

STRUCTURAL NOTES

MISCELLANEOUS

- 1. THE STRUCTURAL SYSTEM IS UNSTABLE UNTIL ALL CONNECTIONS HAVE BEEN MADE AND ALL CONCRETE HAS REACHED ITS MINIMUM DESIGN STRENGTH, AS SHOWN IN THE STRUCTURAL DOCUMENTS.
2. CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION TO ENSURE THE SAFETY OF THE BUILDING UNTIL STRUCTURAL SYSTEM IS COMPLETED. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF WHATEVER TEMPORARY BRACING, SHORING, GUYS OR TIE-DOWNS THAT MAY BE NECESSARY. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.
3. APPLICABLE BUILDING CODE: FLORIDA BUILDING CODE 6th EDITION (2017) - BUILDING
4. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE.
5. DESIGN LOADS (U.N.D.): AREA SUPERIMPOSED LIVE LOAD 30 PSF (20 PSF MAX. FOR WIND DESIGN) AREA DEAD LOAD 20 PSF DESIGN WIND VELOCITY = 129 MPH (ULTIMATE) 108 MPH (ASD)
7. COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
8. CONTACT ENGINEER WITH ANY QUESTIONS OR DISCREPANCIES FOUND ON DRAWINGS.
9. SUBMIT SHOP DRAWINGS AS REQUIRED HEREIN. ALLOW FOR TWO WEEKS REVIEW TIME AFTER RECEIPT OF SUBMITTALS BY THIS FIRM. ALL SUBMITTALS SHALL BE CHECKED AND SIGNED BY THE GENERAL CONTRACTOR AND SIGNED/SEALED BY THE SPECIALTY ENGINEER, WHERE SPECIFIED HEREIN.
10. CONTRACTOR SHALL NOT BE RELIEVED FROM RESPONSIBILITY FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS OR MIX DESIGNS BY THE ENGINEER'S REVIEW THEREOF.

SITE WORK

- 1. A SUBSURFACE INVESTIGATION HAS BEEN COMPLETED AT THE PROJECT SITE BY UNIVERSAL ENGINEERING SCIENCES. SOIL BORING LOGS AND SITE PREPARATION PROCEDURES ARE LISTED IN THE PROJECT SOILS REPORT, DATED JUNE 15, 2018, WHICH IS AN INTEGRAL PART OF THESE CONTRACT DOCUMENTS.
2. ALL SITE WORK SHALL BE DONE IN STRICT ACCORDANCE WITH THE PROJECT SOILS REPORT.
3. DESIGN SOIL BEARING CAPACITY = 2500 PSF.
4. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING TESTS: a. ONE DENSITY TEST FOR EACH 2,000 SQUARE FEET OF COMPACTED SUBGRADE AND COMPACTED FILL. b. ONE DENSITY TEST AT EACH COLUMN FOOTING. c. ONE DENSITY TEST PER 50 FEET OF WALL FOOTING.
5. ONE COPY OF ALL TEST REPORTS SHALL BE SENT DIRECTLY TO OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR.
6. THE SIDES OF FOOTINGS MAY BE EARTH-FORMED IF THE EXCAVATION CAN BE KEPT VERTICAL, CLEAN, AND STABLE; OTHERWISE, PLYWOOD FORMS MUST BE USED.
7. FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACKFILLING PRESSURES UNTIL FLOOR SLABS AT TOP AND BOTTOM ARE IN PLACE.

