GENERAL LOADING NOTES

1. BUILDING CODE - INTERNATIONAL BUILDING CODE/2015.
   LOADS CALCULATED USING ASCE 7-10.

2. LOADS -

LOCATIONS	LIVE LOAD UNIFORM	DEAD LOAD UNIFORM*
SUPERIMPOSED ROOF	20 PSF	22PSF
SLAB ON GRADE	100 PSF	-----
GROUND SNOW LOAD, PG	5 PSF	-----
FLAT-ROOF SNOW LOAD, PF	8.5 PSF	-----

SNOW EXPOSURE FACTOR; Ce = 1.0
SNOW LOAD IMPORTANCE FACTOR; Is = 1.0
THERMAL FACTOR; Ct = 1.0

*UNIFORM DEAD LOAD IS ADDITIVE TO ACTUAL STRUCTURAL WEIGHTS.

3. LATERAL LOADS -

A. WIND LOAD (ANALYTICAL DESIGN WIND PROCEDURE)

BASIC WIND SPEED, 3-SECOND GUST ..... V = 115 MPH
WIND IMPORTANCE FACTOR ..... Iw = 1.00
WIND EXPOSURE C
ENCLOSURE CLASSIFICATION ..... ENCLOSED
INTERNAL PRESSURE COEFFICIENT ..... GCpi = +/- 0.18
WIND PRESSURES -
MWFRS:
Pv = 32.8 PSF * END ZONE
Pv = 31.4 PSF * INTERIOR ZONE
Pparapets = 66.9 PSF *
Proof = -13.5 PSF END ZONE h < 30'
Proof = -8 PSF INTERIOR ZONE h < 30'

C&C:
Proof, H < 30'
ZONE 1, PS = -26.9 / 10 P.S.F.
ZONE 2, PS = -31.9 / 10 P.S.F.
ZONE 3, PS = -31.9 / 10 P.S.F.
Pwalls, H < 30'
ZONE 4, PS = -25.8 / 23.5 P.S.F.
ZONE 5, PS = -29.2 / 23.5 P.S.F.
Pparapets = 39.4 PSF *

* PRESSURE INDICATES COMBINED WINDWARD AND LEEWARD PRESSURE.
DESIGN SHEAR VALUE IN N-S DIRECTION= 124 KIPS.
DESIGN SHEAR VALUE IN E-W DIRECTION= 79.5 KIPS.

B. SEISMIC LOAD

SEISMIC IMPORTANCE FACTOR ..... Ie = 1.0
SPECTRAL RESPONSE ACCELERATIONS ..... Ss = 0.337g
..... S1 = 0.114g
SOIL SITE CLASS ..... D
SPECTRAL RESPONSE COEFFICIENTS ..... Sds = 0.344g
..... Sd1 = 0.179g
SEISMIC DESIGN CATEGORY C
BASIC SEISMIC-FORCE-RESISTING SYSTEM ... ORDINARY STEEL CONCENTRICALLY BRACED FRAMES
DESIGN BASE SHEAR (N-S DIRECTION) ..... 80 KIPS
DESIGN BASE SHEAR (E-W DIRECTION) ..... 78 KIPS
SEISMIC RESPONSE COEFFICIENT ..... Cs = 0.106
RESPONSE MODIFICATION FACTOR ..... R = 3.25
ANALYSIS PROCEDURE UTILIZED ..... EQUIVALENT LATERAL FORCE PROCEDURE

4. GEOTECHNICAL REPORT -

A. GEOTECHNICAL INVESTIGATION PERFORMED BY: TERRACON CONSULTANTS, INC. ATLANTA, GEORGIA

B. REPORT NUMBER : 49175005
C. DATE OF REPORT : MARCH 2, 2017

GENERAL SOIL PREPARATION AND FOUNDATION NOTES

1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT PREPARED FOR THIS PROJECT. A COPY OF THIS REPORT IS AVAILABLE FOR INSPECTION IN THE SPECIFICATIONS.

2. ALL EXCAVATION, FILL, COMPACTION AND GRADING OF THE SITE SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS AND THE RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL REPORT(S).

