MAJOR CODES AND STANDARDS

1. 2012 INTERNATIONAL BUILDING CODE WITH GEORGIA STATE AMENDMENTS
2. ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
3. ACI 318-11 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY
4. AISC MANUAL OF STEEL CONSTRUCTION 13th EDITION
5. NDS – NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION 2015 EDITION

DESIGN LOADS

1. ROOF LOADS
   LIVE LOAD ---------- 20 PSF
2. WIND LOAD
   RISK CATEGORY OF BUILDING ---------- II
   ULTIMATE DESIGN WIND SPEED, V_{ult} ---------- 115 MPH
   NOMINAL DESIGN WIND SPEED, V_{asd} ---------- 89 MPH
   WIND EXPOSURE ---------- C
   INTERNAL PRESSURE COEFFICIENT ---------- ±0.18 (RESTROOM)
   BASE PRESSURE, qh ---------- 24.4 PSF
4. SEISMIC LOAD
   SPECTRAL RESPONSE ACCELERATION, S_{DS} ---------- 0.171g
   S_{D1} ---------- 0.132g
   SEISMIC DESIGN CATEGORY ---------- B
   SITE CLASSIFICATION (SOIL TYPE) ---------- D
   SEISMIC USE GROUP ---------- I
   LATERAL LOAD-RESISTING SYSTEM ---------- BEARING WALL SYSTEM – ORDINARY REINFORCED MASONRY SHEAR WALL
   RESPONSE MODIFICATION COEFFICIENT, R ---------- 2.0
   SEISMIC IMPORTANCE FACTOR, I_E ---------- 1.0
   ANALYSIS PROCEDURE ---------- EQUIVALENT LATERAL FORCE PROCEDURE
5. SNOW LOAD
   GROUND SNOW LOAD, P_g ---------- 5.0 PSF
   SNOW EXPOSURE FACTOR, C_e ---------- 1.0
   SNOW IMPORTANT FACTOR, Is ---------- 1.0
   SLOPE FACTOR, Cs ---------- 1.0
   UNIFORM ROOF DESIGN SNOW LOAD ---------- 3.5 PSF

GENERAL NOTES

1. THE CONTRACTOR SHALL COMPLY WITH THE REQUIREMENTS OF THE STATE OF GEORGIA BUILDING CODE AND ALL APPLICABLE FEDERAL AND STATE CODES, STANDARDS, REGULATIONS AND LAWS.
2. WORK NOT INDICATED ON A PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES SHALL BE REPEATED AND INCLUDED IN THE PROJECT.
3. IN THE CASE OF CONFLICTS BETWEEN THE NOTES, DETAILS AND SPECIFICATIONS, THE MOST RIGID REQUIREMENTS SHALL GOVERN.
4. THE CONTRACTOR SHALL NOT MAKE DEVIATIONS FROM THE DESIGN DRAWINGS WITHOUT WRITTEN APPROVAL OF THE ENGINEER.
5. THE CONTRACTOR SHALL TAKE ALL THE NECESSARY PRECAUTIONS/MEASURES TO PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES FROM DAMAGES. THE CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY FOR ANY DAMAGES THAT MAY OCCUR DURING CONSTRUCTION.
6. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS REQUIRED BY HIM TO PERFORM HIS WORK BEFORE STARTING CONSTRUCTION.
7. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS AND DIMENSIONS OF CHASES, INSERTS, OPENINGS, SLEEVES, DRIPS, REVEALS, FINISHES, DEPRESSIONS, DOORS, EXPANSION JOINT MATERIAL AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON STRUCTURAL DRAWINGS.
8. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, CIVIL AND ALL OTHER CONTRACT DRAWINGS RELATED TO OTHER TRADES. THE CONTRACTOR IS RESPONSIBLE TO CHECK AND COORDINATE DIMENSIONS, CLEARANCES, ETC., WITH THE WORK OF OTHER TRADES.
9. JOB SAFETY, CONSTRUCTION PROCEDURES AND CONSTRUCTION MEANS AND METHODS ARE THE RESPONSIBILITY OF THE CONTRACTOR.
10. COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF ALL APPLICABLE SPECIALTY ITEMS INCLUDING BUT NOT LIMITED TO WOOD TRUSSES, AND CURTAINWALL GLAZING SYSTEMS SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF GEORGIA AND SHALL BE AVAILABLE AT THE JOB SITE DURING THE TIMES OF INSPECTION.
11. CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AND BRACING FOR ALL STRUCTURAL COMPONENTS DURING CONSTRUCTION.