CAST IN PLACE CONCRETE

- 1. CONCRETE TO BE NORMAL WEIGHT WITH THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS: a. FOOTINGS, SLAB-ON-GRADE.....3000 PSI
2. CONCRETE SHALL BE READY-MIX PER ASTM C94: a. PORTLAND CEMENT - ASTM C 150 b. AGGREGATES - ASTM C 33 (1" MAX. NOMINAL) c. NO CALCIUM CHLORIDE d. AIR ENTRAINING - ASTM C 660 e. WATER REDUCING - ASTM C 494 f. FLYASH - ASTM C 681 CLASS F (COOK MAXIMUM BY WEIGHT) g. WATER - CLEAN AND POTABLE
3. REINFORCING STEEL: ASTM A615 GRADE 60.
4. REQUIRED SLUMP RANGE = 3" TO 5".
5. WELDED WIRE FABRIC: ASTM A-185. FURNISH IN SHEETS, NOT ROLLS.
6. MOISTURE BARRIER: 10 MIL POLYETHYLENE. LAP 6" AND TAPE ALL JOINTS.
7. CODES AND STANDARDS: CURRENT EDITION: a. ACI 301 "SPEC FOR STRUCTURAL CONCRETE FOR BUILDINGS." b. ACI 308 "RECOMMENDED PRACTICE FOR HOT WEATHER CONCRETE." c. ACI 318 "BULC. CODE REQUIREMENTS FOR REINFORCED CONCRETE." d. ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT."
8. MINIMUM LAP SPlice = 30 BAR DIAMETERS UNLESS NOTED OTHERWISE.
9. PROVIDE PROPERLY TIED SPACERS, CHAIRS, BOLSTERS, ETC. AS REQUIRED AND NECESSARY TO ASSEMBLE, PLACE AND SUPPORT ALL REINFORCING IN PLACE. USE WIRE BAR TYPE SUPPORTS COMPLYING WITH COST RECOMMENDATIONS. USE PLASTIC TIE LEGS ON ALL EXPOSED SURFACES.
10. ALL SLABS SHALL BE REINFORCED MONOLITHICALLY, EXCEPT FOR REQUIRED CONSTRUCTION JOINTS. PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL.
11. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, AND SLAB RECESSES AS REQUIRED BY OTHER TRADES BEFORE CONCRETE IS PLACED.
12. CONTRACTOR SHALL VERIFY EMBEDDED ITEMS, INCLUDING BUT NOT LIMITED TO ANCHOR BOLTS, BOLT CLUSTERS, WELD PLATES, ETC., BEFORE PLACING CONCRETE. NOTIFY ENGINEER OF ANY CONFLICTS WITH REBAR.
13. SEE ARCHITECTURAL DRAWINGS FOR REQUIRED CONCRETE FINISHES.
14. ALL CONCRETE SHALL BE CURED IMMEDIATELY AFTER FINISHING OPERATIONS IN ACCORDANCE WITH ONE OF THE FOLLOWING METHODS: a. APPLY A LIQUID MEMBRANE FORMING CHEMICAL CURING COMPOUND IN ACCORDANCE WITH ASTM C-309. b. PROVIDE CONTINUOUS MOISTURE TO CONCRETE IN ACCORDANCE WITH ACI 301.
15. GENERAL CONTRACTOR IS RESPONSIBLE FOR THE PROPER DESIGN AND CONSTRUCTION OF ALL FORMWORK AND SHORING. DESIGN SHALL BE PERFORMED BY A LICENSED FLORIDA ENGINEER.
16. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING CONCRETE TESTS ON SITE: a. CYLINDER STRENGTH TESTS - ASTM C39; ONE SET OF FOUR CYLINDERS FOR EACH 500 SQUARE YARDS OR FRACTION THEREOF. TEST ONE CYLINDER AT 7 DAYS AND TWO AT 28 DAYS. THE FINAL CYLINDER IN RESERVE. b. SLUMP TESTS - ASTM C143
17. ONE COPY OF ALL TEST REPORTS SHALL BE SENT DIRECTLY TO OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR.
18. RESTRICT THE ADDITION OF ANY WATER AT THE JOB SITE. DO NOT ADD WATER WITHOUT THE APPROVAL OF THE GENERAL CONTRACTOR AND ENGINEER. IF SLUMP IS EXCESSIVE, ADD ALLOWABLE WATER TO GEMENT. DO NOT USE WELL WATER FROM THE TANK AND REMIX TO ACHIEVE CONSISTENCY. ALL TESTS SHALL BE PERFORMED IN THE PRESENCE OF THE ENGINEER AT THE JOB SITE. ALL TESTS SHALL BE PERFORMED IN THE PRESENCE OF WATER TO THE MIX.
19. REINFORCING BAR COUPLERS UNLESS NOTED OTHERWISE: a. EPOXY b. 3/4" CIRCULAR 1-1/2" DIA
20. CONCRETE SHALL BE PLACED WITHIN 90 MINUTES OF BATCH TIME.
21. ALL DIMENSIONS ARE GIVEN ON DRAWINGS. LENGTH OF HOOK, IF REQUIRED, IS NOT INDICATED.
22. CONCRETE SHALL BE PROTECTED WITH FORM COATING COMPOUNDS THAT WILL NOT BOND, STAIN, OR ADVERSELY AFFECT CONCRETE SURFACES. SET FORMS BEFORE PLACING CONCRETE.
23. CONCRETE SHALL BE CONSOLIDATED IN PLACE USING INTERNAL VIBRATORS.
24. REPAIR AND PATCH DEFECTIVE AREAS WITH CEMENT MORTAR IMMEDIATELY AFTER REMOVAL OF FORMS, EXCEPT WHERE REINFORCING IS VISIBLE. CONTACT STRUCTURAL ENGINEER FOR EVALUATION OF EXPOSED REINFORCING.
25. PROVIDE CORNER BARS AT ALL WALL FOOTING CORNERS TO MATCH HORIZONTAL BARS.