3. AT THE LOCATIONS WHERE UTILITY TRENCHES CROSS THE BUILDING LINE, 5' OF EACH TRENCH CENTERED ON THE BUILDING LINE SHALL BE BACKFILLED WITH A COMPACTED, LOW-PERMEABILITY CLAY.

4. SPREAD FOOTINGS FOUNDATIONS -

A. DESIGN BEARING PRESSURE IS 2500 PSF (NET) FOR FOUNDATIONS BEARING ON COMPACTED ENGINEERED FILL (REFER GEOTECH), BOTTOM OF FOOTING SHALL BE A MINIMUM OF 1'-6" BELOW FINAL EXTERIOR GRADE. BEARING MATERIALS SHALL BE VERIFIED BY A LICENSED GEOTECHNICAL ENGINEER.

B. ALL FOUNDATIONS ARE DESIGNED AND DETAILED WITH FORMED SIDES. IF THE CONTRACTOR ELECTS TO USE EARTH FORMED SIDES, ONE INCH SHALL BE ADDED TO EACH SIDE TO PROVIDE ADEQUATE COVER OVER THE REINFORCING AT THE CONTRACTOR'S EXPENSE.

C. REINFORCING SHALL BE SUPPORTED FROM ABOVE OR WITH 3" SLAB BOLSTER WITH PLATE (CONT. BOTTOM PLATE) AT 4'-0" MAXIMUM CENTERS FOR ALL FOUNDATION REINFORCING.

D. MOISTURE CONTENT IN FOOTING EXCAVATIONS SHALL BE MAINTAINED UNTIL FOOTING IS PLACED. FOOTINGS SHALL BE POURED AS SOON AS PRACTICAL AFTER EXCAVATIONS ARE COMPLETED.

5. DO NOT BACKFILL FOUNDATION WALLS UNTIL THE RETAINING SLABS OR ADEQUATE BRACING ARE IN PLACE. ALL BACKFILL SHALL BE PLACED AND COMPACTED IN ACCORDANCE WITH THE SPECIFICATION.

6. EXTERIOR SLABS SHALL SLOPE AWAY FROM THE STRUCTURE A MINIMUM OF 1/4" PER FOOT UNLESS OTHERWISE NOTED.

CAST-IN-PLACE CONCRETE NOTES

1. THE CONCRETE REQUIREMENTS ARE -

A. 28 DAY CONCRETE COMPRESSIVE STRENGTHS -

LOCATION	MIN. F'C(PSI)	MAX. SLUMP(IN)	MAX. W/C RATIO	MAX. AGGREGATE ASTM D448
FOUNDATION	3500	5	0.55	No. 57
INTERIOR SLAB ON GRADE	NOT LESS THAN 3500	4	0.45	No. 67

B. CONCRETE CLEAR COVER OVER REINFORCING SHALL BE IN ACCORDANCE WITH ACI 318-02, LISTED BELOW, UNLESS OTHERWISE NOTED -

LOCATION	CLEAR COVER (INCHES)
CAST AGAINST EARTH	3
EXPOSED TO EARTH OR WEATHER #6 AND LARGER	2
EXPOSED TO EARTH OR WEATHER #5 AND SMALLER	1 1/2
SLABS NOT EXPOSED TO WEATHER	1
SLABS ON GRADE (COVER FROM TOP OF SLAB)	1 1/2

2. CONCRETE REINFORCING SHALL MEET THE FOLLOWING -

A. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A615 GRADE 60. REINFORCING BARS REQUIRED TO BE WELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A706 GRADE 60. WELDING OF REINFORCING OTHER THAN SPECIFIED IS PROHIBITED.

B. ALL REINFORCING SHALL BE CONTINUOUS. CONTINUOUS BARS SHALL LAP IN ACCORDANCE WITH TABLE "A" ON SHEET S0.1, UNLESS OTHERWISE NOTED.

C. PROVIDE CORNER BARS IN OUTSIDE FACE OF ALL GRADE BEAMS AND WALLS EQUAL IN SIZE AND SPACING TO MAIN HORIZONTAL REINFORCING. EXTEND INSIDE FACE REINFORCING OF ALL GRADE BEAMS AND WALLS TO OUTSIDE FACE AND BEND TO A STANDARD 90 DEGREE HOOK.

