GENERAL NOTES

1. COMPLY WITH COMPLY WITH THE INTERNATIONAL BUILDING CODE 2012 EDITION AND GEORGIA AMENDMENTS.
2. ALL CONSTRUCTION METHODS SHALL COMPLY WITH THE REQUIREMENTS OF CHAPTER 33 OF THE GEORGIA BUILDING CODE, "SAFEGUARDS DURING CONSTRUCTION OR DEMOLITION".
3. THE CONTRACTOR SHALL DO THE FOLLOWING:
   A. VERIFY ALL EXISTING CONDITIONS, SUCH AS THE LOCATIONS OF EXISTING STRUCTURAL AND ARCHITECTURAL ELEMENTS AND DIMENSIONS OF EXISTING CONSTRUCTION. DIMENSIONS SHOWN ARE PER INFORMATION PROVIDED BY THE ORIGINAL STRUCTURAL DRAWINGS OR ARE ASSUMED.
   B. SUBMIT TO THE ENGINEER FOR REVIEW A WRITTEN REPORT INDICATING ACTUAL FIELD CONDITIONS WHICH MAY VARY FROM INFORMATION INDICATED ON THE DRAWINGS. ANY DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER OF RECORD BEFORE PROCEEDING.
   C. COORDINATE THE EXISTING STRUCTURE WITH THE NEW STRUCTURAL ELEMENTS PRIOR TO DEMOLITION, FABRICATION AND CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACCURATE COORDINATION OF THE ARCHITECTURAL, STRUCTURAL, HVAC, ELECTRICAL, PLUMBING, AND FIRE PROTECTION DRAWINGS.
   D. REQUEST CLARIFICATION REGARDING DISCREPANCIES FOUND IN THE CONSTRUCTION DOCUMENTS. IN ANY CASE OF CONFLICT, BETWEEN THE NOTES, DETAILS, AND SPECIFICATIONS, THE MOST RIGID REQUIREMENTS SHALL GOVERN.
   E. HIRE A GEORGIAREGISTERED PROFESSIONAL ENGINEER FOR THE DESIGN OF ALL SHORING, BRACING, UNDERPINNING, AND OTHER ELEMENTS USED TO SUPPORT THE STRUCTURE DURING CONSTRUCTION.
4. THE CONTRACTOR SHALL BE SOLEY RESPONSIBLE FOR MAINTAINING CONDITIONS OF PUBLIC AND WORKER SAFETY DURING EXECUTION OF THE WORK. THIS SHALL INCLUDE COMPLIANCE WITH ALL OSHA REGULATIONS, AND ALL STATE AND LOCAL LAWS WHICH MAY GOVERN THIS TYPE OF WORK.
5. SUBMIT TO ENGINEER TWO COPIES EACH OF MATERIAL SPECIFICATIONS, ERECTION AND DETAIL DRAWINGS, ETC., OF ALL STRUCTURAL MATERIALS AND CONNECTIONS SUFFICIENTLY IN ADVANCE OF CONSTRUCTION TO PERMIT ADEQUATE TIME FOR REVIEW (10 WORKING DAYS MIN.). ENGINEER TO MARK UP ONE COPY AND FORWARD TO ARCHITECT.
6. THE OWNER, THE ARCHITECT AND THE STRUCTURAL ENGINEER ARE NOT RESPONSIBLE FOR ACCURACY OF THE EXISTING CONDITIONS INFORMATION. EXISTING CONSTRUCTION AT AREAS WHERE NEW WORK IS NOT CONTEMPLATED MAY NOT BE COMPLETELY SHOWN.
7. PATCH AND REPAIR EXISTING CONDITIONS DAMAGED DURING THE COURSE OF NEW CONSTRUCTION AT NO ADDITIONAL COST TO THE OWNER.