- 26. SUBMITTALS: a. SUBMIT PROPOSED CONCRETE MIX DESIGN PRIOR TO CONSTRUCTION, INCLUDING BACKUP DATA IN ACCORDANCE WITH ACI 301-CURRENT EDITION CHAPTER 4, SECTION 4.2.3, EXCLUDING SECTION 4.2.3.4B. b. SUBMIT DETAILED SHOP DRAWINGS OF REINFORCING BARS SHOWING NUMBER, SIZE, AND LOCATION. INCLUDE BAR LISTS AND BEND DIAGRAMS.
27. ALL BUILDING AND SITE SLABS-ON-GRADE SHALL BE AT LEAST 4" THICK, REINFORCED WITH 6x6-W/4 X 4 W/4 W/4 W/4, ON 10 MIL VAPOR BARRIER, WITH SAW-CUT CONTROL JOINTS 15'-0" O.C. EACH WAY INCLUDING HOUSEKEEPING PASSEYS AS REQUIRED. SEE PLANS FOR OTHER CONDITIONS.
28. STEP AND SLOPE ALL WALKWAYS AWAY FROM THE BUILDING.

STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL CONFORM TO THE AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", LATEST EDITION, EXCEPT CHAPTER 4.2.1, CODE OF STANDARD PRACTICE.
2. STRUCTURAL STEEL SHAPES, TEES - ASTM A992.
3. STRUCTURAL STEEL ANGLES, CHANNELS, PLATES, BARS - ASTM A36.
4. SIZE AND USE OF HOLES: SEE ALSO TABLE J3.1 U.N.C. a. OVERSIZED OR LONG-SLOTTED HOLES ARE NOT PERMITTED U.N.C. MAXIMUM HOLE DIAMETER = BOLT DIAMETER + 1/16".
4. SHOP PAINT - METAL ALKYL-DIESEL PRIMER, ANY OF THE FOLLOWING: MANUFACTURER DESIGNATION PORTER NO. 298 MOBILE NO. 13F912 TIMBERKNOX NO. 1029 AMERON NO. 5102 AMERCAD
SHOP PAINT ALL SURFACES OF STEEL, EXCEPT ANCHOR BOLTS AND SURFACES TO BE FIELD WELDED. APPLY PAINT IN ACCORDANCE WITH SSPC-PAI. SHOP FIELD AND MAINTENANCE PAINTING. APPLY PAINT IN SUFFICIENT VOLUME OR COATS TO PROVIDE A MINIMUM DRY FILM THICKNESS OF AT LEAST 3 MILS, BUT NOT MORE THAN 5 MILS.
5. SURFACE PREPARATION - PREPARE STEEL SURFACE IN ACCORDANCE WITH SSPC-SP9 POWER TOOL CLEANING. ANY METHOD IN CONFORMANCE WITH AN SSPC SPECIFICATION OF HIGHER QUALITY THAN LISTED WILL BE ACCEPTABLE. AT OPTION OF CONTRACTOR, WHEELABRATOR MAY BE USED FOR PREPARATION OF STEEL SURFACES, PROVIDING RESULTANT SURFACE IS EQUAL IN ALL RESPECTS TO THOSE REQUIRED.
6. NO SPLICES SHALL BE PERMITTED IN ANY STRUCTURAL STEEL MEMBER UNLESS SHOWN ON APPROVED SHOP DRAWINGS.
7. SUBMITTALS: CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS SHOWING ALL STRUCTURAL STEEL LAYOUTS AND DETAILS, SIZES OF MEMBERS, TYPE OF STEEL, CONNECTION DETAILS, WELDS, BOLTS, ETC., AS REQUIRED TO FABRICATE AND ERECT ALL STRUCTURAL STEEL FRAMING. ALL CONNECTIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BY THE DETAILER AND SUBMITTED ON SHOP DRAWINGS, SIGNED AND SEALED BY A REGISTERED FLORIDA ENGINEER. STEEL STAIRS SHALL ALSO BE SUBMITTED ON SEALED DRAWINGS.
8. ERECTION: a. BEFORE ERECTION, THE CONTRACTOR IS TO REMOVE ALL MUD, DIRT OR OTHER FOREIGN MATTER, WHICH ACCUMULATES DURING HANDLING AND STORAGE. b. AFTER ERECTION, CLEAN BOLTED CONNECTIONS AND ABRASION AREAS WHERE SHOP COAT HAS BEEN DAMAGED. SPOT AND PRIME AREAS USING SAME MATERIAL AS SHOP COAT. c. SET ALL MEMBERS SO THAT, IN THEIR FINAL LOCATION, LEVEL, PLUMBNESS AND ALIGNMENT ARE WITHIN THE TOLERANCES PRESCRIBED BY AISC CODE.

