GENERAL NOTES

GENERAL INFORMATION:

1. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED STATE. TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED TO STABILIZE THE STRUCTURE DURING ERECTION UNDER ALL LOADING CONDITIONS SHALL BE DESIGNED, FURNISHED AND INSTALLED BY THE CONTRACTOR.
2. THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE IBC 2012 STANDARD BUILDING CODE AND LATEST STATE OF GEORGIA AMENDMENTS.
3. DESIGN LOADS ARE AS FOLLOWS:
   ROOF LOADS:
   DEAD LOAD: STRUCTURE WEIGHT + 10 PSF (MECH., ELECT., CEILING)
   LIVE LOAD: ROOF 20 PSF
   WIND LOADS:
   ULTIMATE DESIGN WIND SPEED 120 MPH
   RISK CATEGORY IV
   EXPOSURE CATEGORY B
4. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS FOR INSERTS, SLEEVES, CURBS, PADS, ETC., AFFECTING STRUCTURAL WORK NOT SHOWN. SEE TYPICAL DETAILS FOR FRAMING AROUND MISC. OPENINGS.
5. CONTRACTOR TO LOCATE ALL BURIED UTILITY PIPES PRIOR TO THE EXCAVATION FOR THE BUILDING FOUNDATIONS. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED OF POTENTIAL CONFLICTS BETWEEN THE FOUNDATIONS AND THE BURIED UTILITY PIPES.
6. CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS PRIOR TO ORDERING ANY MATERIALS, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER WHERE CONDITIONS VARY FROM WHAT IS SHOWN ON THE DRAWINGS.
7. REFER TO ARCHITECTURAL DRAWINGS FOR STAIRS RISER AND THREAD DIMENSIONS.
8. METAL BUILDING SHALL BE DESIGNED BY A REGISTERED ENGINEER IN THE STATE IF GEORGIA. SUBMIT CALCULATIONS FOR REVIEW.

REINFORCED CONCRETE:

1. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 301, SPECIFICATIONS FOR STRUCTURAL CONCRETE, AS WELL AS TO ACI 318.
2. SHOP DRAWINGS, SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING OF THE REINFORCING STEEL AND ACCESSORIES SHALL BE SUBMITTED FOR REVIEW TO THE ARCHITECT/ENGINEER. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.
3. UNLESS NOTED OTHERWISE, DETAILS OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 315, MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES, AND CRSI MSP-1, MANUAL OF STANDARD PRACTICE, LATEST EDITIONS.
4. UNLESS NOTED OTHERWISE, 28 DAY COMPRESSIVE STRENGTH OF CAST-IN-PLACE CONCRETE FOR EACH PORTION OF STRUCTURE SHALL BE AS FOLLOWS:
   FOOTINGS: 3,000 PSI
   ALL OTHER CONCRETE: 4,000 PSI
5. UNLESS NOTED OTHERWISE, MINIMUM CONCRETE COVER FOR REINFORCEMENT SHALL BE AS FOLLOWS:
   CONCRETE CAST AGAINST EARTH 3"
   FORMED CONCRETE EXPOSED TO EARTH OR WEATHER 2"
   INTERIOR SLABS, WALLS 1"
6. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. ALL WELDED REINFORCING STEEL SHALL BE ASTM A706.
7. ALL SPLICES SHALL CONFORM TO ACI 318, CLASS "B" SPLICE UNLESS NOTED OTHERWISE.
8. LONGITUDINAL REINFORCING IN FOOTINGS SHALL BE PLACED CONTINUOUS AT CORNERS AND INTERSECTIONS.
9. CONSTRUCTION JOINTS SHALL FALL AT CONTROL JOINTS AND SHALL BE KEYED.
10. UNLESS NOTED OTHERWISE, CONCRETE SLAB ON GRADE SHALL BE 4" THICK, W/6X6-W2.1XW2.1 WWF CAST ON 6 MIL POLYETHYLENE VAPOR BARRIER ON 4" COMPACTED #57 STONE.
11. WELDING OR TACK WELDING OF REINFORCING STEEL SHALL NOT BE PERMITTED EXCEPT AS AUTHORIZED OR DIRECTED BY THE STRUCTURAL ENGINEER OR HIS REPRESENTATIVE.
12. UNLESS NOTED OTHERWISE, ALL 8" CONCRETE WALLS TO BE REINFORCED WITH #5@12" E.W. ALL 12" CONCRETE WALLS TO BE REINFORCED WITH #4@12" E.W./E.F.