D. PROVIDE 2 - #5 EACH SIDE OF EACH OPENING THRU WALLS OR SLABS UNLESS NOTED OTHERWISE. BARS TO EXTEND 2'-0" PAST OPENING.

E. WELDED WIRE FABRIC SHALL CONFORM TO THE REQUIREMENTS OF ASTM A185.

F. WELDED WIRE FABRIC MUST HAVE END LAPS AND EDGE LAPS OF ONE FULL MESH AND SHALL BE HELD IN PLACE BY WIRING ALL LAPS SECURELY TOGETHER.

G. WELDED WIRE FABRIC SHALL BE SUPPLIED IN FLAT SHEETS ONLY.

H. SHOP DRAWINGS SHALL BE SUBMITTED WITH REINFORCING STEEL DETAILED IN ACCORDANCE WITH ACI 315-02.

3. FORMING AND EMBEDMENT SHALL MEET THE FOLLOWING -

A. ALL FOUNDATIONS ARE DESIGNED AND DETAILED WITH FORMED SIDES. IF THE CONTRACTOR ELECTS TO USE EARTH FORMED SIDES, ONE INCH SHALL BE ADDED TO EACH SIDE TO PROVIDE ADEQUATE COVER OVER THE REINFORCING AT THE CONTRACTOR'S EXPENSE.

B. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4" INSIDE FORMS OR TOOLED TO 3/4" RADIUS ON SLABS UNLESS OTHERWISE NOTED.

C. SLABS ON GRADE SHALL HAVE CONSTRUCTION JOINTS AND CONTROL JOINTS (SAWED JOINTS) LOCATED AS SHOWN ON THE DRAWINGS. CONTRACTOR SHALL LOCATE SLAB JOINTS ON SHOP DRAWINGS.

D. AT THE CONTRACTOR'S OPTION, EITHER THE SAWED OR KEYED CONSTRUCTION JOINT MAY BE USED. THE KEYED JOINT SHALL BE USED TO TERMINATE ANY PLACEMENT.

E. WHERE NECESSARY, VERTICAL CONSTRUCTION JOINTS SHALL BE LOCATED WITHIN THE CENTER ONE-THIRD OF THE SPAN. ALL JOINTS SHALL BE THOROUGHLY CLEANED AND PURPOSELY ROUGHENED TO 1/4"PRIOR TO PLACING ADJACENT CONCRETE. JOINTS IN EXPOSED CONCRETE SHALL BE USED WITH A MAXIMUM SPACING OF 50'.

F. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL FORMING, TEMPORARY BRACING AND SHORING.

G. NO ALUMINUM SHALL BE EMBEDDED IN CONCRETE. CONDUITS AND PIPING EMBEDDED IN CONCRETE SHALL BE SPACED A MINIMUM OF FOUR DIAMETERS AND THE OUTSIDE DIAMETER SHALL BE LESS THAN 30% OF THE MEMBER THICKNESS PLACED BETWEEN LAYERS OF REINFORCING.

4. CURING FOR CONCRETE SURFACES NOT IN CONTACT WITH FORMS. ONE OF THE FOLLOWING PROCEDURES SHALL BE APPLIED IMMEDIATELY AFTER COMPLETION OF PLACEMENT AND FINISHING -

A. PONDING OR CONTINUOUS SPRINKLING.

B. APPLICATION OF ABSORPTIVE MATS OR FABRIC KEPT CONTINUOUSLY WET.

C. APPLICATION OF WATERPROOF SHEET MATERIALS, CONFORMING TO"SPECIFICATIONS FOR WATERPROOF SHEET MATERIALS FOR CURING CONCRETE" (ASTM C 171).

D. APPLICATION OF A CURING COMPOUND CONFORMING TO "SPECIFICATIONS FOR LIQUID MEMBRANE-FORMING COMPOUNDS FOR CURING CONCRETE" (ASTM C 309). THE COMPOUND SHALL BE APPLIED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER IMMEDIATELY AFTER ANY WATER SHEEN WHICH MAY DEVELOP AFTER FINISHING HAS DISAPPEARED FROM THE CONCRETE SURFACE. IT SHALL NOT BE APPLIED TO ANY SURFACE AGAINST WHICH ADDITIONAL CONCRETE IS TO BE BONDED UNLESS IT IS PROVEN THAT THE CURING COMPOUND WILL NOT PREVENT BOND, OR UNLESS POSITIVE MEASURES ARE TAKEN TO REMOVE IT COMPLETELY FROM AREAS TO RECEIVE BONDED APPLICATIONS. CURING SHALL BE CONTINUED AT LEAST 7 DAYS.