CARPENTRY

- 1. DIMENSIONED LUMBER SHALL BE DRESSED S4S, AND SHALL BEAR THE GRADE STAMP OF THE MANUFACTURER'S ASSOCIATION.
2. ALL LUMBER SHALL BE SOUND, SEASONED, AND FREE FROM WARP.
3. ALL LUMBER SHALL BE SOUTHERN PINE WITH 19% MAXIMUM MOISTURE CONTENT, U.N.C. ON THE PLAN: a. FOR STRUCTURAL LIGHT FRAMING (2 TO 4 INCHES THICK, 2 TO 4 INCHES WIDE) AND STRUCTURAL FRAMING (2 TO 4 INCHES THICK, 5 INCHES AND WIDER), ALL LUMBER TO BE NO. 2 GRADE OR BETTER. b. FOR TIMBERS (5 INCHES AND THICKER) ALL LUMBER TO BE NO. 1 GRADE OR BETTER, AND FREE OF HEART CENTERS.
4. ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED.
5. PRESSURE TREATED LUMBER SHALL BE IMPREGNATED WITH AN ACO CALKULATING COPPER QUATERNARY TREATMENT IN ACCORDANCE WITH THE REQUIREMENTS OF THE AMERICAN WOOD PRESERVERS INSTITUTE.
6. PLYWOOD WALL AND ROOF SHEATHING SHALL BE APA RATED SHEATHING, EXTERIOR BOND CLASSIFICATION.
7. ROOF SHEATHING TO BE INSTALLED WITH ONE PLYWOOD CLIP PER SPAN BETWEEN PANEL EDGES UNLESS NOTED OTHERWISE. FOR PANEL WIDTHS LESS THAN 24" WIDE, INSTALL SOLID BLOCKING AT EACH PANEL EDGE. INSTALL SOLID 2x BLOCKING BETWEEN SUPPORTS AT ALL RISERS, RIDGES, VALLEYS, AND CHANGES IN ROOF SLOPE.
8. INSTALL BRIDGING IN ALL FLOOR OR ROOF JOISTS AT 8'-0" O.C. MAXIMUM. INSTALL BLOCKING IN ALL WALL STUDS 4'-0" O.C. MAXIMUM, LOCATED AT SHEATHING PANEL EDGES.
9. ALL NAILING AND BOLTING SHALL COMPLY WITH AMERICAN INSTITUTE OF TIMBER CONSTRUCTION REQUIREMENTS.
10. ALL CONNECTION HARDWARE SHALL BE GALVANIZED AND SUPPLIED BY SIMPSON STRONG-TIE CO. OR EQUIVALENT. SUBMIT CUT SHEETS FOR ALL CONNECTION HARDWARE TO ENGINEER FOR APPROVAL.
11. ALL CONNECTION HARDWARE IS TO BE FULLY FASTENED PER MANUFACTURER'S REQUIREMENTS UNLESS NOTED OTHERWISE.
12. PROVIDE A SINGLE PLATE AT THE BOTTOM OF DOUBLE PLATE AT THE TOP OF ALL STUD WALLS. SILL PLATES SHALL BE BOLTED TO FOUNDATION.
13. STUDS SHALL BE DOUBLE ALL ANGLES AND ALL CORNERS. STUDS SHALL BE TRIPLED UNLESS NOTED OTHERWISE. ALL LINEAL FINISHES SHALL BE DOUBLE 2x6 HEADERS FOR SPANS UP TO 10' AND DOUBLE 2x8 HEADERS FROM 3'-4" TO 6'-0". SEE PLANS FOR SPANS GREATER THAN 10'. USE TRIPLE 2x6 HEADERS FOR 2x6 STUD FRAMING. DOUBLE AND TRIPLE 2x6 HEADERS FOR 2x6 STUDS. PROVIDE FILLER CUT TO FULL DEPTH OF HEADER BETWEEN 2x6 MEMBER WALLS WITH 16d NAILS AT 12" ON CENTER TOP AND BOTTOM.
14. NAILING SCHEDULE:
TABLE: CONNECTION, COMMON NAIL, NUMBER OR SPACING
15. FASTENER SUBSTITUTIONS: ALL NAILS ARE COMMON NAILS, UNLESS NOTED OTHERWISE. THE FOLLOWING FASTENERS ARE ACCEPTABLE SUBSTITUTIONS. ALL ALTERNATE FASTENERS SHALL BE SPACED AT THE SAME SPACING AS THE SCHEDULED FASTENERS.
SCHEDULED FASTENER ALTERNATE FASTENER
8d COMMON NAIL 8d RING SHANK NAIL 8d SCREW SHANK NAIL 0.131 P-NAIL
10d COMMON NAIL 10d RING SHANK NAIL 10d SCREW SHANK NAIL 0.149 P-NAIL
17. GUN DRIVEN NAILS MUST BE SUBMITTED FOR REVIEW WITH APPROPRIATE BACK-UP DATA.