SPECIAL INSPECTION NOTES

1. THE FOLLOWING MATERIALS AND METHODS OF CONSTRUCTION SHALL BE SUBJECT TO "SPECIAL INSPECTION" IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE:
   A. STEEL CONSTRUCTION (1705.2)
2. A SPECIAL INSPECTOR HIRED BY THE OWNER SHALL SUPERVISE THE TESTING AND INSPECTION OF THE ABOVE ITEMS. THE INSPECTOR SHALL HAVE QUALIFICATIONS IN ACCORDANCE WITH IBC.
3. THE CONTRACTOR SHALL PROVIDE WRITTEN NOTICE TO THE INSPECTOR FOR ALL ITEMS REQUIRING INSPECTION A MINIMUM OF 72 HOURS PRIOR TO COMMENCEMENT OF THAT PORTION OF WORK. FOR ON-GOING INSPECTIONS THE CONTRACTOR SHALL PROVIDE 24 HRS ADVANCE NOTICE BEFORE INSPECTION.
4. THE CONTRACTOR SHALL PROVIDE SAFE ACCESS AND MEANS TO ALLOW THE ABOVE TESTING AND/OR INSPECTION REQUIREMENTS TO BE COMPLETED IN A MANNER CONSISTENT WITH APPLICABLE OSHA, STATE, AND LOCAL DOB REQUIREMENTS. THE CONTRACTOR SHALL PROVIDE SCAFFOLDING, PERSONNEL HOISTS OR ANY OTHER EQUIPMENT NECESSARY TO ACCESS AREAS SUBJECT TO INSPECTIONS.
5. INSPECTION DOES NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY TO PROVIDE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND PROVIDE THE CONTRACTORS INDEPENDENT QUALITY CONTROL. THE INSPECTOR IS NOT RESPONSIBLE FOR PROVIDING "QUALITY CONTROL" SERVICES FOR THE CONTRACTOR.

STRUCTURAL STEEL NOTES

1. ALL DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", LATEST EDITIONS.
2. ALL BOLTING SHALL CONFORM TO THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS", LATEST EDITION.
3. ALL WELDING SHALL CONFORM TO THE AWS CODE D1.1 "STRUCTURAL WELDING CODE – STEEL", LATEST EDITION.
4. THE FABRICATOR/ERECTOR SHALL SUBMIT TO THE ENGINEER OF RECORD, FOR REVIEW, ENGINEERED AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS AND ERECTION DIAGRAMS FOR ALL STRUCTURAL STEEL. ERECTION DRAWINGS AND JOB STANDARDS DESCRIBING TYPICAL CONNECTION DETAILS AND CAPACITIES SHALL BE SUBMITTED AND APPROVED PRIOR TO THE START OF PIECE DETAILING.
5. UNLESS SPECIFICALLY SHOWN ON THE DRAWINGS, ALL CONNECTIONS SHALL BE DESIGNED AND DETAILED BY A GEORGIA STATE PROFESSIONAL ENGINEER HIRED BY THE FABRICATOR. CALCULATIONS SHALL BEAR THE SEAL AND SIGNATURE OF THIS ENGINEER. DETAILING SHALL BE PERFORMED USING RATIONAL ENGINEERING DESIGN AND STANDARD PRACTICES, PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE CONCEPTUAL DETAILS SHOWN ON DRAWINGS DO NOT INDICATE THE REQUIRED NUMBER OF BOLTS OR WELD SIZES, UNLESS SPECIFICALLY NOTED.
6. ALL STAIR STRUCTURES AND MISCELLANEOUS STEEL SHALL BE DESIGNED AND DETAILED BY A GEORGIA STATE REGISTERED PROFESSIONAL ENGINEER HIRED BY THE CONTRACTOR. THE STAIR FABRICATOR/ERECTOR SHALL SUBMIT TO THE ENGINEER OF RECORD, FOR REVIEW, ENGINEERED AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS, ERECTION DIAGRAMS, AND CALCULATIONS. ALL STAIR STEEL SHOP DRAWINGS AND DESIGN CALCULATIONS SHALL BEAR THE SEAL AND SIGNATURE OF THE CONTRACTOR'S ENGINEER.
7. ALL STRUCTURAL STEEL SHALL COMPLY WITH THE FOLLOWING UNLESS OTHERWISE NOTED:

WIDE FLANGES	ASTM A992 GR 50
SQUARE OR ROUND TUBE	ASTM A500-GR B, FY=46 KSI.
PIPES	ASTM A53, FY=35 KSI
ANGLE AND CHANNEL	ASTM A36
STAINLESS STEEL	AISI 304 OR AISI 316
ANCHOR BOLTS/RODS	ASTM F1554
HIGH STRENGTH BOLTS	ASTM A325 OR A490 (MIN. ¾" DIAM. UNO)
HARDENED WASHERS	ASTM F436
DTI WASHERS	ASTM F959
PLATE	ASTM A572 GR 50