E. PERFORM ADEQUATE SLAB MOISTURE EMISSION TESTS PER ASTM F 1869-04 TO CONFORM THAT EMISSION LEVELS MEET THE FLOORING MANUFACTURER'S SPECIFICATION BEFORE PLACING THE COVERING. THESE TESTS SHOULD NOT BE CONDUCTED UNTIL THE BUILDING IS CLOSED IN AND TEMPORARY HVAC EQUIPMENT HAS SUFFICIENTLY TO CREATE TEMPERATURE/ HUMIDITY ENVIRONMENT THAT IS REPRESENTATIVE OF THE TYPICAL CONDITIONS THE FLOORING WILL EXPERIENCE.

5. ALL VAPOR RETARDERS SHALL BE STEGO WRAP 15-MIL CLASS A VAPOR RETARDER OR APPROVED EQUAL UNLESS MORE STRINGENT REQUIREMENTS ARE SHOWN IN SPECIFICATIONS 6 MIL VISQUEEN IS NOT APPROVED EQUAL.

BRICK LINTEL NOTES

PROVIDE LINTEL ANGLES OVER ALL OPENINGS OR RECESSES IN BRICK MASONRY WALLS AND PARTITIONS

PROVIDE AS FOLLOWS FOR 4" NOMINAL THICKNESS OF WALL, UNLESS NOTES

LINTEL SIZE	OPENING WIDTH
(1)∠ 3 1/2X3 1/2X1/4	0'-0" TO 5'-0"
(1)∠ 5X3 1/2X3/8 (LLV)	5'-1" TO 8'-6"
(1)∠ 6X4X3/4 (LLV)	8'-7" TO 12'-6"

LINTELS SHALL HAVE 1" OF BEARING AT EACH END FOR EVERY FOOT OF SPAN, WITH A MINIMUM OF 4", UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL NOTES

1. STRUCTURAL STEEL SHALL MEET THE FOLLOWING REQUIREMENTS UNLESS OTHERWISE NOTED ON THE DRAWINGS -

TYPE	ASTM	GRADE	FY(MIN) (PSI)
STRUCTURAL SHAPES (EXCEPT ANGLES)	A992	50	50,000
STEEL ANGLES, PLATES & RODS	A36	----	36,000
STRUCTURAL TUBING	A500	B	46,000
ANCHOR BOLTS	F1554	55	55,000
STRUCTURAL BOLTS	A325	----	----

2. STRUCTURAL STEEL CONNECTIONS -

A. ALL CONNECTIONS NOT DETAILED OR OTHERWISE NOTED SHALL BE DESIGNED BY THE FABRICATOR USING ASD METHODS. SHOP DRAWINGS AND CONNECTION CALCULATIONS SHALL BE SUBMITTED BEARING THE SEAL OF AN ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. NON-COMPOSITE BEAMS SHEAR CONNECTIONS SHALL BE STANDARD AISC CONNECTIONS AND SHALL BE DESIGNED FOR THE SPAN USING THE "ALLOWABLE UNIFORM LOAD IN KIPS FOR BEAMS LATERALLY SUPPORTED" LOAD DIVIDED BY 2, FOUND IN SECTION 2, "ALLOWABLE LOADS ON BEAMS" OF THE AISC NINTH EDITION MANUAL PLUS ANY AXIAL LOAD INDICATED ON THE DRAWINGS. COMPOSITE BEAMS SHEAR CONNECTIONS SHALL BE STANDARD AISC CONNECTIONS AND SHALL BE DESIGNED FOR THE LOADS INDICATED ON THE DRAWINGS PLUS ANY AXIAL LOAD INDICATED ON THE DRAWINGS. DESIGN SHALL BE BASED ON BEARING TYPE BOLTED CONNECTIONS WITH BOLTS "SNUG TIGHT" PER AISC. WELDED TYPE CONNECTIONS SHALL BE APPROVED FOR USE PRIOR TO SUBMITTAL OF SHOP DRAWINGS.