MICROLLAM L.V.L. LUMBER

- 1. MATERIAL, MANUFACTURE AND QUALITY CONTROL SHALL BE IN CONFORMANCE WITH THE NES REPORT NO. NER-461.
2. VENEERS SHALL BE DOUGLAS FIR OR SOUTHERN PINE OF THICKNESS APPROVED BY THE BUILDING CODE. THEY SHALL BE ULTRASONICALLY GRADED OR GRADED BY OTHER ADVANCED GRADING SYSTEM APPROVED BY THE CODE.
3. ADHESIVES SHALL BE OF THE WATERPROOF TYPE CONFORMING TO THE REQUIREMENTS OF ASTM D-2529.

- 4. MINIMUM ALLOWABLE STRESS VALUES: Fb = 2600 PSI Fv = 280 PSI Fc11 = 2510 PSI E = 2,000,000 PSI
5. IF REQUIRED, PROVIDE WRITTEN CERTIFICATION THAT MICROLLAM MEMBERS CONFORM TO THE REQUIRED SPECIFICATIONS.
6. L.V.L. MEMBERS SHALL BE USED UNDER DRY CONDITIONS AND SHALL NOT BE INSTALLED WHERE THE MAXIMUM MOISTURE CONTENT EXCEEDS 18%.
7. STRUCTURAL MICROLLAM BEAMS SHALL BE IN ACCORDANCE WITH "NATURAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION," LATEST EDITION.
8. SUBMITTALS: ALL SUBMITTALS SHALL BEAR THE EMBOSSED SEAL OF A LICENSED FLORIDA ENGINEER AND SHALL BE SUBMITTED FOR REVIEW BY THE ARCHITECT/ENGINEER PRIOR TO FABRICATION.
9. SUBMITTALS TO INCLUDE: a. ERECTION PLAN. b. MEMBER AND CONNECTION DETAILS. c. TEMPORARY SHORING PLAN, IF REQUIRED.