FOUNDATIONS:

1. FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING CAPACITY OF 2400 PSF.
2. ALL FOUNDATION WORK SHALL BE INSPECTED AND APPROVED FOR REQUIRED SOIL BEARING CAPACITY BY A COMPETENT SOILS ENGINEER, REGISTERED IN GEORGIA, OR HIS REPRESENTATIVE PRIOR TO PLACEMENT OF CONCRETE.
3. AREAS UNDER SLABS AND FOOTINGS SHALL BE BACKFILLED AND COMPACTED IN LIFTS OF NOT MORE THAN EIGHT INCHES UNCOMPACTED DEPTH (FOUR INCHES UNCOMPACTED DEPTH IF HAND OPERATED EQUIPMENT IS USED) TO A DENSITY OF 95% OF THE MAXIMUM DRY DENSITY AS DEFINED BY ASTM D698. IN EXCAVATED AREAS WHICH ARE TO SUPPORT SLABS AND PAVEMENTS, THE UPPER TWELVE INCHES OF SOILS SHALL BE SCARIFIED AND RECOMPACTED TO 98% OF THE MAXIMUM DRY DENSITY.
4. THE FOUNDATION DESIGN IS BASED ON GEODERM ENGINEERS & SCIENTISTS, INT. INC. REPORT DATED MARCH 2, 2028.

STRUCTURAL STEEL:

1. ALL STRUCTURAL STEEL CONSTRUCTION SHALL CONFORM TO: "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" OF THE AISC, LATEST EDITION ADOPTED BY IBC 2000.
2. ALL FABRICATIONS SHALL COMPLY WITH "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", LATEST EDITION, AS PUBLISHED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
3. SHOP DRAWINGS PREPARED IN ACCORDANCE WITH THE LATEST "STRUCTURAL STEEL DETAILING MANUAL OF THE AISC SHALL BE STAMPED BY ENGINEER REGISTERED IN GEORGIA, AND SUBMITTED TO THE ENGINEER FOR REVIEW. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.
4. UNLESS NOTED OTHERWISE, STRUCTURAL STEEL ROLLED SECTIONS SHALL CONFORM TO ASTM A992. STEEL PIPE AND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500 GRADE B. CHANNELS, PLATES, ANGLES AND BARS SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.
5. BEAMS WITHOUT SPECIFIED CAMBER ARE TO BE FABRICATED SUCH THAT AFTER ERECTION ANY CAMBER DUE TO ROLLING OR SHOP FABRICATION IS UPWARD AT MID SPAN.
6. STEEL FRAMING CONNECTIONS SHALL BE SHOP WELDED AND FIELD BOLTED U.N.O..
   A. BOLTS SHALL CONFORM TO ASTM A325 OR A490 AND SHALL BE MINIMUM 3/4" DIA. UNLESS NOTED OTHERWISE. BOLTED CONNECTIONS SHALL BE ASSEMBLED AND TESTED ACCORDING TO "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR ASTM A490 BOLTS".
   B. WELDS SHALL CONFORM TO "STRUCTURAL WELDING CODE" OF THE AMERICAN WELDING SOCIETY, AWS D1.1 USE E70XX ELECTRODES. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE. MINIMUM WELD SIZE IS ACCORDING TO AISC LATEST EDITION ADOPTED BY IBC 2000.
   C. GRIND ALL EXPOSED WELDS SMOOTH TO MATCH AND BLEND WITH ADJOINING SURFACES.
   D. ONE SIDED CONNECTION WILL NOT BE PERMITTED.
   E. UNLESS NOTED OTHERWISE, CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF ONE HALF THE SHEAR CAPACITY OF THE MEMBER.
7. THE FABRICATOR SHALL BE RESPONSIBLE FOR THE SELECTION, DESIGN AND DETAILING OF ALL CONNECTIONS NOT FULLY DETAILED ON THE CONTRACT DRAWINGS. WHERE BEAM REACTIONS ARE NOT SHOWN ON THE DRAWINGS, CONNECTIONS SHALL BE DESIGNED AND DETAILED FOR THE MAXIMUM UNIFORM LOAD WHICH THE BEAM WILL SUPPORT (AS A SIMPLE BEAM) FOR THE SPAN SHOWN ON THE DRAWINGS, OR 12.0 KIPS MINIMUM.
8. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL MISCELLANEOUS STRUCTURAL STEEL NOT SHOWN ON STRUCTURAL DRAWINGS.
9. ALL POST-INSTALLED ANCHORS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS, HILTI ANCHORING SYSTEM OR APPROVED EQUAL.
10. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 AND SHALL BE GALVANIZED IF EXPOSED IN FINISHED STRUCTURE.

