

GENERAL NOTES

- I. GENERAL
A. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, SHOP DRAWINGS AND SPECIFICATIONS.
B. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO OBTAIN ALL CONTRACT DOCUMENTS AND LATEST ADDENDA AND TO SUBMIT TO ALL SUBCONTRACTORS AND SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS.
C. THE GENERAL CONTRACTOR SHALL COMPARE ALL CONTRACT DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN DISCIPLINES AND WITHIN A GIVEN DISCIPLINE TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION AND ERECTION.
D. IF A CONFLICT EXISTS AMONG THE STRUCTURAL DRAWINGS, GENERAL NOTES, OR THE SPECIFICATIONS, THE STRICTEST REQUIREMENTS AS INDICATED BY THE ENGINEER SHALL GOVERN.
E. THE CONTRACTOR SHALL COORDINATE ALL ELEVATIONS AND DIMENSIONS, INCLUDING BUT NOT LIMITED TO THOSE FOR OPENINGS IN WALLS AND IN ROOF AND FLOOR SYSTEMS, WITH THE ARCHITECTURAL, PLUMBING, ELECTRICAL, AND MECHANICAL PLANS.
F. ALL DIMENSIONS, ELEVATIONS, AND ANY OTHER CONDITIONS OF ANY EXISTING STRUCTURES OR OTHER FEATURES SHALL BE VERIFIED BY THE GENERAL CONTRACTOR AND ANY DISCREPANCIES WITH THE CONTRACT DRAWINGS REPORTED TO THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DURING THE CONSTRUCTION PROCESS, IT SHALL BE SOLELY THE CONTRACTOR'S RESPONSIBILITY TO MAINTAIN THE INTEGRITY OF THE EXISTING STRUCTURE AND TO PROTECT FROM DAMAGE ANY PORTIONS THAT ARE TO REMAIN.
G. THE COMPLETED LATERAL-FORCE RESISTING SYSTEMS AND DIAPHRAGMS ARE REQUIRED FOR THE CONSTRUCTION PROCESS. THE CONTRACTOR SHALL PROVIDE ALL REQUIRED BRACING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS UNTIL THE LATERAL-LOAD RESISTING OR STABILITY-PROVIDING SYSTEM IS COMPLETELY INSTALLED AND THE STRUCTURE IS COMPLETELY TIED TOGETHER.
H. UNLESS OTHERWISE NOTED, DETAILS SHOWN ON ANY DRAWING ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.
I. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS AND METHODS AND FOR SAFETY PRECAUTIONS AND PROGRAMS.
J. BRITT, PETERS & ASSOCIATES, INC. SHALL NOT BE RESPONSIBLE FOR THE ACTS OR OMISSION OF THE CONTRACTOR OR FOR THEIR FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
K. PERIODIC SITE OBSERVATION BY BRITT, PETERS & ASSOCIATES, INC. IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS AND IS NOT EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK.
L. THE BUILDING OWNER SHALL PROVIDE PERIODIC MAINTENANCE TO INSURE STRUCTURAL INTEGRITY. SUCH MAINTENANCE SHALL INCLUDE BUT NOT LIMITED TO PAINTING OF STEEL, PROTECTIVE COATING FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS.

- II. DESIGN CRITERIA
A. THE CONTRACT DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE FLORIDA BUILDING CODE, 2017 EDITION.
B. DEAD LOADS
1. TYPICAL ROOF SYSTEMS: (20 PSF TOTAL)
a. MEP: 10 PSF
b. INSULATION & ROOFING: 10 PSF
c. MISCELLANEOUS CEILING AND HANGING MECHANICAL LOADS SUCH AS DUCT WORK AND SPRINKLER PIPES.
** PARTITION LOAD INCLUDED WITH LIVE LOAD FOR OFFICE SPACES.
C. LIVE LOADS
1. LIVE LOADS ARE BASED ON THE MORE RESTRICTIVE OF THE UNIFORM LOAD LISTED BELOW OR THE CONCENTRATED LOAD LISTED BELOW.
a. FLOOR: 200
b. ROOFS: 300
ALL ROOF SURFACES SUBJECT TO WORKERS ORDINARY ROOF OR EQUIPMENT WEIGHT IF GREATER
D. DESIGN SNOW LOAD: PG 0 PSF
E. DESIGN WIND LOADS:
BASIC WIND SPEED: Vult/Vasd 142/118 MPH (3-SEC GUST)
RISK CATEGORY: II
EXPOSURE: B
INTERNAL PRESSURE COEFF: GCPI +/-0.18 (ENCLOSED BUILDING)

Design Wind Pressure (psf) table with columns for Walls, Roof, and Parapet Design Pressure. Includes sub-tables for Effective Wind Area (sqft) and Parapet Design Pressure (psf).