STRUCTURAL STEEL NOTES (Continued)

8. THE FOLLOWING CONNECTIONS SHALL USE SLIP-CRITICAL BOLTS:

   BEAM-TO-COLUMN MOMENT CONNECTIONS
   BEAM-TO-GIRDER MOMENT CONNECTIONS
   WEB SHEAR CONNECTIONS FOR MOMENT CONNECTED BEAMS
   BOLTED BRACE CONNECTIONS
   BOLTED COLUMN SPLICES
   BOLTED BEAM SPLICES

   ALL OTHER CONNECTIONS SHALL BE SIMPLE SHEAR CONNECTIONS UTILIZING FULLY PRETENSIONED HIGH-STRENGTH BOLTS IN BEARING-TYPE CONNECTIONS WITH THREADS INCLUDED IN THE SHEAR PLANE, UNLESS NOTED OTHERWISE.
9. ALL BOLTS IN SLIP CRITICAL CONNECTIONS SHALL UTILIZE DIRECT TENSION INDICATOR WASHERS AND HARDENED WASHERS. DIRECT TENSION INDICATOR WASHERS SHALL COMPLY W/ ASTM 959 AND BE AS MANUFACTURED BY TURNASURE LLC. INSTALL PER RCSC AND MANUFACTURER'S RECOMMENDATIONS.
10. FOR PRETENSIONED AND SLIP CRITICAL JOINTS, PERFORM PRE-INSTALLATION VERIFICATION WITH A TENSION CALIBRATOR (SKIDMORE-WILHELM) AS REQUIRED BY RCSC SECTION 7.
11. MINIMUM SHEAR CONNECTION CAPACITIES SHALL BE AS SHOWN BELOW, UNLESS NOTED OTHERWISE. THE MINIMUM NUMBER OF BOLTS PER CONNECTION SHALL BE TWO (2):

    MINIMUM SHEAR CAPACITIES (SERVICE LOAD)

W4	6 KIPS	W21	70 KIPS
W5	6 KIPS	W24	75 KIPS
W6	6 KIPS	W27	90 KIPS
W8	10 KIPS	W30	115 KIPS
W10	15 KIPS	W33	135 KIPS
W12	25 KIPS	W36	160 KIPS
W14	35 KIPS	W40	185 KIPS
W16	45 KIPS	W44	210 KIPS
W18	60 KIPS		

    DESIGN CHANNEL AND TUBE MEMBERS FOR THE SAME LOAD AS THE SAME DEPTH OF W SHAPE. BEAM REACTIONS EXCEEDING THE ABOVE MINIMUM SHEAR CAPACITIES ARE NOTED ON THE DRAWINGS AND BOXED THUS }--K } AT EACH END OF THE BEAM. DESIGN CONNECTIONS FOR AXIAL FORCES ARE DENOTED }--A }. CONNECTION FORCES GIVEN ON PLAN ARE UNFACTORED AND IN KIPS. THE CONNECTION PATTERN SHALL ENGAGE AT LEAST ½ THE DEPTH OF BEAM.
12. MOMENT CONNECTION SPLICES SHALL DEVELOP THE FULL PLASTIC BENDING CAPACITY OF THE BEAM = FY* ZX/OMEGA AND SHALL HAVE A SHEAR CAPACITY PER STEEL NOTE UNLESS NOTED OTHERWISE.

    MOMENT CONNECTIONS SHALL DEVELOP THE FULL PLASTIC BENDING CAPACITY OF THE BEAM = FY * ZX/Ω AND SHALL HAVE A SHEAR CAPACITY OF 2*FY * ZX/(Ω*BEAM LENGTH) PLUS THE SHEAR CAPACITY REQUIRED PER STEEL NOTE UNLESS NOTED OTHERWISE.