STEEL JOISTS:

1. STEEL JOISTS SHALL CONFORM TO "STANDARD SPECIFICATIONS AND LOAD TABLES" OF THE STEEL JOIST INSTITUTE, LATEST EDITION.
2. FABRICATE THE JOIST SEAT TO MATCH THE SLOPE OF ROOF FRAMING AS REQUIRED.
3. SIZE AND SPACING OF BRIDGING SHALL MEET THE REQUIREMENTS OF THE LATEST EDITION OF THE "STANDARD LOAD TABLES" OF THE STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE.
4. JOIST BRIDGING SHALL NOT BE USED TO SUPPORT PIPING, CONDUIT, DUCTWORK ETC.
5. NET WIND UPLIFT AT EDGE IS 20 PSF.
   NET WIND UPLIFT AT CENTER IS 15 PSF.

METAL DECK

1. METAL DECK SHALL CONFORM TO STEEL DECK INSTITUTE (SDI) SPECIFICATIONS, LATEST EDITION.
2. METAL DECK SHALL BE AS SHOWN ON DRAWINGS. SEE METAL DECK PROPERTIES ON SHEET S1.1
3. DECKING SHALL BE CONTINUOUS OVER 3 SPANS MINIMUM WHERE SUPPORTING STRUCTURE PERMITS. DECK TO SPAN NO LESS THAN TWO SPANS PER PANEL.
4. DECKING SHALL BE ERECTED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS AND STEEL DECK INSTITUTE SPECIFICATIONS.
5. ROOF AND FLOOR METAL DECK SHALL BE WELDED TO SUPPORTING MEMBERS AT MAXIMUM 12" O.C. WITH 5/8" DIA. SPOT WELDS.
6. LAP SPLICES SHALL BE MINIMUM (3) #10 TEK SCREWS.
7. NET WIND UPLIFT AT EDGE IS 22 PSF.
   NET WIND UPLIFT AT CENTER IS 17 PSF.

MASONRY:

1. MATERIALS, TESTING OF MATERIALS AND STORAGE OF MATERIALS SHALL CONFORM TO ACI STANDARD "SPECIFICATIONS FOR MASONRY STRUCTURES AND BUILDING CODE REQUIREMENT FOR MASONRY STRUCTURES" (ACI 530 AND 530.1).
2. REQUIRED 28 DAY COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS, f'm, SHALL BE 1,500 PSI MINIMUM GROUT STRENGTH SHALL BE 2500 PSI.
3. MORTAR AND JOINTS SHALL NOT EXCEED 5/8" THICKNESS IN CONCEALED AREAS AND WITHIN SPECIFIED TOLERANCES IN AREAS EXPOSED TO VIEW.
4. PIPES OR CONDUITS MAY PENETRATE HORIZONTALLY THROUGH MASONRY WALLS BY MEANS OF A GALVANIZED STEEL SLEEVE NO THINNER THAN STANDARD WEIGHT (SCHEDULE 40), ASTM A53, SOLIDLY GROUTED OR MORTARED IN PLACE. PLACE SLEEVES NOT CLOSER THAN THREE DIAMETERS ON CENTER. MAXIMUM SIZE OF SLEEVE SHALL BE 12".
5. MAXIMUM SIZE OF EMBEDDED VERTICAL PIPE OR CONDUIT IN A BEARING WALL SHALL NOT EXCEED 1/3 OF WALL THICKNESS. PIPES SHALL NOT BE PLACED CLOSER THAN THREE DIAMETERS ON CENTER.
6. ALL CONCRETE BLOCK WALLS SHALL HAVE MINIMUM VERTICAL REINFORCING OF #5@32" O.C. UNLESS NOTED OTHERWISE.
7. HORIZONTAL JOINT REINFORCEMENT SHALL BE 9 GAGE LADDER TYPE AND SPACED 16" O.C. UNLESS NOTED OTHERWISE.
8. REINFORCEMENT LAP SPLICE LENGTH SHALL BE 48 BAR DIAMETERS.
9. HORIZONTAL JOINT REINFORCEMENT SHALL NOT EXTEND THROUGH CONTROL JOINTS IN THE MASONRY.
9. JOINT REINFORCEMENT SHALL BE PLACED CONTINUOUSLY WITH THE ENDS LAPPED 6" FOR DEFORMED BARS AND 12" FOR SMOOTH BARS.
10. JOINT REINFORCEMENT SHALL BE FULLY EMBEDDED IN MORTAR WITH MINIMUM COVER OF 5/8".
11. FOUNDATIONS WHERE VERTICAL AND HORIZONTAL ALIGNMENT WOULD CAUSE MORTAR JOINTS TO BE LESS THAN 1/4" OR MORE THAN 5/8" THICK SHALL BE CORRECTED PRIOR TO COMMENCING MASONRY CONSTRUCTION.
12. IN LAYING THE FIRST COURSE, A FULL MORTAR BED SHALL BE PLACED ON THE FOUNDATION FOR THE TOTAL THICKNESS OF THE WALL EXCEPT AT CELLS TO BE GROUTED.
13. UNITS WHERE VERTICAL REINFORCING IS REQUIRED SHALL HAVE VERTICAL CORES TO ALIGN EVENLY FOR THE FULL HEIGHT OF THE CELL TO BE GROUTED.
14. GROUT IN VERTICAL CORES SHALL NOT BE PLACED IN LIFTS GREATER THAN 5 FEET.
15. VERTICAL CORES TO BE GROUTED SHALL HAVE A MINIMUM CLEAR DIMENSION OF 2" AND A CLEAR AREA OF [illegible] SQUARE INCHES.
16. MASONRY WALLS MUST BE BRACED DURING ERECTION FOR WIND AND CONSTRUCTION LOADS. BRACES MUST BE DESIGNED, CONSTRUCTED AND ERECTED IN A FASHION AS TO PREVENT PERMANENT SCARRING OF MASONRY SURFACES AT EXPOSED CONDITION.
17. FILL ALL REINFORCED CELLS WITH GROUT FULL HEIGHT. ALL CELLS LOCATED BELOW GRADE SHALL BE FILLED WITH GROUT. REINFORCE CELLS ADJACENT TO DOOR, WINDOW AND OPENINGS IN ACCORDANCE WITH CMU OPENING REINF. SCHEDULE.
18. IN EXISTING CMU FILL CELLS WITH GROUT AT ANCHOR LOCATIONS.
19. CONCRETE MASONRY UNITS SHALL BE GRADE N, TYPE 1 CONFORMING TO ASTM C90, f'm=1500 PSI.