B. ALL MOMENT CONNECTIONS INDICATED (▶—) ON THE DRAWINGS SHALL DEVELOP THE FULL MOMENT CAPACITY OF THE CONNECTED MEMBERS. MOMENT CONNECTIONS SHALL BE WELDED FLANGES WITH BOLTED SHEAR TABS USING FRICTION TYPE PRETENSIONED BOLTS. BOLTED MOMENT CONNECTIONS USING FRICTION TYPE PRETENSIONED BOLTS SHALL BE APPROVED FOR USE PRIOR TO SUBMITTAL OF SHOP DRAWINGS. DIRECT TENSION INDICATOR (DTI) WASHERS SHALL BE USED TO INSURE PROPER TENSIONING.

C. ALL BOLTED CONNECTIONS REQUIRING FRICTION TYPE PRETENSIONED BOLTS WILL BE INDICATED AS (F.T.). DIRECT TENSION INDICATOR (DTI) WASHERS SHALL BE USED TO INSURE PROPER TENSIONING.

D. ALL WELDING SHALL BE IN ACCORDANCE WITH LATEST AWS CODE. ALL WELDS SHALL USE E70XX ELECTRODES.

E. CONNECTION BOLTS SHOWN IN DRAWINGS ARE 3/4" DIAMETER A325 BEARING TYPE "N" UNLESS NOTED OTHERWISE.

3. STEEL FRAMING CONNECTIONS SHALL BE TIGHTENED AND COLUMNS SHALL BE PLUMBED AND GROUTED BELOW THE BASE PLATE IN PLACE BEFORE DECKING IS ATTACHED TO FRAMING.

4. NO HOLES SHALL BE CUT THRU STEEL BEAMS IN FIELD UNLESS APPROVED IN WRITING BY THE ENGINEER OF RECORD.

STEEL JOISTS NOTES

1. FABRICATION AND ERECTION SHALL CONFORM TO CURRENT S.J.I. STANDARD SPECIFICATIONS AND LOAD TABLES.

2. BRIDGING (NO. OF ROWS AND SIZE) SHALL BE AS REQUIRED BY THE S.J.I. AND/ OR JOIST MFG. ALL BRIDGING SHALL BE PERMANENTLY INSTALLED BEFORE CONSTRUCTION LOADS ARE APPLIED. EACH LINE OF BRIDGING SHALL BE ANCHORED AT ENDS, WALLS, OR BEAMS.

3. NAME OF JOIST MFG. SHALL BE ON JOIST SHOP DRAWINGS.

4. PROVIDE (2) 1/2" ERECTION BOLTS AT JOISTS AT EACH OCCUPIED BAY. COLUMNS. BOLTS MAY BE OMITTED WHERE JOISTS PARALLEL TO THE JOISTS FRAME INTO SUPPORTING BEAMS AT COLUMNS.

5. ALL JOISTS SHALL BE WELDED TO SUPPORTING MEMBERS WITH 1/8"x2" LONG FILLET WELDS EACH SIDE ONLY UNLESS NOTED OTHERWISE.

6. ALL CONCENTRATED LOADS APPLIED TO JOISTS OR JOIST GIRDERS MUST BE PLACED AT THE PANEL POINT OF THE LOADED CHORD, OR ADDITIONAL WEB MEMBERS (ANGLE 2½"x2½" MIN.) MUST RUN FROM THE POINT OF LOADING TO THE NEAREST PANEL POINT ON THE OPPOSITE CHORD.

7. STEEL JOIST LAYOUT SHOWN ON PLANS IS INDICATIVE ONLY. JOISTS MAY BE STAGGERED AS REQ'D. TO PROVIDE A MINIMUM OF 2 1/2" JOIST BEARING FOR JOISTS BEARING ON EITHER SIDE OF BEAMS.

8. STEEL JOISTS & JOIST GIRDERS SHALL BE DESIGNED BY THE JOIST MANUFACTURER TO RESIST A NET UPLIFT OF 10 P.S.F. THIS DOES NOT INCLUDE THE WEIGHT OF THE JOISTS OR JOIST GIRDERS (OR USE PLAN OF NET PRESSURES).