- F. SEISMIC LOADS
SHORT PERIOD SPECTRAL RESPONSE ACCELERATION, SS 0.060
1-SEC PERIOD SPECTRAL RESPONSE ACCELERATION, S1 0.032
SHORT PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION, SD2 0.064
1-SEC PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION, S1 0.051
RISK CATEGORY: D
SEISMIC DESIGN CATEGORY: III
BASIC SEISMIC-FORCE RESISTING SYSTEM: STEEL SYSTEM NOT SPECIFICALLY DESIGNED FOR SEISMIC RESISTANCE
RESPONSE MODIFICATION FACTOR: 3
DEFLECTION AMPLIFICATION FACTOR: 1.0
SEISMIC IMPORTANCE FACTOR: 1.0
SEISMIC RESPONSE COEFFICIENT: 0.01
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
DESIGN BASE SHEAR: 13 KIPS
G. THE CONTRACTOR SHALL SUBMIT FINAL ELEVATION SHOP DRAWINGS SHOWING ALL LOADS PRIOR TO THE FABRICATION OF ALL SUPPORTING STRUCTURAL MEMBERS.
H. THE CONTRACTOR SHALL MECHANICALLY IDENTIFY MEMBER WEIGHTS, LOCATIONS AND ASSOCIATED OPENINGS WITH THE MECHANICAL CONTRACTOR AND SUBMIT SUCH INFORMATION PRIOR TO FABRICATION OF THE SUPPORTING STRUCTURE. PROMPTLY NOTIFY THE ENGINEER IF THE ACTUAL WEIGHT EXCEEDS THE WEIGHT SHOWN ON THE STRUCTURAL DRAWINGS.
I. PARTITIONS SHALL BE MADE IN THE DETAILING, FABRICATION, AND ERECTION OF ALL CLADDING, PARTITIONS, WALLS, ETC. TO ACCOUNT FOR FLOOR TO FLOOR DEFLECTIONS AND LATERAL FRAME DEFLECTIONS.

- III. FOUNDATIONS
A. FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL INVESTIGATION REPORT, "GEOTECHNICAL ENGINEERING SERVICES REPORT: AAA CAR CARE PLUS CENTER, 1701 NORTH WESTSHORE BOULEVARD, TAMPA, FLORIDA, BY PROFESSIONAL SERVICE INDUSTRIES, INC. (PROJECT NO. 07752676), DATED MARCH 7, 2018.
B. CONTRACTOR SHALL OBTAIN A COPY OF THE SOILS REPORT AND ADHERE TO ALL RECOMMENDATION WITHIN, INCLUDING PREPARATION OF SOILS AT BUILDING PAD.
C. ALL SOILS WORK, INCLUDING BACKFILL OF UTILITY TRENCHES AND THE VERIFICATION OF BEARING CAPACITY OF SAME SHALL BE UNDER THE DIRECTION OF A QUALIFIED SOILS ENGINEER. PROXIMITY OF UTILITY TRENCHES TO BUILDING FOUNDATION SYSTEM SHALL BE AS APPROVED BY THE SOILS ENGINEER TO INSURE INTEGRITY OF THE BEARING SOILS.
D. ALL FOOTINGS SHALL BEAR ON UNDISTURBED EARTH OR ENGINEERED FILL AT ELEVATIONS SHOWN ON PLANS AND DETAILS. FLOOR SLABS SHALL BEAR ON 4 INCHES OF GRANULAR FILL. THE MOISTURE RETARDER SHALL BE PLACED BETWEEN THE STONE AND THE SLAB.
E. NO FOUNDATION CONCRETE SHALL BE INSTALLED UNTIL ALL FOUNDATION WORK HAS BEEN COORDINATED WITH UNDERGROUND UTILITIES. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD OF ALL CONFLICTS THAT EXIST BETWEEN FOOTINGS AND UTILITIES.
F. ALL FOUNDATIONS OR PORTIONS THEREOF, BELOW GRADE MAY BE EARTH FORMED BY NEAT EXCAVATIONS.
G. UNLESS OTHERWISE SHOWN, ALL FOOTINGS SHALL BE CENTERED ON WALLS AND/OR COLUMNS.
H. THE CONTRACTOR SHALL DETERMINE THE EXTENT OF CONSTRUCTION DEWATERING REQUIRED FOR THE EXCAVATION. THE CONTRACTOR SHALL SUBMIT TO THE GEOTECHNICAL ENGINEER FOR REVIEW THE PROPOSED PLAN FOR CONSTRUCTION DEWATERING, PRIOR TO EXCAVATION.
I. FOOTINGS SHALL NOT BE PLACED ON FROZEN SUBGRADE OR IN STANDING WATER.
J. FOUNDATION TYPE
1. SPREAD FOOTING:
a. TOTAL LOAD: 2500 PSF

Table with columns: USAGE, STRENGTH (PSI), CONC. TYPE, COMMENTS. Lists concrete requirements for various applications like footings, slabs, and walls.