PRE-ENGINEERED WOOD TRUSSES

- 1. THIS SECTION DEFINES PRE-ENGINEERED, PREFABRICATED, METAL PLATE CONNECTED WOOD TRUSSES AS "WOOD TRUSSES".
2. WOOD TRUSSES SHALL BE DESIGNED AND FABRICATED IN ACCORDANCE WITH THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" (CURRENT EDITION, PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION, "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION, TP1-CURRENT EDITION", PUBLISHED BY THE TRUSS PLATE INSTITUTE) AND THE APPLICABLE BUILDING CODE LISTED IN THE MISCELLANEOUS SECTION OF THESE SPECIFICATIONS.
3. THE WOOD TRUSS MANUFACTURER MUST PARTICIPATE IN A CODE APPROVED THIRD PARTY QUALITY ASSURANCE PROGRAM SUCH AS THE TRUSS PLATE INSTITUTE'S "QUALITY CONTROL INSPECTION PROGRAM" OR EQUIVALENT.
4. WOOD TRUSS MEMBERS AND CONNECTIONS SHALL BE DESIGNED FOR ALL LOADS SHOWN ON THE CONTRACT DOCUMENTS INCLUDING LIVE, DEAD, AND CONCENTRATED LOADS, PLUS WIND LOADS IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE: a. MINIMUM SUPERIMPOSED DEAD LOADS: 1) TOP CHORD ..... 10 PSF 2) BOTTOM CHORD ..... 10 PSF b. MINIMUM NET UPLIFT: 1) INTERIOR ..... 16 PSF 2) OVERHANG ..... 16 PSF
5. DURATION OF LOAD FACTORS: ROOF DL+LL+W ..... 1.25 FLOOR DL+LL ..... 1.00
6. WOOD TRUSS DESIGN SHOP DRAWINGS SHALL INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING INFORMATION: a. SPAN LENGTH, OVERHANG AND EAVE DIMENSIONS, SLOPE AND SPACING OF THE WOOD TRUSSES. b. ALL DESIGN LOADS AND THEIR POINTS OF APPLICATION, VALLEY AND CONVENTIONAL FIRMING MUST BE CONSIDERED. c. ADJUSTMENTS TO ALLOWABLE VALUES, DURATION OF LOAD FACTORS, ETC. d. REACTIVE FORCES AND THEIR LOCATIONS. e. BEARING TYPE AND MINIMUM BEARING LENGTH. f. DEFLECTIONS, SPAN AND REACTION. g. METAL CONNECTION PLATE TYPE, GAUGE, SIZE, AND LOCATION. h. LUMBER SIZE, SPECIES, GRADE AND MOISTURE CONTENT. i. LOCATION AND CONNECTION DESIGN OF REQUIRED CONTINUOUS LATERAL BRACING. j. TRUSS SPLICES MUST BE DETAILED. THIS INCLUDES "PIGGY BACK" SPLICES. k. CONNECTION DETAILS: TRUSS TO BEARING, TRUSS TO TRUSS, TRUSS TO TRUSS TO TRUSS, PIGGY BACK TO TRUSS, ETC. l. BRACING: NOTE MINIMUM BOTTOM CHORD BRACING AND CROSS BRACING REQUIREMENTS BELOW.
7. DEFLECTIONS (UNLESS NOTED OTHERWISE): a. SPAN LIVE LOAD: LESS THAN OR EQUAL TO SPAN. b. SPAN TOTAL LOAD: LESS THAN OR EQUAL TO SPAN/2.
8. SUPPORTS: WOOD TRUSSES SHALL BE DESIGNER WITH AT LEAST ONE LATERAL ROLLER CONNECTION PER SPAN SPAN AND HORIZONTAL CONNECTIONS TO INCLUDE SUPPORTS UNDER DEAD OR LIVE LOADS.
9. WOOD TRUSSES MUST BE CHECKED FOR WIND, WIND VELOCITY PRESSURES, AND TYPE OF EXPOSURE FOR WIND. MUST BE SHOWN ON THE SUBMITTED SHOP DRAWINGS.
