

GENERAL NOTES:

- (THESE SPECIFICATIONS ARE IN ADDITION TO AND DO NOT EXCLUDE ANY FOUND IN THE GENERAL SPECIFICATIONS FOR THE PROJECT)
1. THE CONTRACT STRUCTURAL DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION...
2. SHOP DRAWINGS FOR CONCRETE AND MASONRY REINFORCING, STRUCTURAL STEEL, JOIST, AND METAL FLOOR AND ROOF DECKING SUBMITTALS SHALL COMPLY WITH THE FOLLOWING:
A. CONTRACTOR SHALL FURNISH COMPLETE AND DETAILED SHOP DRAWINGS PREPARED UNDER SUPERVISION OF A REGISTERED STRUCTURAL ENGINEER...
B. INDICATE THE DATE OF THE STRUCTURAL DRAWINGS USED FOR SHOP DRAWING PREPARATION.
C. INDICATE WELDS BY STANDARD AWS SYMBOLS AND SHOW SIZE LENGTH AND TYPE OF EACH WELD.
D. PROVIDE SETTING DRAWINGS, TEMPLATES AND DIRECTIONS FOR INSTALLATION OF ANCHOR BOLTS AND OTHER ANCHORAGES TO BE INSTALLED BY OTHERS.
E. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS PRIOR TO SUBMITTAL FOR ENGINEERING REVIEW.
F. CONTRACTOR SHALL HAVE AN APPROVED SET OF STRUCTURAL STEEL SHOP DRAWINGS AND PROOF OF WELDER CERTIFICATION AT THE JOBSITE AT ALL TIMES.
G. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
H. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR BUILDING LOCATION AND ORIENTATION. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
I. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE AND SIMILAR CONDITIONS EVEN THOUGH NOT SPECIFICALLY MARKED ON THE DRAWINGS.
3. DESIGN LOADS: THE BUILDING STRUCTURE DESCRIBED IN THESE PLANS SHALL BE CONSTRUCTED IN COMPLIANCE WITH 2012 IBC WITH ALL GEORGIA AMENDMENTS.
A. GRAVITY LOADS
DEAD LOADS:
ROOF: 15 PSF
ELEVATED FLOOR: 65 PSF
LIVE LOADS:
ROOF: 20 PSF
FLOOR: (a) CLASSROOM & TYP. FLOOR = 40 PSF
(b) CORRIDORS 2nd FLOOR = 80 PSF
(c) ELEVATOR LOBBIES, STAIRS & LANDINGS = 100 PSF
STORAGE LOADS: 125 PSF
B. SNOW LOADS:
GROUND SNOW LOAD (Ps): 5 PSF
UNBALANCED ROOF SNOW LOAD:
WINDWARD SNOW LOADING: 1.0 PSF
LEEWARD SNOW LOAD FROM RIDGE TO 7.5' = 13.7 PSF
LEEWARD SNOW LOAD FROM 7.5' TO THE EAVE = 3.3 PSF
SNOW EXPOSURE FACTOR (Ce): 1.0
SNOW IMPORTANCE FACTOR (Is): 1.1
THERMAL FACTOR (Ti): 0
C. WIND LOADS ULTIMATE LOADS:
ULTIMATE WIND SPEED: 120 MPH
NOMINAL WIND SPEED: 93 MPH
RISK CATEGORY: II
EXPOSURE CATEGORY: B
INTERNAL PRESSURE (Gc): +/- 0.18
SEE ASCE 7-10 FOR COMPONENT & CLADDING LOADS BASED ON TRIBUTARY AREAS FOR INDIVIDUAL ELEMENTS
D. SEISMIC DESIGN CRITERIA:
SEISMIC IMPORTANCE FACTOR (I): 1.25
RISK CATEGORY: II
MAPPED SPECTRAL RESPONSE ACCELERATIONS:
Sa: 0.42g S1: 0.101g
SITE CLASS: D PER GEOTECH REPORT
SDS: 0.25g SD1: 0.181g
SITE COEFFICIENT: Fv = 2.38g
SEISMIC DESIGN CATEGORY: C
BASIC SEISMIC FORCE RESISTING SYSTEM:
INTERMEDIATE REINFORCED MASONRY SHEAR WALLS
DESIGN BASE SHEAR: 292 KIPS (2 STORY CLASSROOM BUILDING)
SEISMIC DESIGN COEFFICIENT: 0.088
DESIGN RESPONSE MODIFICATION FACTOR (R): 4.0
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATION NOTES:

- 1. CONCRETE COMPRESSIVE STRENGTH SHALL BE 3000 PSI U.N.O.. EXTERIOR CONCRETE SHALL BE AIR ENTRAINED TO 6% +/- 112%.
2. SEE ARCHITECTURAL DRAWINGS FOR SIDE WALK EXTENTS, PLANTER, AND PAVEMENT LOCATIONS, CONCRETE PADS AND STAIRS, SEE ARCHITECTURAL FOR DIMENSIONS AT INTERIOR MASONRY PARTITIONS, AND DETAILS.
3. COORDINATE FINISHED FLOOR ELEVATIONS (F.F.E.) WITH ARCH. AND CIVIL DRAWINGS.
4. FOUNDATION DESIGN IS BASED UPON THE REPORT OF SUBSURFACE EXPLORATION, DATED FEBRUARY 28, 2016, BY SALDUS ENGINEERING ASSOCIATES (SEA) - SEA JOB #191-017. CONTRACTOR SHALL FOLLOW ALL RECOMMENDATIONS MADE IN THE GEOTECHNICAL REPORT.
5. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER OR TESTING AGENCY PRIOR TO PLACING ANY FOUNDATION CONCRETE. CONTACT STRUCTURAL ENGINEER IF THE ALLOWABLE SOIL BEARING PRESSURE IS LESS THAN 2500 PSF.
6. ALL EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 2'-0" BELOW F.F.E. OR THE ADJACENT EXTERIOR FINISH GRADE, UNLESS NOTED OTHERWISE.
7. TOP OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 0'-9" BELOW F.F.E. COORDINATE TOP OF FOOTING ELEV. W/ PLUMBING AND LOWER FOOTING IF NECESSARY.
8. PRIOR TO COMMENCING ANY FOUNDATION WORK, COORDINATE WORK WITH ANY EXISTING OR NEW UTILITIES. LOWER FOUNDATION AS REQUIRED TO AVOID INTERFERENCE WITH UTILITIES. EXCEPT WHERE ZERO LOT LINE FOOTINGS ARE LOCATED PARALLEL TO ADJACENT BUILDINGS. REFER TO THE FOUNDATION PLAN FOR FOOTING STEPS AT ADJACENT BUILDINGS - CONTACT ARCHITECTURAL ENGINEER IF A CONFLICT OCCURS.
9. PROTECT PIPES AND CONDUITS RUNNING THROUGH WALLS AND SLABS WITH 1/2" INCH EXPANSION MATERIAL. LOWER CONTINUOUS FOOTING AND GRADE BEAMS PERPENDICULAR TO PIPE RUNS TO ALLOW PIPES TO PASS ABOVE THE FOOTINGS OR THROUGH THE GRADE BEAMS. ALTERNATIVELY, PROVIDE A CONCRETE JACKET IF PIPES ARE LOW ENOUGH TO BE PLACED BELOW THE FOOTINGS AND GRADE BEAMS PARALLEL TO PIPE RUNS TO AVOID SURCHARGE ONTO ADJACENT TRENCH EXCAVATIONS.
10. ARRANGE FOR OWNER'S INDEPENDENT TESTING AGENCY TO MONITOR CUT AND FILL OPERATIONS AND PERFORM FIELD DENSITY AND MOISTURE CONTENT TESTS TO VERIFY COMPACTION AND APPROVE FOOTING SUBGRADES PRIOR TO PLACING CONCRETE.