- V. POST-INSTALLED ANCHORS
A. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS.
B. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER BEFORE USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
C. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REBAR. REBAR SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS. ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACINGS INDICATED IN THE MANUFACTURER'S LITERATURE. CONTACT MANUFACTURER PRIOR TO ANCHOR INSTALLATION, IF TRAINING IS REQUIRED.
D. UNLESS SPECIFIED OTHERWISE, ANCHORS SHALL BE EMBEDDED IN THE APPROPRIATE SUBSTRATE WITH A MINIMUM EMBEDMENT OF 8 TIMES THE NOMINAL ANCHOR DIAMETER OR EMBEDMENT REQUIRED TO SUPPORT THE INTENDED LOAD.
E. ADHESIVE ANCHOR DESIGN STRENGTH HAS BEEN BASED ON CRACKED CONCRETE, ACI 308.4-4. TEMPERATURE CATEGORY 1. ALL INSTALLATIONS IN DRY HOLLOW DRILLED USING A HAMMER DRILL INTO CONCRETE THAT IS CURED FOR AT LEAST 28 DAYS. ADHESIVE ANCHORS SHALL BE INSTALLED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER PER ACI 318-11, D.9.2.2 WHERE INDICATED ON THE CONTRACT DOCUMENTS. INSTALLATIONS REQUIRING CERTIFIED INSTALLERS SHALL BE INSPECTED PER ACI 318-11, D.9. SUBSTITUTION OF PRODUCTS OTHER THAN THOSE LISTED BELOW, SHALL BE SUBMITTED TO THE ENGINEER WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE BUILDING CODE. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED WITH A CURRENT ICC-ES REPORT INDICATING USE OF POST-INSTALLED ANCHOR IN CRACKED CONCRETE.
G. ACCEPTABLE PRODUCTS ARE:
1. MASONRY MECHANICAL ANCHORS:
a. HILTI KB-TZ
b. HILTI KWIK HUB-EZ
c. SIMPSON STRONG-TIE TITEN-HD
d. SIMPSON STRONG-TIE "STRONG-BOLT"
2. CONCRETE ADHESIVE ANCHORS:
a. HILTI HIT 100
b. HILTI HY 200
c. SIMPSON STRONG-TIE "SET-XP"
d. SIMPSON STRONG-TIE "AT-XP"