    ALL BRACE CONNECTIONS SHALL DEVELOP THE FULL TENSION CAPACITY OF THE MEMBER = PN/Ω, UNO.
13. ALL WELDING SHALL BE PERFORMED BY GEORGIA LICENSED WELDERS.
14. MINIMUM FILLET WELD SIZES SHALL COMPLY WITH THE AISC, BUT SHALL NOT BE LESS THAN 1/4 INCH, UNLESS NOTED OTHERWISE.
15. ALL FIELD WELDING ELECTRODES SHALL BE E70XX LOW HYDROGEN. SHOP WELDING ELECTRODES SHALL BE E70XX LOW HYDROGEN OR E70T-9 GRADE D. ALL FILLER METAL SHALL HAVE A MINIMUM CHARPY V-NOTCH (CVN) TOUGHNESS OF 20 FT. LBS. AT MINUS 20 DEGREES F.
16. SHOP AND FIELD TESTING OF WELDS AND BOLTS SHALL BE AS FOLLOWS:
    A. ALL WELDS SHALL BE VISUALLY INSPECTED. 15% AT RANDOM SHALL BE MEASURED.
    B. FILLET WELDS FOR BEAM AND GIRDER SHEAR CONNECTION PLATES (15 % AT RANDOM) SHALL BE CHECKED BY MAGNETIC PARTICLE FOR FINAL PASS ONLY.
    C. ULTRASONICALLY TEST 100% OF ALL FULL PENETRATION WELDS.
    D. CHECK ALL PRETENSIONED BOLTS BY EITHER THE T/C METHOD OR BY CALIBRATED TORQUE WRENCH. IN EACH SHEAR CONNECTION, CHECK 100% OF BOLTS WHEN USING T/C METHOD AND 25% OF BOLTS WITH OTHER METHOD FOR EACH SHEAR CONNECTION, BUT NOT LESS THAN TWO (2) BOLTS PER CONNECTION.
    E. CHECK ALL SLIP CRITICAL BOLTS WITH THE "DIRECT TENSION INDICATOR" METHOD. ALL BOLTS SHALL BE VISUALLY INSPECTED. MEASURE WITH FEELER GAGES, AT LEAST 15 PERCENT OF BOLTS IN EACH CONNECTION, BUT NOT LESS THAN TWO BOLTS PER CONNECTION.
    F. THE OWNER'S SPECIAL INSPECTOR OR TESTING AGENCY SHALL PERFORM ALL SHOP AND FIELD INSPECTION AND TESTING AS OUTLINED ABOVE.
17. FABRICATE SIMPLY SUPPORTED BEAMS WITH NATURAL CAMBER UP, FABRICATE CANTILEVER BEAMS WITH NATURAL CAMBER DOWN, UNLESS NOTED OTHERWISE.
18. GAS OR ARC CUTTING OF NEW OR EXISTING SECTIONS IS NOT ALLOWED. GAS OR ARC CUTTING TO ENLARGE EXISTING OR NEW BOLT HOLES IS NOT ALLOWED. GAS OR ARC CUTTING TO CREATE NEW HOLES IN EXISTING OR NEW STEEL IS NOT ALLOWED.
19. FIELD FABRICATED BEAM SPLICES AND FIELD CUTTING OF STRUCTURAL STEEL MEMBERS ARE NOT ALLOWED WITHOUT PRIOR APPROVAL OF THE ENGINEER.
20. ALL NEW HOLES IN EXISTING AND NEW STEEL SHALL BE DRILLED.
21. ALL NEW BEAMS INSTALLED ADJACENT TO PROPOSED FLOOR OPENINGS THROUGH THE EXISTING FLOOR CONSTRUCTION SHALL BE ERECTED, SHIMMED AND DRY PACKED WHERE REQUIRED, PRIOR TO THE CUTTING OF EXISTING FLOOR CONSTRUCTION.
22. ALL STEEL EXPOSED TO WEATHER OR AS INDICATED SHALL BE HOT DIP GALVANIZED PER ASTM A123, 2 OZ. PER SQ. FOOT MINIMUM DEPOSITION, INCLUDING DUNNAGES, STEEL LINTELS, SHELF ANGLES, LINTEL HANGERS, AND ALL OTHER STEEL FRAMING OUTSIDE THE BUILDING ENVELOPE.
23. PROVIDE 3-COAT TNEMEC PAINT SYSTEM (OR APPROVED EQUAL) FOR EXTERIOR STEEL OR AS NOTED AS FOLLOWS:

PRIME COAT:	90-97 TNEME-ZINC AT 2.5 – 3.5 MILS DFT
INTERMEDIATE COAT:	27 FC TYPOXY OR V69F EPOXOLINE II, 4-6 MILS DFT
FINISH COAT:	73 ENDURA-SHIELD AT 2-3 MILS DFT COLOR OF FINISH COAT TO BE SELECTED BY ARCHITECT/OWNER.