CONCRETE SLAB NOTES:

- 1. FLOOR SLABS ON GRADE SHALL BE 4" THICK 3000 PSI MINIMUM COMPRESSIVE STRENGTH NORMAL WEIGHT CONCRETE REINFORCED W/ 8# @ 18" ON CENTER UNLESS NOTED OTHERWISE. LOCATE SINGLE LAYER OF W/F AT MID DEPTH OF SLAB. PROVIDE 1/2" POLYETHYLENE VAPOR BARRIER WITH JOINTS LAPPED 6" AND TAPED UNDER ALL INTERIOR SLABS.
2. PROVIDE 4" THICK #57 STONE DRAINAGE FILL BELOW ALL INTERIOR SLABS ON GRADE UNLESS NOTED OR DETAIL OTHERWISE.
3. WHERE CONTROL/CONSTRUCTION JOINTS ARE NOT INDICATED, PROVIDE CONTROL/CONSTRUCTION JOINTS AT 15' O.C. IN EACH DIRECTION. THE LENGTH OF ANY PANEL SHALL NOT EXCEED 1.5 TIMES THE WIDTH. CONSTRUCTION JOINTS AT ADJACENT INTERIOR WALLS SHALL BE DOVELED WITH 3/4" DIAMETER SMOOTH RODS AT 24" O.C. LOCATED AT MIDSPAN OF SLAB.
4. CONDUITS AND PIPES EMBEDDED IN SLABS ON GRADE SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE-THIRD THE OVERALL THICKNESS OF THE SLAB. THE SLAB SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER. A MINIMUM SLAB THICKNESS OF 2 1/2 TIMES THE THICKNESS MUST BE MAINTAINED OVER THE EMBEDDED CONDUITS OR PIPES.
5. COORDINATE THE EXACT LOCATION AND EXTENTS OF ALL FLOOR SLOPES, RECESSED AREAS, DRAIN AND TRENCH LOCATIONS WITH OWNER, ARCHITECTURAL AND PLUMBING DRAWINGS.
6. NO CONDUIT SHALL BE PLACED IN SLAB ON METAL DECK.

REINFORCED CONCRETE NOTES:

- 1. STRUCTURAL MEMBERS OF REINFORCED CONCRETE SHALL BE CONSTRUCTED IN ACCORDANCE WITH AC308.
2. ALL CONCRETE SHALL HAVE A SLAB MINIMUM (4'-1") AND MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI U.N.O.. CONCRETE EXPOSED TO FREEZE-THAW SHALL BE AIR ENTRAINED TO 6% +/- 112%.
3. ALL STEEL REINFORCEMENT SHALL BE FABRICATED FROM ALL WELDED STEEL REINFORCEMENT SHALL BE ASTM A709-GRADE 60 WELDED WIRE REINFORCEMENT SHALL BE ASTM A185-GRADE 60. FABRICATED FROM AS DRAWN STEEL WIRE OR PLAIN STEEL SHEETS.
4. UNLESS NOTED OTHERWISE, CASTER CLASS PLACEMENT OF CONCRETE JOISTS SHALL HAVE THE FOLLOWING TENSILE LOAD ADDED AROUND ALL OPENINGS: (1) LENGTH OF OPENING (L) x 181 LBS PER LINEAL FOOT OF OPENING AND (2) 45X5'-0" DIAGONALLY AT EACH CORNER.
5. CONCRETE COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE AT JOB SITE.
6. ALL L. FORCE LENGTHS SHALL BE AS SHOWN IN TABLE ON THIS SHEET.
7. MINIMUM SPACINGS FOR REINFORCING STEEL: (UNO)
A. CONCRETE CAST AND PERMANENTLY EXPOSED TO EARTH: 3"
B. CONCRETE EXPOSED TO EARTH AND WEATHER:
REBAR AND COLUMNS: 4"
REBAR AND WALLS: 3"
REBAR AND JOISTS: 2"
C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH:
REBAR, WALLS AND JOISTS:
#14 OR #13 BARS: 1 1/2"
#12 BARS AND SMALLER: 2"
BEAMS AND COLUMNS: 2"
8. CONTRACTOR SHALL COORDINATE DIMENSIONS OF EMBEDDED STEEL ITEMS FOR EQUIPMENT AND OTHER ITEMS.