- VI. STRUCTURAL STEEL
A. ALL HOT ROLLED STEEL PLATES, SHAPES, SHEET PILING, AND BARS SHALL BE NEW STEEL CONFORMING TO ASTM SPECIFICATION A6-98A.
B. STRUCTURAL STEEL SHALL BE AS FOLLOWS, U.N.O.:
1. WIDE FLANGE SHAPES: ASTM A992 FY = 50 KSI
2. STRUCTURAL RECTANGULAR TUBING: ASTM A500 GRADE B FY = 46 KSI
3. ALL OTHER STRUCTURAL STEEL: ASTM A36 FY = 36 KSI
4. CONNECTION MATERIALS:
a. BEAM COLUMN STIFFENER PLATES AND DOUBLER PLATES: ASTM A572 GRADE 50
b. ALL OTHER CONNECTION MATERIAL, U.N.O.: ASTM A36 UNLESS A HIGHER GRADE OF STEEL IS REQUIRED BY STRENGTH AND PROVIDED THE RESULTING SIZES ARE COMPATIBLE WITH THE CONNECTED MEMBERS.
8. ASTM A572 GRADE 50 IS ACCEPTABLE AS A SUBSTITUTE FOR A992.
C. STRUCTURAL STEEL SHALL MEET THE LATEST AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
D. THE CENTERLINES OF ALL COLUMNS AND BEAMS SHALL BE LOCATED ON COLUMN LINES UNLESS OTHERWISE SHOWN.
E. CONNECTIONS:
1. BOLTS SHALL BE A325M TYPE 1, UNLESS OTHERWISE INDICATED.
2. ALL BOLTS SHALL BE SNUG TIGHT, UNLESS NOTED OTHERWISE. BOLTS SHALL BE TIGHTENED UNTIL ALL PLIES OF THE JOINT ARE IN FIRM CONTACT.
3. BOLTS THAT ARE DESIGNATED AS SLIP CRITICAL SHALL BE FULLY TENSIONED TO THE MINIMUM LOADS AS INDICATED IN THE "SPECIFICATION OF STRUCTURAL JOINTS USING ASTM325 OR A490 BOLTS".
4. ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 36, UNLESS NOTED OTHERWISE.
5. THREADED RODS SHALL CONFORM TO ASTM A36.
6. BOLTED MOMENT CONNECTIONS SHALL BE SLIP-CRITICAL CONNECTIONS. OTHER CONNECTIONS SHALL BE BEARING CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANES.
7. WELDING SHALL CONFORM TO THE STANDARDS SET FORTH IN AWS PUBLICATION, "WELDING IN BUILDING CONSTRUCTION".
8. UNLESS NOTED OTHERWISE, ELECTRODES FOR WELDING SHALL CONFORM TO E70XX (SMAW), F7XX-EXXX (SAW), E70S-X (GMAW), OR E7X-X (FCAW).
9. UNLESS INDICATED "CJP" SHALL BE COMPLETE JOINT PENETRATION GROOVE WELDS. FABRICATOR SHALL PRODUCE COMPLETE JOINT PENETRATION GROOVE WELDS WHICH CONFORM TO ALL AWS D1.1 QUALIFIED WELD REQUIREMENTS AND WHICH ARE APPLICABLE TO THE SPECIFIC CONDITIONS SHOWN.
10. ALL ERECTION DRAWINGS SHALL SHOW ALL FIELD WELDS REQUIRED.
11. CONNECTION DETAILS NOT COMPLETELY DETAILED ON THE DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER TO RESIST FORCES INDICATED ON THE DRAWINGS. INDICATED FORCES ARE BASED ON FACTORED LOADS AND ARE INTENDED FOR USE WITH THE LOAD AND RESISTANCE FACTOR DESIGN METHOD. WHERE NONE ARE INDICATED, BEAMS SHALL BE DESIGNED FOR AN END REACTION EQUAL TO 1/2 OF THE TOTAL UNIFORM LOAD CAPACITY TABULATED IN THE UNIFORM LOAD TABLES OF THE AISC MANUAL. THE CONTRACTOR SHALL EMPLOY THE ASSISTANCE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. DESIGN CALCULATIONS FOR THE CONNECTIONS DESIGNED BY THE SPECIALTY ENGINEER SHALL BE SUBMITTED FOR THE FILES OF THE ARCHITECT AND ENGINEER. ALL DESIGN CALCULATIONS SHALL BE PERFORMED BY THE PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. SHOP DRAWINGS CONTAINING CONNECTIONS FOR WHICH CALCULATIONS HAVE NOT BEEN RECEIVED WILL BE RETURNED UNCHECKED AS AN INCOMPLETE SUBMITTAL. CONNECTIONS ECCENTRICITY SHALL BE TAKEN INTO ACCOUNT WHEN DESIGNING AND DETAILING THE CONNECTION.
F. WHERE THE WORK OF OTHER TRADES REQUIRES CUTS, HOLES, ETC., IN STRUCTURAL STEEL MEMBERS, CUTS, HOLES, ETC., SHALL BE MADE IN THE SHOP AND SHALL BE SHOWN ON THE SHOP DRAWINGS. MAKING HOLES OR CUTS IN STRUCTURAL STEEL MEMBERS IN THE FIELD WILL NOT BE PERMITTED WITHOUT SPECIFIC APPROVAL OF THE ENGINEER.
G. BAR GRATINGS SHALL BE STEEL 1 1/2" X 3/16" GALVANIZED GRATING. GRATINGS SHALL BE CLAMPED TO SUPPORT BEAMS WITH GALVANIZED CLAMPS THAT DO NOT REQUIRE DRILLING OF BEAMS. EDGES AND OPENINGS IN GRATING MORE THAN 4" IN DIAMETER SHALL BE BANDED.
H. GROUT BELOW BASE PLATES SHALL BE NON-METALLIC GROUT. SHRINK GROUT WITH A MINIMUM STRENGTH OF 6000 PSI WHEN BEARING ON 3000 PSI CONCRETE OR LESS. SHRINK GROUT OF 8000 PSI WHEN BEARING ON CONCRETE BETWEEN 3000 AND 4000 PSI.
I. ALL STRUCTURAL STEEL SHALL BE SHIPPED WITH ONE COAT OF PRIMER EXCEPT THOSE MEMBERS THAT ARE GALVANIZED OR IN AREAS SCHEDULED FOR INDUCED FIRE RATING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING AREAS TO BE FIRE PROTECTED.