24. STEEL PROTECTED FROM WEATHER IN THE FINISHED STRUCTURE, OR WHERE INDICTATED, SHALL BE PRIMED UNLESS NOTED OTHERWISE BY ARCHITECT.
25. STRUCTURAL STEEL TO BE FIREPROOFED SHALL NOT BE PAINTED, AND SHALL BE CLEANED OF ALL RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS PRIOR TO FIREPROOFING APPLICATION.
26. FIREPROOF ALL NEW STRUCTURAL STEEL AND ANY EXISTING STRUCTURAL STEEL EXPOSED DURING THE WORK AS REQUIRED TO MAINTAIN HOURLY RATING, UNLESS NOTED OTHERWISE. CONSULT WITH ARCHITECT REGARDING REQUIRED RATINGS AND FIREPROOFING.
27. ALL STEEL THAT REMAINS EXPOSED TO VIEW IN THE FINISHED STRUCTURE SHALL CONFORM TO THE REQUIREMENTS OF ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS), UNLESS NOTED OTHERWISE.

METAL JOIST NOTES

1. REFER TO FLOOR PLANS FOR ME[illegible] DIMENSIO[illegible]
2. COMPLY WITH MANUFACTURER'S REQUIRE[illegible] FOR THE INSTALLATION [illegible] FASTENING OF METAL FRAMING.
3. THE FLOOR ASSEMBLY SH[illegible] "PLY-CEM [illegible] AND RO[illegible]STEMS" AS DEFINED BY U.S. ARCHITE[illegible] PRODUCTS, [illegible] COMPLY WITH [illegible]FACTURER'S REQUIREMENTS FOR MATERIALS AND [illegible], AS NOTE[illegible]OW.
4. [illegible]-CEM PAN[illegible]ALL BE ¾ [illegible] NOMINAL.
5. [illegible]LS SH[illegible] BE [illegible]ENED TO TH[illegible]OOR JOIST WITH #10 x 1½" FLAT RIBBED REAMER NON-CO[illegible]DING TE[illegible]EW FAST[illegible]ERS AT A MAXIMUM OF 12 INCHES ON CENTER. HEADS OF FASTENE[illegible] SHALL BE [illegible] WITH THE PANEL SURFACE, AND LOCATED AT A MINIMUM OF ½"FROM [illegible] EDGES [illegible] PLY-CEM PANEL. PLACE A ¼" BEAD OF CONSTRUCTION ADHESIVE, CONTE[illegible]200, O[illegible] EQUAL ON THE JOIST FLANGES PRIOR TO MECHANICAL FASTENING OF THE PLY-CE[illegible] [illegible]O THE JOISTS. FASTEN THE PANELS TO THE JOISTS WHILE THE ADHESIVE IS STILL IN A [illegible] CONDITION.
6. [illegible]-CEM PANEL JOINTS SHALL BE SUPPORTED BY THE FRAMING SYSTEM OR THE T-SPLINE.
7. ALL ACCESSORY STEEL SHALL BE 16 GAUGE. ALL JOISTS AND ACCESSORY STEEL SHALL HAVE A MINIMUM G-60 COATING.
8. PROVIDE 2 LINES OF SOLID BRIDGING FOR 3 FEET EACH END WITH CONTINUOUS STRAP BRIDGING (2" WIDE, 18 GAUGE) BETWEEN, AT THIRD POINTS OF JOIST SPAN.
9. ALL STEEL JOISTS AND ACCESSORIES SHALL COMPLY WITH ASTM A446 -STANDARD SPECIFICATION FOR STEEL SHEET, ZINC-COATED (GALVANIZED) BY THE HOT-DIP PROCESS, STRUCTURAL (PHYSICAL) QUALITY. COMPLY WITH ALL OTHER APPLICABLE ASTM STANDARDS. ALL JOISTS AND ACCESSORIES 16 GAUGE OR HEAVIER SHALL BE FORMED STEEL WITH A YIELD OF 50 KSI AND AS SET FORTH IN SECTION 1.2 OF THE AISI "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS". THE MANUFACTURER THAT CERTIFY THAT THE MATERIALS CONFORM TO THE APPROPRIATE REQUIREMENTS.
10. CUTTING OF STEEL FRAMING MEMBERS MAY BE ACCOMPLISHED WITH A SAW OR SHEAR. TORCH CUTTING IS NOT PERMITTED.
11. PROVIDE WEB STIFFENERS AT ALL JOISTS AT INTERIOR STUD BEARING WALLS.
12. PROVIDE JOIST HANGERS WITH THE MINIMUM RATED CAPACITY AS INDICATED ON THE DRAWINGS.
13. TIE PARALLEL JOISTS TO EXTERIOR WALL WITH ½ INCH EXPANSION BOLTS AT 4 FEET MAXIMUM ON CENTER TO MASONRY WALL. GROUT 2 BLOCKS SOLID ALL AROUND AT EACH FLOOR BOTH AT PARALLEL AND PERPENDICULAR JOISTS. PERPENDICULAR JOISTS SHALL BE FULLY MORTARED IN PLACE.
14. COORDINATE ALL FLOOR AND ROOF OPENINGS WITH THE ARCHITECTURAL, MECHANICAL AND PLUMBING DRAWINGS. PROVIDE FIRE STOPPING AS REQUIRED.
15. PROVIDE ADDITIONAL JOISTS UNDER PARALLEL PARTITIONS WHERE THE PARTITION LENGTH EXCEEDS ½ OF THE JOIST SPAN.
16. END BLOCKING SHALL BE PROVIDED WHERE JOIST ENDS ARE NOT OTHERWISE RESTRAINED FROM ROTATION.
17. BEARING OF JOISTS SHALL BE 2 INCHES MINIMUM AT ENDS AND 3½ INCHES MINIMUM AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE ON THE DRAWINGS.
18. EACH JOIST SHALL BE ATTACHED TO, OR RESTRAINED AT, THEIR SUPPORT TO PREVENT LATERAL MOVEMENT OF THE BOTTOM FLANGE.