STEEL NOTES:

- 1. STRUCTURAL STEEL:
A. SHALL CONFORM TO THE LATEST STANDARDS OF ASTM:
WIDE FLANGE BEAMS: A992
OTHER STRUCTURAL STEEL SHAPES, PLATES AND BARS: A36
HOLLOW STRUCTURAL STEEL SECTIONS (ROUND AND RECTANGULAR): A500 GRADE B
STRUCTURAL STEEL PIPE: A53 GRADE B
B. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-10) USING LOAD AND RESISTANCE FACTOR DESIGN.
C. PROVIDE 1" (MINIMUM) NON-SHRINK 5000 PSI GROUT UNDER ALL BASE PLATES.
D. SHOP OR FIELD SPLICES BETWEEN SUPPORTS THAT ARE NOT REQUIRED BY DESIGN WILL NOT BE ALLOWED. ANY MEMBERS CONTAINING SUCH SPLICES FOUND IN THE FIELD SHALL BE REMOVED AND REPLACED WITH UNSPLICED MEMBERS AT THE FABRICATOR'S EXPENSE.
2. STEEL CONNECTIONS:
A. WHERE BEAM REACTIONS OR DETAILS ARE NOT SHOWN IN THE CONSTRUCTION DOCUMENTS, CONNECTIONS SHALL BE DESIGNED FOR ONE-HALF THE MAXIMUM (SIMPLE SPAN) UNIFORM LOAD WHICH THE MEMBER WILL SUPPORT FOR THE SPAN SHOWN ON THE DRAWINGS.
B. UNLESS LARGER REACTION IS SHOWN ON DRAWINGS, PROVIDE MINIMUM DESIGN FORCES AS FOLLOWS:
(1) NON-COMPOSITE BEAMS: BEAM-TO-BEAM OR BEAM-TO-COLUMN CONNECTION TO DEVELOP THE REACTION OF CONNECTED BEAM. OBTAIN END REACTION FROM ALLOWABLE UNIFORM LOAD TABLES IN PART 2 OF THE AISC MANUAL OF STEEL CONSTRUCTION.
C. ADD TO REACTIONS LISTED ABOVE, LOADS OR REACTIONS OF MEMBERS SUPPORTED BY BEAM WITHIN THREE FEET OF BEAM END AND VERTICAL COMPONENTS OF FORCES IN BRACE MEMBERS FRAMING INTO BEAM.
D. BRACE CONNECTIONS SHALL BE IN ACCORDANCE WITH THE DETAILS SHOWN ON THE DRAWINGS, ANGLE SIZES, PLATE SIZES, AND SIZE AND LENGTHS OF WELDS SHALL BE DESIGNED IN ACCORDANCE WITH THE FOLLOWING:
(1) DESIGN CONNECTIONS OF DIAGONAL MEMBERS TO DEVELOP THE LOADS SHOWN ON THE BRACE DETAILS.
(2) WHERE FORCES ARE NOT INDICATED ON THE DETAILS, DESIGN CONNECTIONS OF DIAGONAL MEMBERS TO DEVELOP THE FULL TENSILE CAPACITY OF THE DIAGONAL MEMBER.
(3) SIZE GUSSET PLATES AND ALL WELDS TO RESIST THE FORCE OF THE DIAGONAL MEMBERS. PLATES AND WELDS SHALL BE SIZED FOR TENSIONS, SHEARS, AND MOMENTS CAUSED BY CONCENTRIC AND ECCENTRIC FORCES.
(4) ALL BRACE CONNECTIONS SHALL USE WELDS OR FULLY TENSIONED A325 CLASS A SLIP CRITICAL BOLTS.
(5) BOLTS SHALL BE HIGH STRENGTH A-325 BOLTS OF SAME SIZE AND NUMBER AS SHOWN ON DRAWINGS. CONNECTIONS SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS. CONNECTIONS ARE BEARING TYPE.
E. BOLTS SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITION UNLESS OTHERWISE NOTED ON THE DRAWINGS.

WELDS:

- 1. PROVIDE MINIMUM SIZE OF FILLET WELDS AS SPECIFIED IN TABLE J2.4 OF THE AISC MANUAL.
2. ALL WELDING SHALL CONFORM TO THE LATEST "STRUCTURAL WELDING CODE" BY THE AMERICAN WELDING SOCIETY. ALL WORK SHALL BE PERFORMED BY CERTIFIED WELDERS EXPERIENCED IN THE TYPE OF CONSTRUCTION INVOLVED. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE.
3. DEVELOP THE FULL TENSILE STRENGTH OF THE MEMBER ELEMENT JOINED, ON ALL SHOP AND FIELD WELDS, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
4. ALL WELDS ARE CONTINUOUS FOR THE FULL LENGTH OF THE CONNECTION UNLESS NOTED OTHERWISE ON DRAWINGS.
5. HEADED STUDS (SHEAR AND ANCHOR) AND DEFORMED ANCHORS:
PROVIDE HEADED STUDS (SHEAR AND ANCHOR) MADE OF MATERIAL CONFORMING TO ASTM A108.
(1) PROVIDE DEFORMED ANCHORS MADE OF MATERIAL CONFORMING TO ASTM A995.
(2) WELD STUDS ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. MANUAL ARC (STICK) WELDING OF HEADED STUDS AND/OR DEFORMED ANCHORS IS NOT ALLOWED.

STEEL JOISTS:

- A. SIZE TO BE AS SHOWN ON PLANS. ALL K SERIES JOISTS TO BE WELDED TO BEAMS, JOIST GIRDERS OR WELD PLATES WITH 1/2" MIN OF 1/8" WELD ON EACH SIDE OF STEEL JOIST. LH SERIES JOISTS SHALL BE WELDED WITH 2" OF 1/4" FILLET WELD EACH SIDE. WELDS ARE IN ADDITION TO ANY BOLTS REQUIRED FOR ERECTION. ALL JOIST BRIDGING PER SJI REQUIREMENTS. SEE SPECIFIC DETAILS FOR ADDITIONAL WELD REQUIREMENTS.
B. JOISTS TO HAVE STANDARD CAMBER AND BE DESIGNED AND FABRICATED ACCORDING TO THE LATEST STANDARDS OF THE STEEL JOIST INSTITUTE.
C. ALL BRIDGING TERMINATING AT MASONRY WALL OR STEEL BEAM TO BE ANCHORED TO WALL OR BEAM.
D. JOISTS AT COLUMNS TO BE BOLTED TO BEAM, JOIST GIRDER, OR BEARING SEAT AT TIME OF ERECTION.
E. JOISTS 40' AND LONGER SHALL BE BOLTED TO SUPPORT PER SJI RECOMMENDATION.
F. GENERAL CONTRACTOR AND JOIST MANUFACTURER ARE RESPONSIBLE FOR REPLACING AND/OR REPAIRING DAMAGED MEMBERS. IF REPAIRS ARE MADE, A LETTER BEARING THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA MUST BE PROVIDED BY THE JOIST MANUFACTURER APPROVING SUCH REPAIRS.
G. JOISTS 60ES INDICATED HAVE BEEN DESIGNED TO SUPPORT MECHANICAL EQUIPMENT SHOWN AT LOCATIONS INDICATED. NOTIFY ARCHITECT/ENGINEER IF UNITS ARE RELOCATED OR A HEAVIER EQUIPMENT IS SUPPLIED.
H. CONCENTRATED LOADS ON BAR JOISTS NOT LOCATED DIRECTLY AT A TOP OR BOTTOM CHORD PANEL POINT SHALL HAVE AN ADDITIONAL WELDED ANGLE ADDED AT THE POINT OF LOAD PER DETAIL 14S4.2.
I. JOIST NET UPLIFT - 10 PSF AT END ZONES, 4.5 PSF AT INTERIOR ZONES. END ZONE WIDTH, a = 8'-2".

COLD FORMED STEEL FRAMING:

- A. COLD FORMED STEEL FRAMING STRENGTH CRITERIA:
(1) 18 MIL-48 MIL - 33 KSI MIN. YIELD STRESS
(2) 14 MIL-36 MIL - 30 KSI MIN. YIELD STRESS
(3) RUNNER TRACK - 33 KSI MIN. YIELD STRESS (UNO)
B. ATTACH METAL FRAMING TO PRIMARY STRUCTURE WITH A DEFLECTION TRACK OR A STEEL NETWORK INC. VERTICAL DEFLECTION CLIPS CAPABLE OF ALLOWING 1" OF VERTICAL DEFLECTION IF WALL FRAMING IS SUPPORTED BY SLAB ON GRADE OR OTHER RIGID SUPPORT.
C. PLACE ALL COLD-FORMED STEEL STUD WALL BRIDGING HORIZONTALLY WITH A MAXIMUM VERTICAL SPACING OF FOUR FEET UNLESS NOTED OTHERWISE. AS AN OPTION, CONTINUOUS COLD-FORMED CHANNELS MAY BE POSITIONED THROUGH THE STUD WALL JOINTS AS BRIDGING PROVIDED THE CHANNEL IS PROPERLY FASTENED TO EACH STUD. PROVIDE MANUFACTURER'S RECOMMENDED FASTENERS TO BOTH FLANGES OF MEMBER IF FLANGE IS NOT PERMANENTLY BRACED WITH GYPSUM OR PLYWOOD SHEATHING.
D. PLUMB, ALIGN, AND SECURELY ATTACH STUDS TO THE FLANGES OF BOTH UPPER AND LOWER TRACKS. SPLICES IN STUDS ARE NOT PERMITTED.
E. PROVIDE HEADERS AND SUPPORTING STUDS FOR FRAMING OF WALL OPENINGS PER MANUFACTURER'S RECOMMENDATIONS BASED ON THE SPAN OF THE OPENING.
F. ALL INTERIOR SUSPENDED GYPSUM BOARD SOFFITS SHALL BE SUPPORTED BY STRUCTURAL STEEL MEMBERS PER DETAIL 8A9/94.3. SOFFITS SHALL NOT BE SUSPENDED FROM METAL DECK.
G. METAL STUD FASTENERS:
UNLESS NOTED OTHERWISE,
A. SCREW CONNECTIONS:
MIN. 10-18 KWIK-LOCK SELF-DRILLING SCREWS OR APPROVED EQUIVALENTS UNLESS OTHERWISE NOTED. 2 SCREWS PER CONNECTION MIN. UNLESS NOTED OTHERWISE OR A PRE-ENGINEERED JOIST.
B. POWER DRIVEN FASTENERS:
(1) FASTENING TO CONCRETE:
USE 0.148" DIA. DOME HEAD NAIL TYPE "Z" BY TEST OR APPROVED EQUIVALENTS UNLESS OTHERWISE NOTED.
MIN. EMBEDMENT = 1 1/4" MIN. EDGE DISTANCE = 1"
MIN. SPACING = 3" MAX. SPACING = 12"
(2) FASTENING TO STEEL:
USE 0.148" DIA. DOME HEAD KNURLED SHANK FASTENER TYPE "H" BY TEST OR APPROVED EQUAL UNLESS OTHERWISE NOTED.
MIN. EMBEDMENT = FULL PENETRATION
MIN. EDGE DISTANCE = 1 1/2" MIN. SPACING = 12"
(3) PINS SHALL BE LOCATED 1/2" FROM OUTSTANDING LEG OF CHANNELS WHEN ATTACHING TO STRUCTURAL STEEL ANGLES.

METAL DECKING:

- A. PROVIDE ALL FABRICATED METAL DECK CONFORMING TO THE STEEL DECK INSTITUTE'S "CODE OF RECOMMENDED STANDARDS FOR FABRICATED METAL DECKING" AND BASIC DESIGN SPECIFICATIONS".
B. ALL METAL SHALL BE FABRICATED TO PROVIDE A THREE SPAN CONDITION UNLESS INDICATED OTHERWISE. PROVIDE 1 1/2" MINIMUM DECK BEARING AT EXTERIOR AND 3" MIN. AT INTERIOR CONDITION.
C. ATTACHMENT AT PERIMETER OF DECK SHALL BE EQUAL TO ATTACHMENT AT DECK SHEET LAPS AND DECK SHEET ENDS. ANY PARTIAL OR SKEWED SHEETS SHALL BE ATTACHED AT EVERY FLUTE REGARDLESS OF ATTACHMENT PATTERN.
D. PROVIDE STEEL JOIST FRAMING AT ALL OPENINGS IN FLOOR AND ROOF DECK 12" OR GREATER. ROOFTOP UNIT CURBS AND OTHER UNITS SHALL BE SECURED DIRECTLY ON STEEL ANGLE FRAME. FASTEN CURBS TO FRAMES W/ #12 SCREWS @ 12" O.C. OR EQUIVALENT WELDS.
E. PROVIDE L3 1/2x4 1/2x3/8" (UNO) LVL CONTINUOUS ANGLE AT JOIST BEARING. WELD ANGLE TO JOIST WITH 2" OR 3/8" FILLET WELD ON TOP OF JOIST @ TOE OF ANGLE AND UNDERNEATH ANGLE TO EDGES OF JOIST. ATTACH DECK TO CONTINUOUS ANGLE WITH REQUIRED SIDELAP FASTENERS OR AT ALL FLUTES IF DECK END BEARS ON ANGLE. WELD ANGLE TO COLUMNS ON EACH LEG WHERE CROSSING COLUMN.
F. PROVIDE L3x4 1/2x4 1/2x3/8" (UNO) LVL CONTINUOUS ANGLE AT FLOOR DECK EDGE AND OPENINGS. WELD ANGLE TO JOIST OR BEAM WITH 3/8" - 2 @ 12" O.C. FILLET WELD ON TOP OF JOIST @ TOE OF ANGLE AND UNDERNEATH ANGLE TO EDGES OF JOIST. WELD TO CONTINUOUS BEAM WITH 1 1/2" OF 3/8" FILLET WELDS AT 12" O.C. ATTACH DECK TO CONTINUOUS ANGLE WITH REQUIRED SIDELAP FASTENERS OR AT ALL FLUTES IF DECK END BEARS ON ANGLE. WELD ANGLE TO COLUMNS ON EACH LEG WHERE CROSSING COLUMN.
G. ALTERNATE FASTENER TYPES MAY BE ALLOWED WITH PRIOR APPROVAL BY STRUCTURAL ENGINEER. FASTENER PATTERN FOR DECK FASTENERS WILL BE DETERMINED DURING APPROVAL.
H. BUILDING ELEMENTS SHALL NOT BE SUPPORTED FROM METAL FLOOR OR ROOF DECK. THIS INCLUDES: SOFFITS, CEILING, MECHANICAL EQUIPMENT AND DUCTS; ELECTRICAL EQUIPMENT; CONDUITS LARGER THAN 3/4" AND THEIR JUNCTION BOXES; LIGHTING; PIPING SYSTEMS FOR WATER, GAS, SPRINKLERS.

METAL PAN STAR FRAMING:

- 1. METAL PAN STAR FRAMING SIZES SHOWN ARE FOR REFERENCE ONLY. STAR FRAMING SHALL BE DESIGNED BY FABRICATOR. REFER TO ARCHITECTURAL DRAWINGS FOR STAR DIMENSIONS AND LAYOUT. REFER TO SPECIFICATIONS FOR REQUIRED DESIGN LOADS. LANDINGS MAY BE ANCHORED TO WALLS OR POSTS MAY BE ADDED TO BEAR ON SLAB BELOW WHERE POSTS ARE NOT EXPOSED IN THE FINISHED STAR.

J. COLUMN BASEPLATES:

- 1. COLUMN BASEPLATES THAT DO NOT HAVE THREE ANCHORS SHALL BE ADEQUATELY BRACED OR SUPPORTED BY OTHER MEANS DURING ERECTION TO SATISFY OSHA REQUIREMENTS.

REINFORCED MASONRY NOTES:

- 1. ALL MASONRY WALLS SHALL HAVE CELLS REINFORCED AND DOVELED INTO FOUNDATION OR TOP OF CONCRETE WALL AS NOTED ON THE DRAWINGS. ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID.
2. REINFORCED CELLS SHALL BE FILLED WITH GROUT AND SHALL BE CONTINUOUS FROM FOUNDATION TO THE TOP OF THE WALL.
3. FILL REINFORCED CELLS WITH MECHANICALLY MIXED GROUT.
A. GROUT SHALL CONFORM TO ASTM C-476 AND BE SPECIFIED BY PROPORTION REQUIREMENTS. MINIMUM GROUT STRENGTH SHALL BE 3000 PSI PER PART 2 OF THE AISC MANUAL.
B. GROUT SHALL BE MIXED FOR AT LEAST 5 MINUTES AND PLACED WITHIN 1 1/2 HOURS FROM THE INITIAL INTRODUCTION OF WATER AND PRIOR TO INITIAL SET.
C. BETWEEN GROUT POURS, A HORIZONTAL CONSTRUCTION JOINT SHALL BE FORMED BY STOPPING ALL CMU AT THE SAME ELEVATION AND WITH THE GROUT STOPPING A MINIMUM OF 1 1/2" BELOW A MORTAR JOINT, EXCEPT AT THE TOP OF THE WALL WHERE BOND BEAMS OCCUR. THE GROUT POUR SHALL BE STOPPED A MINIMUM OF 1/2" BELOW THE TOP OF THE MASONRY.
D. CMU WALLS SHALL BE CONSTRUCTED USING LOW-LIFT GROUTING (5'-0" MAX. POUR HEIGHTS).
E. CONSOLIDATE GROUT POURS 12 INCHES OR LESS IN HEIGHT BY MECHANICAL VIBRATION OR PULDING. CONSOLIDATE GROUT POURS EXCEEDING 12 INCHES IN HEIGHT BY MECHANICAL VIBRATION, AND RECONSOLIDATE BY CONSOLIDATE VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED. ALTERNATE METHODS OF CONSOLIDATION MUST BE APPROVED BY THE SPECIAL INSPECTOR AND ENGINEER PRIOR TO BEGINNING GROUT PLACEMENT.
4. ALL CMU SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH (Fm) OF 1500 PSI. MASONRY UNITS SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI.
5. MORTAR SHALL BE ASTM 270 TYPE S. MINIMUM MORTAR STRENGTH SHALL BE 1800 PSI.
A. THICKNESS OF BED JOINTS SHALL BE 3/8" EXCEPT THAT THE THICKNESS OF THE BED JOINT OF THE STARTING COURSE PLACED OVER FOUNDATIONS SHALL NOT BE LESS THAN 1/4" AND NOT MORE THAN 3/4". FIRST JOINT AT TOP OF FOOTING SHALL BE A FULL MORTAR BEDDED AREA.
B. MORTAR SHALL BE MIXED UNTIL MIXTURE IS UNIFORM THROUGHOUT.
C. UNUSED MORTAR SHALL BE DISCARDED WITHIN 2 1/2 HOURS AFTER INITIAL MIXING.
D. PROPOSED MORTAR MIX DESIGN SHALL BE LABORATORY TESTED PRIOR TO SUBMITTAL FOR APPROVAL TO PROVIDE BASIS FOR MONITORING OF FIELD MORTAR TESTS.
6. ALL MASONRY SHALL CONFORM TO ASTM C-90 BLOCK AND BE CONSTRUCTED USING RUNNING BOND (U.N.O.).
7. REINFORCING STEEL SHALL CONFORM TO ASTM A615-GRADE 60. FOR WELDED REINFORCING USE ASTM A706-GRADE 60.
8. ALL MASONRY WALLS SHALL HAVE STANDARD HORIZONTAL (9 GA.) LADDER TYPE REINFORCING @ 16" O.C. JOINT REINFORCEMENT SHALL CONFORM TO ASTM A601.
9. COORDINATE REQUIRED TYPE, SIZE, GAUGE, AND SPACING OF STEEL ANCHORS FOR ATTACHMENT OF MASONRY VENEER WITH ARCH. DRAWINGS. MINIMUM ANCHOR SPACING SHALL BE 16" O.C. VERTICALLY AND 24" O.C. HORIZONTALLY. INDIVIDUAL ANCHORS SHALL BE 9 GA. WIRE MINIMUM. EYE AND KNOTS OF JOINT REINFORCING AND ALL ANCHORS SHALL BE HOT-DIPPED GALVANIZED.
10. CONTROL JOINTS IN CONCRETE MASONRY SHALL HAVE A MAXIMUM SPACING OF 30' O.C. (UNO). CONTROL JOINTS IN MASONRY WALLS WITH ARCH. DWGS. CONTROL JOINTS SHALL NOT BE LOCATED WITHIN STRUCTURAL PLASTER OR MASONRY LINTELS.
11. SLEEVE ALL PLUMBING OR FIRE PROTECTION PIPING THROUGH CMU WALLS.
12. REINFORCING BARS FOR VERTICAL FILLED CELLS SHALL BE LAPPED ACCORDING TO MASONRY LAPPING TABLE.
13. COORDINATE EXACT LOCATION AND EXTENT OF ALL OPENINGS IN MASONRY WALLS WITH ARCH. DRAWINGS. SEE ARCH. DRAWING SCHEDULE AND LINTEL SCHEDULE 15S4.1.
14. MINIMUM MASONRY COVER FOR REINFORCING STEEL:
A. MASONRY EXPOSED TO WEATHER OR EARTH:
2" FOR BARS LARGER THAN #5
1 1/2" FOR #5 AND SMALLER BARS
B. MASONRY NOT EXPOSED TO WEATHER OR EARTH: 1 1/2"
C. BARS SHALL BE LOCATED IN CENTER.
15. ALL BARS OVER STEEL BEAMS SHALL HAVE DOWELS WELDED TO THE TOP FLANGE OF THE BEAM WITH A 5/16" FILLET WELD ALL AROUND. DOWEL LENGTH SHALL BE AT LEAST ALL WORK REQUIRING INSPECTION IS PERFORMED IN COMPLIANCE WITH ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS, BUILDING CODE REQUIREMENTS AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.
16. SPECIAL INSPECTIONS ARE REQUIRED FOR THE ITEMS NOTED IN THE STATEMENT OF SPECIAL INSPECTIONS AND THE 2008 IBC. THE CONTRACTOR SHALL OBTAIN A COPY OF THE STATEMENT OF SPECIAL INSPECTIONS AND NOTIFY THE SPECIAL INSPECTOR WHEN WORK IS READY TO BE INSPECTED.
17. FAILURE TO NOTIFY THE SPECIAL INSPECTOR PRIOR TO OCCURRING AN ITEM REQUIRING INSPECTION MAY RESULT IN THE CONTRACTOR REMOVING OTHER WORK TO ALLOW INSPECTION. THIS WORK WILL BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE. FAILURE TO HAVE REQUIRED ITEMS INSPECTED IS REASON FOR REJECTION OF THE WORK.
18. PREMATURITY NOTIFICATION FOR INSPECTIONS WILL RESULT IN ADDITIONAL INSPECTION WITH ALL EXPENSES AND FEES PAID FOR BY THE CONTRACTOR.
19. SEE SHEET S6.1 FOR SPECIAL INSPECTION CHECKLIST.

SPECIAL INSPECTIONS:

- 1. THE SPECIAL INSPECTOR SHALL BE ENGAGED BY THE OWNER. SPECIAL INSPECTOR SHALL BE FULLY QUALIFIED, APPROVED BY THE BUILDING OFFICIAL, REGISTERED BY APPLICABLE REGISTRATION BOARD IF REQUIRED BY THE LOCAL BUILDING OFFICIAL, AND SHALL BE AVAILABLE TO THE ARCHITECT.
2. THE SPECIAL INSPECTOR SHALL PREPARE VERIFICATION OF CONSTRUCTION QUALITY CONTROL INSPECTIONS AND TESTING. THE SPECIAL INSPECTOR SHALL VERIFY THAT ALL WORK REQUIRING INSPECTION IS PERFORMED IN COMPLIANCE WITH ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS, BUILDING CODE REQUIREMENTS AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.
3. SPECIAL INSPECTIONS ARE REQUIRED FOR THE ITEMS NOTED IN THE STATEMENT OF SPECIAL INSPECTIONS AND THE 2008 IBC. THE CONTRACTOR SHALL OBTAIN A COPY OF THE STATEMENT OF SPECIAL INSPECTIONS AND NOTIFY THE SPECIAL INSPECTOR WHEN WORK IS READY TO BE INSPECTED.
4. FAILURE TO NOTIFY THE SPECIAL INSPECTOR PRIOR TO OCCURRING AN ITEM REQUIRING INSPECTION MAY RESULT IN THE CONTRACTOR REMOVING OTHER WORK TO ALLOW INSPECTION. THIS WORK WILL BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE. FAILURE TO HAVE REQUIRED ITEMS INSPECTED IS REASON FOR REJECTION OF THE WORK.
5. PREMATURITY NOTIFICATION FOR INSPECTIONS WILL RESULT IN ADDITIONAL INSPECTION WITH ALL EXPENSES AND FEES PAID FOR BY THE CONTRACTOR.
6. SEE SHEET S6.1 FOR SPECIAL INSPECTION CHECKLIST.

