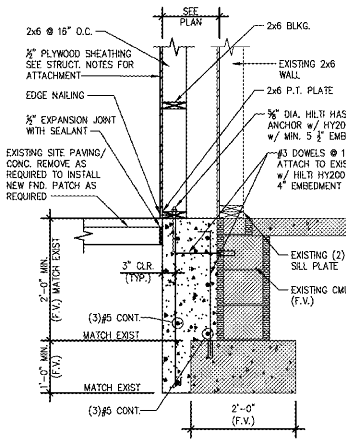
WALL ELEVATION

SYMBOL LEGEND

SYMBOL	DESCRIPTION
1 / S-401	CALLOUT TAG
SIM. 1 / S-401	DETAIL SECTION TAG
0	COLUMN GRID DESIGNATION FOR NEW CONSTRUCTION
0	COLUMN GRID DESIGNATION FOR EXISTING CONSTRUCTION
	MOMENT CONNECTION
℄	CENTERLINE SYMBOL
PL	PLATE SYMBOL
FS1.0	COLUMN FOOTING TAG
R1	ROOF FRAMING NOTE TAG
F1	FLOOR FRAMING NOTE TAG
BP1	BEARING PLATE TAG
RT1	ROOF TRUSS PROFILE TAG
	SPAN DIRECTION - 9/16" METAL FLOOR DECK
	SPAN DIRECTION - 1 1/2" METAL ROOF DECK
	SPAN DIRECTION - 5/8" PLYWOOD ROOF DECK
R1	SPAN DIRECTION - SPECIAL ROOF DECK
	SPAN DIRECTION - SPECIAL FLOOR DECK
	TOP OF FOOTING ELEV. TAG
A	SHEAR WALL TAG

ABBREVIATION

(E)	EXISTING	FTG.	FOOTING
A.B.	ANCHOR BOLT	G.L.B.	GLUELAM BEAM
A.F.F.	ABOVE FINISHED FLOOR	GA.	GAUGE
ANCH.	ANCHOR	GB	GRADE BEAM
ARCH.	ARCHITECT/ARCHITECTURAL	H.C.	HOLLOWCORE
B.F.E.	BOTTOM OF FOOTING ELEV.	HORIZ.	HORIZONTAL
B.L.E.	BRICK LEDGE ELEVATION	J.B.E.	JOIST BEARING ELEVATION
BLDG.	BUILDING	JT.	JOINT
BLKG.	BLOCKING	L.G.M.F.	LIGHT GAUGE METAL FRMG.
BM.	BEAM	L.L.	LIVE LOAD
BOT.	BOTTOM	LLH	LONG LEG HORIZONTAL
BRNG.	BEARING	LLV	LONG LEG VERTICAL
B.S.E.	BRICK/BLOCK SEAT ELEV.	M.O.	MASONRY OPENING
C.M.U.	CONCRETE MASONRY UNIT	MAX.	MAXIMUM
CB.	CONCRETE BLOCK	MFG.	MANUFACTURER
CJ	CONTROL JOINT/ CONSTRUCTION JOINT	MIN.	MINIMUM
		MTL.	METAL
℄	CENTER LINE	O.C.	ON CENTER
COL.	COLUMN	OPNG.	OPENING
CONC.	CONCRETE	PL	PLATE
CONT.	CONTINUOUS	P.L.F.	POUNDS PER LINEAL FOOT
CRW	CONCRETE RETAINING WALL	P.S.F.	POUNDS PER SQUARE FOOT
CTRD.	CENTER	P.S.I.	POUNDS PER SQUARE INCH
CWP	CONCRETE WALL PANEL	P.T.	PRESSURE TREATED
D.L.	DEAD LOAD	R	ROOF FRAMING MARK
DBL.	DOUBLE	REINF.	REINFORCED
DIA.	DIAMETER	REQ'D.	REQUIRED
DP	DRILLED PIER (CAISSON)	RET.	RETAINING
DWG.	DRAWING(S)	S.Y.P.	SOUTHERN YELLOW PINE
E.F.	EACH FACE	SCHED.	SCHEDULE
E.J.	EXPANSION JOINT	SECT.	SECTION
E.N.	EDGE NAILING	SIM.	SIMILAR
E.W.	EACH WAY	STL.	STEEL
EA.	EACH	STIFF.	STIFFENER
ELEV.	ELEVATION	STRUCT.	STRUCTURAL
EXIST.	EXISTING	T.B.E.	TRUSS BEARING ELEVATION
EXP.	EXPANSION	T.F.E.	TOP OF FOOTING ELEVATION
EXT.	EXTERIOR	T.L.	TOTAL LOAD
F	FLOOR FRAMING MARK	T.S.E.	TOP OF STEEL ELEVATION
F.F.E.	FINISHED FLOOR ELEVATION	T.W.E.	TOP OF WALL ELEVATION
F.N.	FINISH NAILING	TYP.	TYPICAL
F.V.	FIELD VERIFY	U.N.O.	UNLESS NOTED OTHERWISE
FDN.	FOUNDATION	VERT.	VERTICAL
FR	FOOTING RECTANGLE	WB	WOOD BEAM
FRMG.	FRAMING	w/	WITH
FS	FOOTING SQUARE	WWF	WELDED WIRE FABRIC



SECTION 1 / S1.1
SCALE: 3/4" = 1'-0"

hmdGROUP PA architects
10556 n.w. 26 street suite d-101 doral, fl 33172
phone 305-594-3329 email hmdgroup@bellsouth.net
WILFREDO PEREZ FLORIDA LIC. NO. 13418 LICENSE # AA F000074
ARCHITECTURE INTERIOR DESIGN CORPORATE PLANNING

ARCHITECT'S PROJECT #: 17-0341

MOHAMMAD R. SERAJI P.E.
432 SOUTH BROADWAY RD. LEXINGTON, KY 40508
PCSCE #18-061

Olive Garden ITALIAN KITCHEN

Issue Date: 04-12-2018

REVISION INFORMATION
1
2
3
4
5
6
7
8

Restaurant #: 1173

P1-B (HIGH)

1711 GALLATIN RD. NORTH

MADISON, TN

STRUCTURAL NOTES AND FOUNDATION DETAILS

S1.1