COLD-FORMED STEEL NOTES

1. ALL WORK SHALL MEET THE REQUIREMENTS OF THE FOLLOWING STANDARDS:
   A. AMERICAN IRON AND STEEL INSTITUTE (AISI) DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS.
   B. AMERICAN WELDING SOCIETY (AWS) D1.1 AND D1.3 SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURE.
   C. AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM)
2. ALL STUD AND TRACK MATERIAL TO CONFORM TO THE FOLLOWING:
   A. 16GA. AND HEAVIER:
      50 KSI MIN. YIELD, 65 KSI MIN. TENSILE STRENGTH ASTM A1003 STRUCTURAL GRADE 50 TYPE H (ST50H)
   B. 18 GA. AND LIGHTER:
      33 KSI MIN. YIELD, 45 KSI MIN. TENSILE STRENGTH ASTM A1003 STRUCTURAL GRADE 33 TYPE H (ST33H)

   TYPICAL STUDS SIZES USED ARE AS FOLLOW (U.O.N. ON PLANS):
   362T162-54
   362S162-54
   600T162-54
   600S162-54
   1200S162-54
   1200T162-54
3. MISCELLANEOUS STEEL TO CONFORM TO THE FOLLOWING:

A. 20GA. – 18GA.	33 KSI MIN. YIELD, 45 KSI MIN. TENSILE
B. 16GA. – 10GA.	50 KSI MIN. YIELD, 65 KSI MIN. TENSILE
C. 3/16" AND HEAVIER	ASTM A36

4. ALL WELDING TO BE PERFORMED BY CERTIFIED LIGHT GAUGE WELDERS CERTIFIED FOR ALL APPROPRIATE DIRECTION COMPLYING WITH AWS D1.3.