9. MANUFACTURER SHALL DESIGN JOIST AND JOIST GIRDERS IN ACCORDANCE WITH THE FIRE DESIGN REQUIREMENTS IN ORDER TO ACHIEVE THE FIRE RATING SPECIFIED IN ARCHITECTURAL DRAWINGS.

10. JOIST MANUFACTURER SHALL FURNISH ENGINEERING CALCULATIONS TO THE S.E.R. FOR APPROVAL PRIOR TO FABRICATION.

STEEL DECK NOTES

1. STEEL ROOF DECK SHALL COMPLY WITH THE STEEL DECK INSTITUTE REQUIREMENTS.

2. CONTRACTOR SHALL TAKE NECESSARY PRECAUTIONS TO PREVENT DAMAGE TO THE DECK DURING CONSTRUCTION. DAMAGED DECK MUST BE REPLACED.

3. DECK SPANS EXCEEDING THOSE IN THE "CONSTRUCTION LOAD TABLES" PUBLISHED BY THE S.D.I. MAY REQUIRE SPECIAL PRECAUTIONS DURING CONSTRUCTION.

4. END LAPS SHALL BE 2" MINIMUM AND OCCUR AT A SUPPORT.

5. FRAME ALL OPENINGS THRU DECK OVER 10" IN EITHER DIMENSION WITH ANGLE 3x3x1/4" EACH SIDE OF OPENING.

6. NO POINT LOADS (SUCH AS DUCT OR PIPE HANGERS, GRID CEILING HANGERS,ETC.) SHALL BE PLACED ON OR ATTACHED TO THE DECK.

7. STEEL ROOF DECK SHALL BE 1 1/2" DEEP, 20 GA. (1.5B20) WIDE RIB METAL DECKING. DECKING SHALL HAVE A MINIMUM Ip OF 0.201 IN4(FT), A MINIMUM Sp OF 0.234 IN3(FT),AND A MINIMUM Sn OF 0.247 IN3(FT). DECKING SHALL RECEIVE FINISH PER SPECIFICATION. DECKING SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SUPPORTS. EACH DECK UNIT SHALL BE ATTACHED TO SUPPORTING MEMBERS AND ADJACENT PANELS AS FOLLOWS -

	36" PANEL
NO. OF END AND SUPPORT 5/8" PUDDLE WELDS	4
NO. OF EQUALLY SPACED #10 TEK SCREWS BETWEEN SUPPORTS IN SIDE LAPS (OR)	4
NO. OF END AND SUPPORT #12 TEK SCREWS	5
NO. OF EQUALLY SPACED #10 TEK SCREWS BETWEEN SUPPORTS IN SIDE LAPS (OR)	5
NO. OF END AND SUPPORT SIMPSON STRUCTURAL SCREW XLQ114T1224	4
NO. OF EQUALLY SPACED SIMPSON SIDE LAP FASTENER XQ1S1214 BETWEEN SUPPORTS IN SIDE LAPS	5

8. THE DECK SHALL BE SUPPORTED BY STRUCTURAL STEEL BEAMS OR JOISTS AT EDGES WITH A CONTINUOUS (ANGLE 21/2 "x21/2"x3/16") ALONG THE BOUNDARIES WELDED/ANCHORED TO THE ANGLE UNLESS OTHERWISE NOTED.

COLD FORMED METAL FRAMING NOTES (CFMF)

1. ALL EXTERIOR WALLS SHALL BE CONSTRUCTED OF STEEL "C" STUDS OF THE SIZE SHOWN IN THE PLANS AND SHALL CONFORM AISI SPECIFICATIONS. MINIMUM SECTION PROPERTIES SHALL BE AS FOLLOWS:

SIZE (IN.X GA.)	IX-X (IN.4)	SX-X (IN.3)	RX-X (IN.)	IY-Y (IN.4)	RY-Y (IN.)
3 5/8"X16	0.873	0.482	1.483	0.154	0.604
3 5/8"X18	0.710	0.392	1.446	0.127	0.611
6x16	2.860	0.954	2.268	0.184	0.570
6X16 TRACK	2.267	0.742	2.183	0.054	0.336
12X12	26.966	4.494	4.137	0.331	0.486
8X16	5.740	1.435	2.927	0.195	0.539
6X18	2.317	0.772	2.277	0.148	0.577
6X12	4.824	1.614	2.234	0.287	0.544
8X14	7.088	1.772	2.913	0.235	0.530