10. CONTINUOUS BOTTOM CHORD LATERAL BRACING IS REQUIRED AT A MAXIMUM SPACING OF 10' O.C. UNLESS NOTED OTHERWISE. CONTINUOUS BRACING IS CONTINUOUS FROM ONE END OF THE TRUSS TO THE OTHER. CONTINUOUS BRACING AT LEAST ONE TRUSS SPACE. USE A MINIMUM OF 2 X 4 LUMBER AT LEAST 10' LONG, WITH 2-16d NAILS AT INTERSECTION AND 6d WALL STUDS AND CONNECTIONS.
11. CROSS BRACING SHALL BE INSTALLED AT CONTINUOUS LATERAL BRACING UNLESS NOTED OTHERWISE. LOCATE CROSS BRACING AT OR NEAR THE BOTTOM CHORD BRACING. INSTALL CROSS BRACING AT EACH END AND AT MIDSPAN. ALONG THE LENGTH OF THE LATERAL BRACING. CROSS BRACING IS ACCOMPLISHED BY ATTACHING DIAGONAL WEB BRACING TO OPPOSITE SIDES OF THE SAME GROUP OF SIMILAR WEB MEMBERS. SLOPE CROSS BRACING IN OPPOSITE DIRECTIONS AT APPROXIMATELY 45 DEGREES FORMING A CROSS "X". USE A MINIMUM OF 2 X 4 GRADE MARKED LUMBER WITH AT LEAST 2-16d NAILS AT EACH CONNECTION.
12. TRUSS ERECTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING OF TRUSS SYSTEM DURING CONSTRUCTION.
13. HANDLING, INSTALLATION, AND BRACING OF WOOD TRUSSES SHALL BE IN ACCORDANCE WITH "BCSI-CURRENT EDITION", PUBLISHED BY THE TRUSS PLATE INSTITUTE.
14. ALL WOOD TRUSSES SHALL BE FASTENED TO THEIR SUPPORTS WITH APPROVED HURRICANE CLIPS OR STRAPS.
15. CONTRACTOR SHALL ORDER AND INSTALL HURRICANE CLIPS OR STRAPS FOR THE UPLIFT AND LATERAL FORCES SHOWN ON THE SUBMITTED WOOD TRUSS DESIGN CALCULATIONS.
16. ALL CONNECTION HARDWARE SHALL BE GALVANIZED AND SUPPLIED BY SIMPSON STRONG-TIE CO. OR BY APPROVED EQUIVALENT MANUFACTURER.
17. ALL CONNECTION HARDWARE IS TO BE FULLY FASTENED PER MANUFACTURER'S REQUIREMENTS WITH MAXIMUM NUMBER AND SIZE: NAILS OR BOLTS UNLESS NOTED OTHERWISE.
18. PILING OF PLYWOOD ON WOOD TRUSSES IS NOT ALLOWED.
19. INSTALLATION OF BROKEN, DAMAGED, WARPED, OR IMPROPERLY REPAIRED WOOD TRUSSES IS NOT ALLOWED.
20. IMPROPER OR UNAUTHORIZED FIELD ALTERATIONS OF WOOD TRUSSES IS NOT ALLOWED.
21. ALL CONNECTIONS AND BRACING MUST BE INSTALLED BEFORE SHEATHING THE ROOF.
22. WOOD TRUSS DESIGN ENGINEER MUST BE PROVIDED WITH A COPY OF THESE DRAWINGS AND SPECIFICATIONS.
23. SUBMITTALS: ALL SUBMITTALS SHALL BEAR THE EMBOSSED SEAL OF A LICENSED FLORIDA ENGINEER AND SHALL BE SUBMITTED FOR REVIEW BY THE STRUCTURAL ENGINEER PRIOR TO TRUSS FABRICATION. a. SUBMIT SEALED WOOD TRUSS DESIGN CALCULATIONS AND PROFILES FOR EACH TYPE OF TRUSS, WITH PERMANENT BRIDGING REQUIREMENTS. b. SUBMIT WOOD TRUSS ERECTION PLAN, INCLUDING CONNECTION DETAILS. c. SUBMIT WOOD TRUSS TEMPORARY ERECTION BRACING PLAN.

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