STRUCTURAL ENGINEER OF RECORD OBSERVATIONS:

- 1. THE SPECIAL INSPECTOR IS RESPONSIBLE FOR CONTINUOUS AND PERIODIC INSPECTION OF ALL WORK AS REQUIRED IN THE STATEMENT OF SPECIAL INSPECTIONS. THE STRUCTURAL ENGINEER OF RECORD OBSERVATION IS IN ADDITION TO THE SPECIAL INSPECTION AND DOES NOT SERVE AS THE SPECIAL INSPECTOR FOR THE ITEMS NOTED. THE STRUCTURAL ENGINEER OF RECORD WILL OBSERVE THE INITIAL AREAS WHERE THE WORK NOTED IN THE FOLLOWING LIST IS BEING PERFORMED. CORRECTIONS TO THE WORK PERFORMED IN THE INITIAL AREAS NOTED BY THE ENGINEER OF RECORD SHALL BE FOLLOWED IN SUBSEQUENT AREAS WHERE SIMILAR WORK IS TO BE PERFORMED.
2. CONTRACTOR SHALL NOTIFY THE ARCHITECT FIVE WORKING DAYS PRIOR TO THE FOLLOWING CONSTRUCTION MILESTONES SO THE PROJECT ENGINEER CAN OBSERVE THE WORK:
A. SPREAD FOOTING FOUNDATIONS. EACH TYPE OF RETAINING WALL FOUNDATIONS - AFTER FOUNDATION REINFORCING IS PLACED AND PRIOR TO PLACING CONCRETE FOR THE FOUNDATIONS.
B. EACH TYPE OF RETAINING WALL - AFTER WALL REINFORCING IS PLACED AND PRIOR TO PLACING CONCRETE FOR THE WALL.
C. FIRST SLAB ON GRADE PLACEMENT - AFTER SLAB REINFORCING IS PLACED AND BEFORE PLACING SLAB ON GRADE.
D. REINFORCED MASONRY WALLS - AFTER WALLS AND REINFORCING ARE INSTALLED AND PRIOR TO GROUTING.
E. STEEL STRUCTURE - AFTER FLOOR AND ROOF FRAMING AND ROOF DECK IS FULLY INSTALLED AND PRIOR TO INSTALLING INSULATION OR ROOFING.
F. FLOOR SLAB ON METAL DECK PLACEMENT - AFTER SLAB REINFORCING IS PLACED AND BEFORE PLACING SLAB.
3. FAILURE TO NOTIFY THE STRUCTURAL ENGINEER PRIOR TO ANY CONSTRUCTION MILESTONE OBSERVATION MAY RESULT IN THE CONTRACTOR REMOVING OTHER WORK TO ALLOW OBSERVATION. THIS WORK WILL BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
4. PREMATURITY NOTIFICATION FOR SITE VISIT WILL RESULT IN ADDITIONAL OBSERVATION WITH ALL EXPENSES AND FEES PAID FOR BY THE CONTRACTOR.

PRE-ENGINEERED TRUSS NOTES:

- 1. PRE-ENGINEERED TRUSSES, INCLUDING ATTACHMENT TO BUILDING STRUCTURE, SHALL BE DESIGNED IN ACCORDANCE WITH APPLICABLE PROVISIONS OF LATEST EDITION OF AISI AND SEALED BY A GEORGIA REGISTERED ENGINEER.
2. ALL TRUSSES ARE TO BE DESIGNED FOR UPLIFT LOADS REQUIRED BY THE 2012 INTERNATIONAL BUILDING CODE. ALSO, TRUSSES TO BE DESIGNED TO SUPPORT THE MECHANICAL EQUIPMENT IF SHOWN ON THE MECHANICAL DRAWINGS.
3. ALL HARDWARE (BOLTS, HANGERS, STRAPS, ETC.) REQUIRED FOR CONNECTIONS BETWEEN PRE-ENGINEERED TRUSSES SHALL BE DESIGNED AND SPECIFIED BY THE TRUSS DESIGN ENGINEER.
4. TRUSS DESIGN DRAWINGS SHALL INCLUDE THE FOLLOWING LOADS AS APPLICABLE:
A. TOP CHORD LL = 20 PSF TOP CHORD DL = 10 PSF
B. BOT CHORD LL = 0 PSF BOT CHORD DL = 10 PSF
C. WIND LOAD = 120 MPH (2012 IBC FIGURE 1608A)
5. TRUSSES SHALL BE SUFFICIENTLY BRACED DURING INSTALLATION TO PREVENT TOPPLING OR BUCKLING. INSTALL ALL BRACING BEFORE PLACING CONCENTRATED LOADS ON TRUSSES.
6. ALL PRE-ENGINEERED TRUSS SHOP DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE DURING THE TIMES OF INSPECTION AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED AND APPROVED BY THE PROJECT STRUCTURAL ENGINEER OR RECORD.
7. PRE-ENGINEERED TRUSS ENGINEER SHALL PROVIDE CONNECTIONS BETWEEN TRUSSES AND BUILDING STRUCTURE.
8. TRUSS MANUFACTURER/FABRICATOR SHALL SUPPLY SHOP DRAWINGS THAT INCLUDE TRUSS LAYOUT PLAN, TRUSS ELEVATIONS, TRUSS TO TRUSS CONNECTIONS, TRUSS TO BEARING CONNECTIONS AND DETAILS, DIAPHRAGM SHEAR TRANSFER BRACINGS, LATERAL WEB BRACING, BOTTOM CHORD BRACING, AND ALL TEMPORARY OR ERECTION BRACING. SHOP DRAWINGS AND/OR CALCULATIONS SHALL SHOW THE LINE AND DEAD LOADS AT THE TOP AND BOTTOM CHORD AND THE STRESSES IN EACH TRUSS MEMBER.
9. COLD-FORMED METAL TRUSSES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST AISI.