   WELDING RODS TO CONFORM TO THE FOLLOWING:

A. 18GA. AND LIGHTER	E60XX
B. 16GA. AND HEAVIER	E70XX OR E6013
C. LT. GAUGE TO STRUCT'L STL.	E70XX LOW HYDROGEN

   WELDING WIRE FOR FCAW TO CONFORM TO THE FOLLOWING:

A. 18GA. AND LIGHTER	E6XT-X OR E7XT-X
B. 16GA. AND HEAVIER	E7XT-X
C. LT. GAUGE TO STRUCT'L STL.	E7XT-X LOW HYDROGEN (EXCLUDES -2, -3, -10, -13, -14X AND -GS SUFFIXES)

5. NOMINAL WELD SIZES FOR WELDING LIGHT GAUGE MATERIAL SHALL BE AS FOLLOWS:

A. 20 GA.	1/16"
B. 18GA.	3/32"
C. 16 GA. AND HEAVIER	1/8"

6. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY OR ON AN ANGLE SUCH AS BRACING TO SQUARELY FIT AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD FIRMLY IN POSITION UNTIL PROPERLY FASTENED.
7. ALL STUDS SHALL BE ATTACHED BY WELDING UNLESS NOTED OTHERWISE. WIRE TYING OF FRAMING COMPONENTS IS NOT PERMITTED.
8. SPLICES IN TOP TRACK ARE REQUIRED WHERE TOP TRACK IS NOT ATTACHED TO A COMMON CONTINUOUS STRUCTURAL MEMBER AND SHALL BE ACCOMPLISHED WITH A NESTED STUD OF THE SAME GAGE AS TRACK WITH A 10" LENGTH AND (4_ #10 S.M.S. EACH SIDE, EACH TRACK U.N.O. ON PLANS.
9. BUTT WELDS OR SPLICES SHALL BE USED AT ALL JOINTS IN TRACK. SPLICES IN AXIAL LOAD STUDS OR BRACES ARE NOT PERMITTED. ALL WELDS SHALL BE PLUG, BUTT, OR SEAM WELDS. WHERE STUDS ARE BURNED THROUGH BY WELDING, PROVIDE SUITABLE STITCH PLATE OF THE SAME GAUGE.
10. ALL CALCULATED STUD PROPERTIES PER AISI SPECIFICATION ARE BASED ON THE FOLLOWING THICKNESS:

A. 10GA. (118 MIL)	0.1242"
B. 12GA. (97 MIL)	0.1017"
C. 14GA. (68 MIL)	0.0713"
D. 16GA. (54 MIL)	0.0566"
E. 18GA. (43 MIL)	0.0451"
F. 20GA. (33 MIL)	0.0346"

11. LATERAL BRIDGING FOR STEEL STUDS IS REQUIRED WHEN WALL BOARD, INSTALLED IN ACCORD WITH UNIFORM BUILDING CODE REQUIREMENTS DOES NOT CONTINUE FULL HEIGHT ON BOTH SIDES. BRIDGING SHALL BE INSTALLED IN ACCORD WITH MANUFACTURER'S RECOMMENDATIONS AND RELATED DETAILS.
12. TRACK SHALL BE UNPUNCHED WITH GAUGE TO MATCH STUD FRAMING UNLESS NOTED OTHERWISE.
13. UTILITY PUNCH HOLES IN STUDS SHALL BE LOCATED AWAY FROM CONNECTIONS.
14. WALL STUD BRIDGING SHALL BE INSTALLED IN A MANNER TO PROVIDE RESISTANCE TO BOTH MINOR AXIS BENDING AND ROTATION. BRIDGING ROWS SHALL BE EQUALLY SPACED AS PER SPECIFICATIONS.
15. AXIAL LOAD BEARING STUDS MUST BE FULLY SEATED INTO THE WALL [illegible], (1/16" MAXIMUM GAP BETWEEN THE STUDS AND THE [illegible]CK WEBS).
16. OPENINGS IN STUD WEBS OTHER THAN STANDARD HOLES PUNCHED BY THE MANUFACTURER ARE PROHIBITED UNLESS SPECIFICALLY DETAILED.
17. ALL EXTERIOR STEEL STUDS SHAL[illegible] MANUFACTURED [illegible] AN SSMA MEMBER AND CONFORM TO ICC REPO[illegible]3P OR APPROVED EQUAL.
18. LOW VELOCITY FASTE[illegible]RS (SHOTPINS[illegible] CALLED OU[illegible]N PLAN SHALL BE SIMPSON PD[illegible] SHOTPINS INSTALLE[illegible] [illegible]ORD WITH [illegible]E REQUIREMENTS OF THE GOV[illegible]THORITY. LOW VE[illegible]ENERS [illegible]ALL BE 0.145" DIAMETER [illegible]" MINIMUM EMB[illegible]ME[illegible]SS N[illegible]ED OTHERWISE.
19. SCREWS SHALL BE SIM[illegible] XQ SERIE[illegible] OR EQU[illegible] DRILLING/SELF-TAPPING ST[illegible]EWS U.[illegible]. INSTALLE[illegible]CORD WITH BUILDING DEPARTMENT. SCRE[illegible] BE [illegible]FFICIENT LEN[illegible]O ENSURE PENETRATION INTO STEEL STUD [illegible] 3 FULL DIAMETER THREADS.