2. MINIMUM WIDTH OF THE STUDS SHALL BE 1 5/8 ", AND THE LIP OF THE "C" PORTION SHALL BE A MINIMUM OF 1/2".

3. STUD TRACK SHALL BE 18 GA. MINIMUM AND SHALL BE ANCHORED AS FOLLOWS:

TO STEEL
HILTI DX-35, ESD DRYWALL TRACK PINS @ 24" O.C. (OR APPROVED EQUAL)

TO CONCRETE
RED HEAD (RAMSET) LARGE DIAMETER TAPCON (LDT) ANCHORS 3/8" ø WITH 21/2" EMBEDMENT@ 32" O.C. (OR APPROVED EQUAL)

4. STEEL STUDS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. HORIZONTAL BRIDGING SHALL BE PLACED AT 4'-0" O.C. OR AS PER MANUFACTURER'S RECOMMENDATION IF LESS THAN 4'-0

MISCELLANEOUS NOTES

1. REPRODUCTION OF STRUCTURAL CONTRACT DOCUMENTS AS SHOP DRAWINGS, ERECTION PLANS, FABRICATION PLANS OR DETAILS IS NOT AUTHORIZED AND IS SUBMITTED, WILL BE REJECTED WITHOUT CHECKING. ANY REPRODUCTIONS OR ALL OF THE CAD FILES FOR THE LIMITED PURPOSE OF ASSISTING CONTRACTOR'S PREPARATION OF SHOP DRAWINGS FOR SUBMITTAL UNDER THE CONSTRUCTION CONTRACT MAY BE PURCHASED FROM SAI, PLC. UNDER A STANDARD FORM OF AGREEMENT FOR $1,000. FILES WERE CREATED IN AUTOCAD VERSION 2004.

2. CONCRETE PADS FOR MECHANICAL AND ELECTRICAL EQUIPMENT ON FLOORS SHALL BE 3 1/2" HIGH UNLESS OTHERWISE NOTED AND REINFORCED WITH #3 BARS ON 12" CENTERS EACH WAY, 1 1/2" FROM TOP OF SLAB. WHEN THE PAD EXCEEDS 10" IN THICKNESS, REINFORCE WITH #3 BARS ON 12" CENTERS EACH WAY TOP AND BOTTOM. ANCHOR PAD TO SLAB WITH #4 DOWELS AT 24" EACH WAY CAST OR EPOXIED INTO SUPPORTING SLAB.

3. CONTRACTOR SHALL SUPPLY ALL ITEMS FOR ATTACHING MECHANICAL AND ELECTRICAL EQUIPMENT TO THE BUILDING STRUCTURE TO RESIST ALL LOADS INCLUDING SEISMIC FORCES. ATTACHMENT SHALL BE MADE SO AS NOT TO OVERSTRESS STRUCTURAL MEMBERS. COORDINATE THE ATTACHMENTS AND LOCATIONS OF THE EQUIPMENT WITH THE STRUCTURAL SHOP DRAWINGS. REFER TO THE MECHANICAL AND ELECTRICAL DRAWINGS FOR ADDITIONAL REQUIREMENTS.

4. SUBSTITUTION OF EXPANSION ANCHORS FOR EMBEDDED ANCHORS SHOWN ON THE DRAWINGS WILL NOT BE PERMITTED UNLESS APPROVED BY THE ENGINEER IN ADVANCE.