- VII. DECKING
A. STEEL DECK SHALL BE DESIGNED AND FABRICATED IN CONFORMANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS PUBLICATION.
B. PROVIDE JOIST LENGTHS TO DATE FOR A THREE-SPAN CONDITION WHERE POSSIBLE.
C. THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF ALL OPENINGS, SLEEVES, INSERTS, ETC., WITH SHOP DRAWINGS OF THE EQUIPMENT TO BE INSTALLED.
D. STEEL DECK SHALL BE ATTACHED TO ALL MEMBERS ON WHOM IT BEARS.
E. STEEL DECK SHALL HAVE AN EVIDENT BEARING AT END AND INTERMEDIATE SUPPORTS TO PREVENT WEB BUCKLING (1 1/2" MINIMUM BEARING). MINIMUM SHORING BEAM WIDTHS SHALL BE PER THE DECK MANUFACTURER'S RECOMMENDATIONS.
F. DECK MANUFACTURER SHALL PROVIDE CELL CLOSURES, COLUMN CLOSURES, FINISH STRIPS, GIRDER BEARS, AND THE ATTACHMENTS AS REQUIRED TO ACHIEVE A COMPLETE SYSTEM.
G. DECK MANUFACTURER SHALL PROVIDE L2 1/2" X 3/16" WELDED TO FACE OF COLUMNS TO SUPPORT DECK. ALL LOCATIONS WHERE DECKING IS CUT AROUND COLUMNS.
H. UNLESS INDICATED OTHERWISE, PROVIDE POUR STOPS OF LENGTH, DEPTH AND GAGE APPROPRIATE FOR OVERLAP AND SLAB DEPTH. PROVIDE WELD WASHERS WHEN WELDING DECK THINNER THAN 22 GAUGE MATERIAL. GENERAL CONTRACTOR SHALL SUBMIT ALL ALTERNATE PRODUCTS TO THE ENGINEER OF RECORD FOR APPROVAL PRIOR TO FABRICATION OF ANY MATERIAL.
K. ALUMINUM SHALL NOT BE EMBEDDED IN ANY CONCRETE.
R. SEE PLANS FOR DECK PROFILE AND ATTACHMENT REQUIREMENTS.

- VIII. MASONRY
A. HOLLOW CONCRETE BLOCK (MASONRY) UNITS SHALL BE LIGHTWEIGHT WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI ON THE NET AREA AND 1000 PSI ON THE GROSS AREA (FM = 1500PSI) AND SHALL CONFORM TO ASTM C-90.
B. ALL MORTAR FOR USE IN MASONRY SHALL CONFORM TO ASTM C-270, TYPE M OR S. ALL GROUT FOR USE IN MASONRY SHALL CONFORM TO ASTM C-476, MIN. 3000 PSI.
C. REINFORCING BARS TO MEET ASTM A-618, GRADE 60.
D. VERTICAL AND HORIZONTAL REINFORCING SHALL BE CONTINUOUS AND LAPPED A MINIMUM OF 72 BAR DIAMETERS.
E. HOLD VERTICAL BARS STRAIGHT AND TRUE AND ACCURATELY LOCATED IN WALL AS DETAILED. INSTALL REBAR POSITIONERS @ 4'-0" MAXIMUM THAT ARE DESIGNED TO HOLD REBAR IN PROPER LOCATION WITHIN THE GROUTED CELL.
F. PROVIDE #9 LADDER OR TRUSS TYPE JOINT REINFORCEMENT @ 16" OC FOR TYPICAL HORIZONTAL REINFORCEMENT.
G. ALL REINFORCED MASONRY COLUMN AND WALL SECTIONS REQUIRE DOWELS FROM FOOTING, SAME SIZE AND QUANTITY AS VERTICAL REINFORCEMENT IN COLUMN OR WALL.
H. GROUT FILL ALL CELLS AND ALL WALLS BELOW GRADE. SLUSH JOINT BETWEEN WYTHES.
I. LOW-LIFT GROUTING PROCEDURES SHALL BE USED FOR ALL FILLED-CELL MASONRY CONSTRUCTION. IF HIGH-LIFT GROUTING PROCEDURES ARE FOLLOWED, PROVIDE CLEANOUTS AT EACH LOCATION.
1. GROUT POURS SHALL NOT EXCEED 5 FEET PER LIFT, UNLESS CLEANOUTS ARE PROVIDED IN THE BOTTOM COURSE OF EACH 5 FOOT LIFT.
2. MECHANICALLY VIBRATE ALL LIFTS IN EXCESS OF 1 FOOT.
3. SHALL NOT BE STOPPED WITHIN 1-1/2" OF BED JOINT.
4. TOTAL GROUT POUR SHALL NOT EXCEED 24 FEET WHEN GROUTING THE CELLS OF HOLLOW MASONRY.
K. ALL CMU TO BE LAID IN RUNNING BOND PATTERN.
L. GLAZE ALL MASONRY UNITS UNTIL MASONRY AND GROUT HAVE BEEN ALLOWED TO SET FOR A MINIMUM OF 7 DAYS.
M. ALL MASONRY WALLS HAVE BEEN DESIGNED IN THE FINAL CONSTRUCTED CONFIGURATION ONLY ASSUMING FULL BRACING TOP, BOTTOM, AND/OR SIDE OF WALL. DURING CONSTRUCTION, THE CONTRACTOR SHALL BRACE ALL WALLS TO RESIST ERECTION AND LATERAL LOADS THAT MAY BE APPLIED PRIOR TO COMPLETION OF CONSTRUCTION.