ABBREVIATIONS:

Table with 3 columns: Abbreviation, Full Name, and another Abbreviation. Includes ADDL., ARCH., BAL., BOT., C.J., C., CA, CB, C.C., CL, COL., CONC., CONN., CONSTR., CONT., DET., DIA., DWG., DWLS, EA, EE, E.F., E.J., ELE., ELECT., E.W., E.O.S., E.O.D., EQUIV., FIN., FLOOR, FRIZ., H.P., H.T., JS, L.L.H., L.L.V., L.P., LT., LW., MAX., M.C., MFR., MIN., N.F., NO., NTS, ON CENTER, OPNG., P.A.F., P.C., PL., P.T., REINF., REOD., SCHED., SECT., SIMILAR, SOFT JOINT, SLAB ON GRADE, SQUARE, STAINLESS STEEL, STEEL, STANDARD, STIFF, STIFFENER, SHORT WAY, SYMM., TOP & BOTTOM, T.F., T.O.S., TYP., U.O.N., VERT., V.F.F., WT., W.W.F., LOW POINT, LIGHT, LIGHT WEIGHT, MAXIMUM, MOMENT RESISTING CONNECTION, MANUFACTURER, MINIMUM, NEAR FACE, NUMBER, NOT TO SCALE, ON CENTER, OPENING, POWDER ACTUATED FASTENERS, PRECAST CONCRETE, PLATE, PRESSURE TREATED, REINFORCEMENT, REQUIRED, SCHEDULED, SECTION, STANDARD, VERIFIED, VERIFY IN FIELD, WEIGHT, WELDED WIRE FABRIC.

REINFORCED CONCRETE TENSION LAP SPlice LENGTH:

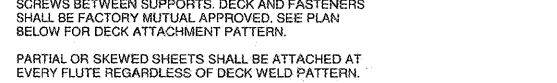
Table with 3 columns: BAR SIZE, Fc=3000 PSI, Fc=4000 PSI. Rows for #3, #4, #5, #6, #7.

MASONRY REINFORCING LAP LENGTH:

Table with 4 columns: BAR SIZE, 6" CMU, 8" CMU, 12" CMU. Rows for #3, #4, #5, #6, #7.

METAL ROOF DECK SHALL BE SUPPLIED IN 36" WIDTHS. ROOF DECK SHALL BE ATTACHED TO EACH SUPPORT WITH 5/8" PUDDLE WELD OR EQUIVALENT. METAL ROOF DECK SIDELAPS SHALL BE ATTACHED WITH #10 DRILL-POINT SELF-TAPPING SCREWS. DECK AND FASTENERS SHALL BE FACTORY MUTUAL APPROVED. SEE PLAN BELOW FOR DECK ATTACHMENT PATTERN.

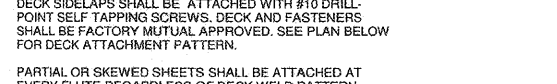
PARTIAL OR SKEWED SHEETS SHALL BE ATTACHED AT EVERY FLUTE REGARDLESS OF DECK WELD PATTERN.



1.5" TYPE B

METAL FLOOR DECK SHALL BE SUPPLIED IN 36" WIDTHS. FLOOR DECK SHALL BE ATTACHED TO EACH SUPPORT WITH 5/8" PUDDLE WELDS OR EQUIVALENT. METAL FLOOR DECK SIDELAPS SHALL BE ATTACHED WITH #10 DRILL-POINT SELF-TAPPING SCREWS. DECK AND FASTENERS SHALL BE FACTORY MUTUAL APPROVED. SEE PLAN BELOW FOR DECK ATTACHMENT PATTERN.

PARTIAL OR SKEWED SHEETS SHALL BE ATTACHED AT EVERY FLUTE REGARDLESS OF DECK WELD PATTERN.



2" TYPE VLI COMPOSITE DECK

CONCRETE RETAINING WALL NOTES:

- 1. ALL CONCRETE TO BE 4000 PSI COMPRESSIVE STRENGTH AT 28 DAYS UNLESS NOTED OTHERWISE.
2. DESIGN PARAMETERS:
SOIL BEARING PRESSURE: 2500 PSF
ACTIVE EQUIVALENT FLUID PRESSURE: 40.0 PCF
AT REST EQUIVALENT FLUID PRESSURE: 65.0 PCF
PASSIVE EQUIVALENT FLUID PRESSURE: 335.0 PCF
FRICTION COEFFICIENT: 0.4
3. REINFORCING STEEL:
A. SHALL BE DETAILED, FABRICATED AND PLACED ACCORDING TO THE LATEST STANDARDS OF A.C.I.
OR 25' O.C. MAX. ALTERNATE LONGITUDINAL BARS SHALL BE CUT EXACTLY OPPOSITE SUCH CONTRACTION JOINTS.
B. CONTRACTION JOINTS BETWEEN SUCCESSIVE POURS OF CONCRETE TO BE KEPT (METAL KEYHOLE OR SIMILAR).
C. CONSTRUCTION JOINTS MAY BE SUBSTITUTED FOR ANY CONTROL JOINT.
D. DO NOT LOCATE CONTRACTION JOINT, CONSTRUCTION JOINT OR EXPANSION JOINT WITHIN 8'-0" EITHER SIDE OF PIER OR PILECAP.
E. DO NOT LOCATE CONTRACTION JOINT, CONSTRUCTION JOINT OR EXPANSION JOINT UNDER BEAM POCKET OR CAST IN PLACE BEARING PLATE.
4. BACKFILL:
A. COMPACT BACKFILL PER THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER.

GENERAL NOTES

05-07-2018

PROJECT NUMBER 17-303

SHEET NUMBER S0.1

Professional seal and contact information for Robertson India Roof Architects and Engineers, Inc. (Professional Seal No. 2757-A, State of Georgia, License No. 770-674-2600 / www.riac.com) and Dawson County College and Career Academy (Faculty No. 642-0198) and Dawson County Schools (Overall Squarefootage = 35,800 SF, FTE = 1623).