5. THE CONTRACTOR SHALL PROVIDE THE FOLLOWING ADDITIONAL SERVICES -

A. VERIFICATION OF ALL DIMENSIONS, ELEVATIONS, OPENING SIZES, AND MECHANICAL EQUIPMENT WEIGHTS PRIOR TO STARTING WORK.

B. VERIFICATION OF ALL DIMENSIONS AND MEMBER SIZES RELATING TO EXISTING BUILDING.

C. VERIFICATION OF ALL FLOOR DEPRESSIONS AND OFFSETS WITH ARCHITECTURAL DRAWINGS.

D. REMOVE ALL ABANDONED FOUNDATIONS, UTILITIES, PIPELINES, ETC. THAT INTERFERE WITH NEW CONSTRUCTION.

E. REVIEW AND APPROVE ALL SHOP DRAWINGS PRIOR TO SUBMITTAL NOTING CHANGES MADE WHICH DO NOT COMPLY WITH DESIGN DRAWINGS.

F. PROVIDE TEMPORARY BRACING AND SHORING AS REQUIRED FOR STABILITY DURING CONSTRUCTION.

6. PLANS, SECTIONS, AND DETAILS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.

7. SEE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS, SLEEVES, CURBS, PADS, INSERTS, ETC.NOT SHOWN ON STRUCTURAL DRAWINGS. BEFORE FABRICATION OF MATERIALS, COORDINATE WITH MECHANICAL AND ELECTRICAL EQUIPMENT REQUIREMENTS.

8. CONSTRUCTION DOCUMENTS CONSIST OF THESE DRAWINGS AND A SEPARATE BOOK OF SPECIFICATIONS. THE DRAWINGS AND SPECIFICATIONS ARE COMPLEMENTARY, NEITHER IS MEANT TO STAND ALONE FOR ANY PORTION OF THE WORK DESCRIBED HEREIN. ANY CONFLICT BETWEEN DRAWINGS AND SPECIFICATIONS SHALL BE REPORTED IMMEDIATELY TO THE ARCHITECT.

ABBREVIATIONS :

1. C# = COLUMN MARK; REFER SCHEDULE ON SHEET S0.1
2. F# = FOOTING MARK; REFER SCHEDULE ON SHEET S0.1
3. SC# = STUB COLUMN MARK; REFER SCHEDULE ON SHEET S0.1
4. ▨ = RECESS FOR FREEZERS; REFER DETAIL 13, SHEET S3.0
5. D.B. = DIAGONAL BRACE; REFER ELEVATIONS 1, 10 & 13, SHEET S
6. T.O.F. = TOP OF FOOTING ELEVATION
7. A.F.F = ABOVE FINISH FLOOR
8. B.F.F = BELOW FINISH FLOOR
9. (H#) = HEADER BEAM; REFER SCHEDULE ON SHEET S0.1
10. J.B. = JOIST BEARING ELEVATION
11. (X WEIGHT) = ROOF TOP MECHANICAL UNIT FOR OPENING UNDER UNIT REFER DETAIL ON SHEET S4.0, REFER TO ROOF EQUIPMENT PLAN ON S2.1 FOR OPENING SIZES.
12. ▶— = MOMENT CONNECTION: REFER DETAIL 6, SHEET S4.0
13. REFER SHEET S4.0 FOR TYP. ROOF FRAMING SECTIONS.
14. C.J. = CONTROL JOINT; REFER DETAIL 9, SHEET S3.0
15. F.D. = FLOOR DRAINS; CONTRACTOR TO COORDINATE FLOOR DRAIN LOCATIONS WITH ARCH/ MEP DWGS; REFER DETAIL 15, SHEET S3.0
16. F.F.E. = FINISHED FLOOR ELEVATION
17. B.F.F. = BOTTOM OF STEEL ABOVE F.F.E.
18. SP# = SPECIAL JOIST; REFER SHEET S7.0 FOR LOADING.
19. T.O.S. = TOP OF STEEL

Pascal Aughtry & Associates,PC
425 483.3484
Fax 425.483.3483
937 East Britton Road
Oklahoma City, OK 73114
paseatech.com

GEORGIA REGISTERED NO. PE 028341 PROFESSIONAL ENGINEER STAM SUNDAR MANNAM
05/24/2018
SAI JOB #18405

A NEW TRAVEL STOP
STORE No. 735
CALHOUN, GA

Love's Travel Stops

Revisions:	
No.	Date

Project No.: LVS18735
Date: 01/18/2018
Sheet No.:
S0.0
GENERAL NOTES
OF:

Z:\JOBS\Jobs Current\01 - LOVES TRAVEL STOPS AND YARD BUILDING\TRAVEL STOPS\18405-Trovel Stop-Cochoun, GA\LAT-ENG\ 18405-S0.0.dwg 05-24-2018 - 8:38cm