- IX. OPEN WEB STEEL JOISTS AND JOIST GIRDERS
A. STEEL JOISTS, JOIST GIRDERS, AND BRIDGING SHALL BE DESIGNED, FABRICATED, AND ERECTED PER SJI RECOMMENDATIONS.
B. THE SJI LOAD TABLES SHALL BE TAKEN AS THE MINIMUM DESIGN LOADINGS FOR JOISTS AND JOIST GIRDERS. JOISTS AND JOIST GIRDERS SHALL ADDITIONALLY BE DESIGNED TO CARRY ALL OTHER LOADINGS INDICATED ON THE DRAWINGS.
C. BRIDGING INDICATED ON PLANS IS FOR PURPOSES OF ILLUSTRATING MISCELLANEOUS ATTACHMENTS AND DETAILS ONLY. GREATER OR FEWER LINES OF BRIDGING MAY BE REQUIRED BY SJI AND THESE REQUIREMENTS WILL SUPERSEDE THE CONTRACT DOCUMENTS. SEE PLANS AND DETAILS FOR SPECIAL BRIDGING AND BRACING REQUIREMENTS.
D. ALL JOISTS SHALL BE CAMBERED IN ACCORDANCE WITH SJI CRITERIA.
E. STEEL JOIST SPACING SHALL NOT EXCEED SPACING INDICATED ON DRAWINGS AND PLACEMENT OF JOISTS SHALL BE CAREFULLY COORDINATED WITH PARTITIONS AND WORK OF OTHER TRADES TO AVOID INTERFERENCES.
F. PROVIDE STABILITY BRACING FOR JOIST GIRDERS AS REQUIRED PER JOIST GIRDER MANUFACTURER.
G. ALL STEEL JOISTS, JOIST GIRDERS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED FOR ALL LOADS INDICATED ON THE DRAWINGS. WHERE UPLIFT LOADS ARE INDICATED, AN OPPOSING DEAD LOAD OF 10PSF MAY BE UTILIZED.
H. JOISTS AT OR NEAREST TO CENTERLINES OF MEMBERS ARE TO HAVE BOLTED CONNECTIONS. SEAT DEPTHS SHALL BE AS FOLLOWS. ANY DEVIATIONS MUST BE APPROVED PRIOR TO SUBMITTAL OF SHOP DRAWINGS.
1. LHS & DLH JOIST:
a. <18 SECTION # 5.0"
b. >18 SECTION # 7.5"