UNISTRUT NOTES

1. DESIGN OF THE UNISTRUT FRAMING SYSTEM (AND ALL CONNECTIONS TO THE BASE BUILDING) TO BE BY CONTRACTOR. SUBMIT SIGNED AND SEALED COLD FORMED METAL FRAMING SHOP DRAWINGS AND CALCULATIONS BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF GEORGIA.
2. PROVIDE ALL UNISTRUT METAL FRAMING MATERIAL, FITTINGS AND RELATED ACCESSORIES (STRUT SYSTEM) AS INDICATED ON THE CONTRACT DRAWINGS.
3. PROVIDE ALL LABOR, SUPERVISION, ENGINEERING, AND FABRICATION REQUIRED FOR INSTALLATION OF THE STRUT SYSTEM IN ACCORDANCE WITH THE CONTRACT DRAWINGS AND AS SPECIFIED HEREIN.
4. THE MANUFACTURER SHALL NOT HAVE HAD LESS THAN 10 YEAR'S EXPERIENCE IN MANUFACTURING STRUT SYSTEMS AND MUST CERTIFY IN WRITING ALL COMPONENTS SUPPLIED HAVE BEEN PRODUCED IN ACCORDANCE WITH AN ESTABLISHED QUALITY ASSURANCE PROGRAM.
5. INSTALLER MUST BE A UNISTRUT TRAINED MANUFACTURER'S AUTHORIZED REPRESENTATIVE/INSTALLER WITH NOT LESS THAN 5 YEARS EXPERIENCE IN THE INSTALLATION OF STRUT SYSTEMS OF THIS SIZE AND CONFORMATION.
6. ALL STRUT SYSTEM COMPONENTS MUST BE SUPPLIED BY A SINGLE MANUFACTURER.
7. WORK SHALL MEET THE REQUIREMENTS OF THE FOLLOWING STANDARDS: FEDERAL, STATE AND LOCAL CODES. AMERICAN IRON AND STEEL INSTITUTE (AISI) SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS LATEST EDITION. AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM).
8. DESIGN OF METAL FRAMING SYSTEM (AND ALL CONNECTIONS TO THE BASE BUILDING) TO BE BY THE CONTRACTOR. SUBMIT STRUCTURAL CALCULATIONS FOR APPROVAL BY THE PROJECT ENGINEER.
9. SUBMIT ALL SHOP/ASSEMBLY DRAWINGS NECESSARY TO COMPLETELY INSTALL THE STRUT SYSTEM IN COMPLIANCE WITH THE CONTRACT DRAWINGS. SUBMIT ALL PERTINENT MANUFACTURERS PUBLISHED DATA.
10. ALL STRUT SYSTEM COMPONENTS SHALL BE AS MANUFACTURED BY UNISTRUT CORPORATION OR APPROVED EQUAL AS DETERMINED BY THE ARCHITECT OR ENGINEER OF RECORD.
11. ALL CHANNEL MEMBERS SHALL BE FABRICATED FROM STRUCTURAL GRADE STEEL CONFORMING TO ONE OF THE FOLLOWING ASTM SPECIFICATIONS: A 1011 SS GR 33, A 653 GR 33.
12. ALL FITTINGS SHALL BE FABRICATED FROM STEEL CONFORMING TO ONE OF THE FOLLOWING ASTM SPECIFICATIONS: A 575, A 576, A 36 OR A 635.

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ISSUE FOR PERMIT / BID

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Bowlmor | AMF
Client Address
222 W44th Street
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Bowlero

Project Address
3067 Washington Rd.
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Issue Drawing Log

	2018.05.29	ISSUE FOR PERMIT
	2018.05.29	ISSUE FOR BID

Seal

Sheet Identification

GENERAL NOTES

S-001