ABBREVIATIONS

AB	ANCHOR BOLT
A.F.F.	ABOVE FINISHED FLOOR
ACI	AMERICAN CONCRETE INSTITUTE
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ANCH	ANCHOR, ANCHORAGE
A.B.	ANCHOR BOLTS
ARCH.	ARCHITECT, ARCHITECTURAL
B. PL.	BASE PLATE
BRG	BEARING
BETWN.	BETWEEN
BOT.	BOTTOM
BLDG.	BUILDING
CTR	CENTER
℄	CENTER LINE
CLR	CLEAR
COL.(S)	COLUMN(S)
CONC.	CONCRETE
CMU	CONCRETE MASONRY UNIT
CRSI	CONCRETE REINFORCING STEEL INSTITUE
CONN.	CONNECT, CONNECTION
CONSTR.	CONSTRUCTION
CJ	CONTROL JOINT
CONT.	CONTINUE, CONTINUOUS
CONTR	CONTRACTOR
D.L.	DEAD LOAD
DET.	DETAIL
DIAG.	DIAGONAL
DIA.	DIAMETER
DIM.	DIMENSION
DO.	DITTO
DWL	DOWEL
DWG.	DRAWING
EA.	EACH
E.F.	EACH FACE
E.W.	EACH WAY
E.O.D.	EDGE OF DECK
E.O.S.	EDGE OF SLAB
ELEC	ELECTRICAL
EL.	ELEVATION (HEIGHT LEVEL)
ELEV.	ELEVATOR
EMBED	EMBEDMENT
EQ.	EQUAL
EQPT.	EQUIPMENT
EXIST.	EXISTING
EXP.	EXPANSION
E.J.	EXPANSION JOINT
EXT.	EXTERIOR
FT.	FEET, FOOT
FIN.	FINISH, FINISHED
FIN. FLR	FINISH FLOOR
FLR.	FLOOR
F.D.	FLOOR DRAIN
FT'G	FOOTING
FD'N	FOUNDATION
FRMG	FRAMING
GA.	GAUGE
GALV.	GALVANIZED
GR.	GRADE
HVAC	HEATING, VENTILATING & AIR CONDITIONING
H.P.	HIGH POINT
HORIZ.	HORIZONTAL, HORIZONTALLY
IN	INCH
INCL	INCLUDE
JT	JOINT
JST	JOIST
L.L.	LIVE LOAD
L.L.H.	LONG LEG HORIZONTAL
L.L.V.	LONG LEG VERTICAL
MAX.	MAXIMUM
MECH.	MECHANICAL
MFR.	MANUFACTURER
MIN.	MINIMUM
MISC.	MISCELLANEOUS
NO. OR #	NUMBER
NTS	NOT TO SCALE
O.C.	ON CENTER
OPNG.	OPENING
OPP. HD.	OPPOSITE HAND
PL.	PLATE
PROJ.	PROJECT, PROJECTION
P.S.F.	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
SCHED.	SCHEDULE
SECT.	SECTION
SHT.	SHEET
SLH	SHORT LEG HORIZONTAL
SLV	SHORT LEG VERTICAL
SIM.	SIMILAR
S.O.G.	SLAB ON GRADE
SPCS	SPACES
SPEC	SPECIFICATION
STD.	STANDARD
STL	STEEL
SDI	STEEL DECK INSTITUTE
STIFF.	STIFFENER
STIR.	STIRRUP
STRUCT.	STRUCTURE, STRUCTURAL
SYMM.	SYMMETRICAL
TEMP	TEMPORARY, TEMPERATURE
THK.	THICK,THICKNESS
T&B	TOP AND BOTTOM
T.O.J.	TOP OF JOISTS
T.O.S.	TOP OF STEEL
T.O.W.	TOP OF WALL
TYP.	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
W.W.F.	WELDED WIRE FABRIC
WL	WIND LOAD
W.D.	WOOD
W/	WITH
W/O	WITHOUT

Rev	Date	Description

CLAYTON COUNTY
NEW FIRE STATION #9
4176 OLD DIXIE HIGHWAY
ATLANTA, GEORGIA 30354

CLAYTON COUNTY
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GEORGIA REGISTERED PROFESSIONAL ENGINEER No. 16028 VICTOR L. WOOD 7/26/18

100%
CONSTRUCTION PLANS
RELEASED FOR CONSTRUCTION

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PROJECT NO.: 2017-81
DRAWN BY: HB
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GENERAL NOTES

S-0.1

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