- X. METAL STUD FRAMING
A. DESIGN OF COLD-FORMED FRAMING MEMBERS AND CONNECTIONS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SHOW SIZE AND GAUGE OF MEMBERS AND ALL CONNECTIONS IN SHOP DRAWINGS AND SUBMIT WITH SHOP DRAWINGS THAT CORRESPONDS TO THE REQUIREMENTS OF AISI "SPECIFICATIONS FOR A PROFESSIONAL ENGINEER IN THE PROJECT STATE.
B. DESIGN, FABRICATION, AND ERECTION SHALL CONFORM TO AISI "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", LATEST EDITION. ALL METAL STUDS SHALL BE GALVANIZED.
C. ALL 33 MIL AND 43 MIL STUDS, JOISTS, TRACK, BRIDGING, END CLOSURES, AND ACCESSORIES SHALL BE OF STEEL AS REQUIRED BY THE REQUIREMENTS OF AISI "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" WITH A MINIMUM YIELD OF 33 KSI U.N.O. ALL 54 MIL AND THICKER MEMBERS SHALL HAVE A MINIMUM YIELD OF 50 KSI.
D. ALL MATERIAL AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING MEETING ASTM A 653.
E. UNLESS NOTED OTHERWISE, ALL SCREWS OR PINS SHALL BE NON-CORROSIVE NO. 8-18 (D=125") OR LARGER. DO NOT USE STAINLESS STEEL OR COPPER COATED FASTENERS.
F. UNLESS NOTED OTHERWISE, TRACKS SHALL BE THE SAME DEPTH AS STUDS OR JOISTS AND OF EQUAL OR THICKER GAUGE THAN STUDS OR JOISTS. TRACKS SHALL BE CONNECTED TO SUPPORT AT 16" OC. MAX. STUDS OR JOISTS SHALL BE CONNECTED TO TRACKS AT EACH SIDE.
G. THE QUANTITY OF STUDS AND JOISTS DISPLACED OR CUT FOR OPENING SHALL BE PLACED HALF ON EACH SIDE OF OPENING PER METAL STUD HEADER SCHEDULE ON THIS SHEET.
H. INSTALLATION OF CURTAIN WALL FRAMING SHALL ACCOMMODATE VERTICAL DISPLACEMENT OF THE PRIMARY STRUCTURE.
I. THE DESIGN OF SLIP TRACKS SHALL CONFORM TO GUIDELINES ESTABLISHED IN SSI TECHNICAL NOTE NO. 1 PUBLISHED JAN. 2001.
J. PROVIDE THE MANUFACTURER'S STANDARD TRACK, CLIP ANGLES, BRACING AND REINFORCEMENTS, FASTENERS, AND ACCESSORIES AS REQUIRED BY THE MANUFACTURER FOR THE APPLICATION INDICATED AND AS NEEDED TO PROVIDE A COMPLETE SYSTEM. UNLESS OTHERWISE NOTED, INSTALL THE FRAMING SYSTEM IN ACCORDANCE WITH THE MANUFACTURER'S SHOP INSTRUCTIONS AND RECOMMENDATIONS.
K. ALL EXTERIOR METAL WALL STUDS FOR ELEVATED FLOORS SHALL BE 6" GAUGE STUDS SPACED @ 16" ON CENTER. MINIMUM SECTION PROPERTIES FOR STUDS SHALL BE:
1. I = 1.30 IN^4
2. S = 0.66 IN.
3. FY = 33 KSI (MIN.)
L. WALL SHEATHING SHALL BE ATTACHED TO SUPPORTING FRAMING WITH NO. 10, FLAT-HEAD SELF-DRILLING TAPPING SCREWS WITH A MINIMUM HEAD DIAMETER OF 0.333 INCHES AT THE SPACING INDICATED BELOW. UNLESS NOTED OTHERWISE, THE SHEAR WALL SCHEDULE:
1. WALL PORTED PANEL EDGES AND FROM JOIST OF WALL 6" OC
2. CENTER OF PANEL 12" OC
M. PREPUNCE HOLES SHALL NOT BE LOCATED WITHIN 10 INCHES OF THE STUD SUPPORT LOCATIONS.
N. USE FLUX COVERED METAL ELECTRODES AWS E-6012, E-6013, OR E-7014 FOR WELDING STEEL STUDS. ALL WELDING SHALL BE PERFORMED IN ACCORDANCE AWS PROCEDURES. CONSULT MANUFACTURER FOR WELDING RECOMMENDATIONS AND PROPER ELECTRODE SELECTION. TOUCH UP WELDED AREAS WITH A ZINC RICH PAINT.

- XI. SUBMITTALS
A. THE GENERAL CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING FOR REVIEW. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND/OR ENGINEER AND HAVE THE ENGINEER'S SHOP DRAWING STAMP AFFIXED PRIOR TO FABRICATION. FABRICATION AND ERECTION SHALL BE FROM REVIEWED SHOP DRAWINGS. PLEASE ALLOW 10 BUSINESS DAYS FOR REVIEW.
B. A RECORD SET OF APPROVED SHOP DRAWINGS SHALL BE KEPT IN THE FIELD BY THE GENERAL CONTRACTOR.
C. ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO THE STRUCTURE OR ANY PART OF THE STRUCTURE DETAILED ON THE CONTRACT DOCUMENTS SHALL BE SUBMITTED IN WRITING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SUBMITTED FOR REVIEW DO NOT CONSTITUTE A WAIVER OF THE CONTRACTOR'S OBLIGATIONS AND PROPER ELECTRODE SELECTION. TOUCH UP WELDED AREAS WITH A ZINC RICH PAINT.
D. THE CONTRACTOR SHALL PREPARE A LIST AND SCHEDULE OF ALL STRUCTURAL SUBMITTALS PRIOR TO CONSTRUCTION.
E. THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR FOR THE ENGINEER'S REVIEW:
1. BAR JOISTS (1,3)
2. METAL AND FABRIC CANOPIES - CONNECTION TO BUILDING SHALL BE BY SUPPLIER (1,3)
3. STRUCTURAL STEEL, SHOP AND ERECTION DRAWINGS (1,3)
4. ROOF METAL DECK
5. CONCRETE MIX DESIGNS
6. PENETRATIONS IN BEAMS AND JOISTS
7. REINFORCING STEEL
8. EXTERIOR WINDOW WALL SYSTEM (2)
F. ITEMS MARKED (1) SHALL HAVE SHOP DRAWINGS SEALED BY A REGISTERED ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. ITEMS MARKED (2) SHALL BE SUBMITTED TO ENGINEER FOR OWNER'S RECORD ONLY AND WILL NOT HAVE THE ENGINEER'S SHOP DRAWING STAMP AFFIXED. ITEMS MARKED (3) SHALL HAVE DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED.
G. THE CONTRACTOR SHALL SUBMIT ONE SET OF REPRODUCIBLES AND TWO SETS OF PRINTS FOR ALL SHOP DRAWINGS SPECIFIED TO BE RETURNED BY THE ENGINEER.
2. THE OMISSION FROM THE SHOP DRAWINGS OF ANY MATERIALS REQUIRED BY THE CONTRACT DOCUMENTS TO BE FURNISHED SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY OF FURNISHING AND INSTALLING SUCH MATERIALS, REGARDLESS OF WHETHER THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED.
G. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREAFTER.

- XII. SPECIAL INSPECTION AND TESTING (FBC CHAPTER 17)
A. ALL TESTS AND INSPECTIONS SHALL BE PERFORMED BY AN INDEPENDENT TESTING AND INSPECTION AGENCY. THE SPECIAL INSPECTOR FROM THIS TESTING AGENCY SHALL OBSERVE THE WORK FOR CONFORMANCE TO THE DESIGN DRAWINGS AND SPECIFICATIONS.
B. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE ENGINEER OR ARCHITECT OF RECORD, AND ALL OTHER DESIGNATED INDIVIDUALS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. THEN, IF NOT CORRECTED, TO THE PROPER DESIGN AUTHORITY AND TO THE BUILDING OFFICIAL.
C. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS, SPECIFICATIONS, SOILS REPORT AND APPLICABLE WORKMANSHIP PROVISIONS OF THE INTERNATIONAL BUILDING CODE.
D. MASONRY WORK SHALL HAVE SPECIAL INSPECTION AS DEFINED BY THE AFOREMENTIONED BUILDING CODE.
E. THE FOLLOWING ITEMS MARKED "X" REQUIRE SPECIAL INSPECTIONS. (REFER TO AFOREMENTIONED BUILDING CODE FOR ADDITIONAL INFORMATION)

- PROTECTION OF OPENINGS. (2017 FBC)
GLAZED OPENINGS IN BUILDINGS LOCATED WITHIN 1 MILE OF THE COASTAL MEAN HIGH WATER LINE WHERE THE ULTIMATE DESIGN WIND SPEED VULT IS 130 OR GREATER OR IN AREAS WHERE THE ULTIMATE DESIGN WIND SPEED VULT IS 140 MPH OR GREATER SHALL BE PROTECTED FROM WIND-BORNE DEBRIS. GLAZED OPENING PROTECTION FOR WIND-BORNE DEBRIS SHALL MEET THE REQUIREMENTS OF SSTD 12, ASTM E 1886 AND ASTM E 1996 WITH MODIFICATIONS PER SECTION 1609.1.2.4 OF THE FBC, OR TAS 201, 202 AND 203 OR AAMA 506 REFERENCED THEREIN.
1. GLAZED OPENINGS LOCATED WITHIN 30 FEET OF GRADE SHALL MEET THE REQUIREMENTS OF THE LARGE MISSILE TEST OF ASTM E 1996.
2. GLAZED OPENINGS LOCATED MORE THAN 30 FEET ABOVE GRADE SHALL MEET THE PROVISIONS OF THE SMALL MISSILE TEST OF ASTM E 1996.

- FOR RISK CATEGORY II BUILDINGS AND III BUILDINGS, EXCEPT HEALTH CARE FACILITIES, THE ULTIMATE DESIGN WIND SPEED VULT SHALL BE BASED ON FIGURE 1609A AND FOR RISK CATEGORY IV BUILDINGS AND RISK CATEGORY III HEALTH CARE FACILITIES, ULTIMATE DESIGN WIND SPEED VULT SHALL BE BASED ON FIGURE 1609B.
1. GLAZED OPENINGS LOCATED WITHIN 30 FEET OF GRADE SHALL MEET THE REQUIREMENTS OF THE LARGE MISSILE TEST OF ASTM E 1996.
2. GLAZED OPENINGS LOCATED MORE THAN 30 FEET ABOVE GRADE SHALL MEET THE PROVISIONS OF THE SMALL MISSILE TEST OF ASTM E 1996.
FOR RISK CATEGORY II BUILDINGS AND III BUILDINGS, EXCEPT HEALTH CARE FACILITIES, THE ULTIMATE DESIGN WIND SPEED VULT SHALL BE BASED ON FIGURE 1609A AND FOR RISK CATEGORY IV BUILDINGS AND RISK CATEGORY III HEALTH CARE FACILITIES, ULTIMATE DESIGN WIND SPEED VULT SHALL BE BASED ON FIGURE 1609B.

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NO. DATE DESCRIPTION
FILE NUMBER 78890009
PROJECT MANAGER
PROFESSIONAL
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CHECKED BY
Checker

GENERAL NOTES
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