FOUNDATION

1. THE NET ALLOWABLE BEARING PRESSURE FOR INDIVIDUAL SPREAD FOOTINGS IS ASSUMED AS 2,500 PSF. IF SITE CONDITIONS INDICATED OTHERWISE, THE ENGINEER OF RECORD SHALL BE NOTIFIED IMMEDIATELY.
2. ALL FOOTING EXCAVATIONS SHOULD BE OBSERVED AND TESTED BY GEOTECHNICAL ENGINEER. IF SOFT, LOOSE OR DISTURBED SOILS ARE ENCOUNTERED IN FOOTING EXCAVATIONS, THE UNSUITABLE SOILS SHOULD BE REMOVED AND REPLACED WITH APPROVED, PROPERLY COMPACTED GRANULAR FILL EXTENDING DOWN TO SUITABLE NATIVE SOILS.
3. ALL STRUCTURAL FILL SHOULD BE COMPACTED TO AT LEAST 95 PERCENT OF THE SOIL'S STANDARD PROCTOR MAXIMUM DRY DENSITY, AS DETERMINED BY ASTM STANDARD D-698. THE UPPER FOOT OF FILL WHICH WILL SUPPORT PAVEMENTS OR SLABS SHOULD BE COMPACTED TO AT LEAST 98 PERCENT OF THE SOIL'S STANDARD PROCTOR MAXIMUM DRY DENSITY FOR IMPROVED SUPPORT. IN AREAS WHICH ARE AT OR ABOVE THE FINISHED GRADE, AND WHICH WILL SUPPORT PAVEMENTS OR SLAB, THE UPPER 8 INCHES IMMEDIATELY BELOW THESE SYSTEMS SHOULD BE SCARIFIED AND RECOMPACTED TO THE 98 PERCENT CRITERIA.
4. IF UNSUITABLE SOILS SUCH AS BURIED TOPSOIL AND LOW STRENGTH OR DISTURBED SOILS ARE ENCOUNTERED BELOW THE DESIGN FOOTING BASE ELEVATION, THE FOOTING EXCAVATIONS SHOULD BE EXTENDED DEEPER TO REACH ADEQUATE BEARING SOILS. THE FOOTINGS COULD BEAR AT THIS LOWER LEVEL OR AN OVEREXCAVATION AND GRANULAR BACKFILL PROCEDURE COULD BE PERFORMED. SEE THE GEOTECHNICAL REPORT FOR THE OVEREXCAVATION AND BACKFILL PROCEDURE.
5. THE OVEREXCAVATION SHOULD THEN BE PROPERLY BACKFILLED UP TO THE DESIGN ELEVATION WITH WELL-GRADED GRANULAR MATERIALS THAT ARE APPROVED BY THE GEOTECHNICAL ENGINEER.
6. THE BASE OF THE FOUNDATION EXCAVATIONS SHOULD BE FREE OF WATER AND LOOSE SOIL PRIOR TO PLACING CONCRETE. CONCRETE SHOULD BE PLACED AS SOON AS POSSIBLE AFTER EXCAVATING TO REDUCE THE POTENTIAL FOR BEARING SOIL DISTURBANCE. SOILS AT BEARING LEVEL THAT BECOME FROZEN, DISTURBED OR SATURATED SHOULD BE REMOVED PRIOR TO PLACING CONCRETE.
7. THE CONTRACTOR SHALL FAMILIARIZE HIMSELF WITH THE SUBSURFACE AND EXISTING CONDITIONS BEFORE COMMENCING WORK.
8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADEQUATE PROTECTION OF ALL EXCAVATION SLOPES.
9. FOR CONCRETE REQUIREMENTS FOR FOOTINGS, SEE REINFORCED CONCRETE NOTES.

ROOF SHEATHING

1. ROOF WOOD STRUCTURAL PANEL (DIAPHRAGMS) SHALL BE MIN. 5/8" APA RATED OSB SHEATHING COVERED IN DOC PS 1 AND PS 2 U.O.N.

METAL CONNECTOR

1. IN HIGHLY CORROSIVE ENVIRONMENTS, SPECIAL APPLIED COATINGS OR STAINLESS STEEL MAY BE REQUIRED.
2. AT THE REQUEST OF ENGINEER, METAL CONNECTOR SUPPLIER SHALL FURNISH A CERTIFIED RECORD THAT MATERIALS COMPLY WITH STEEL SPECIFICATIONS.

SHOP DRAWINGS

1. SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS MUST BE SUBMITTED BY THE CONTRACTOR AND REVIEWED BY THE ENGINEER. IF A CONTRACTOR OR OWNER FAILS TO SUBMIT THE SHOP DRAWINGS, WESTBROOK ENGINEERING, INC. WILL NOT BE RESPONSIBLE FOR THE STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT.
2. SHOP DRAWINGS ARE REVIEWED BY THE ENGINEER AS A CONVENIENCE TO THE CONTRACTOR AND ARE NOT CONTRACT DOCUMENTS.
3. AT THE TIME OF SHOP DRAWING SUBMISSION, THE CONTRACTOR SHALL INFORM THE ENGINEER IN WRITING OF ANY DEVIATIONS OR OMISSIONS FROM THE CONTRACT DOCUMENTS.
4. REPRODUCTION OF STRUCTURAL DRAWINGS FOR USE AS SHOP DRAWINGS SHALL NOT BE PERMITTED.
5. COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN TEAM-OF-RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA AND SHALL BE AVAILABLE AT THE JOB SITE DURING THE TIMES OF INSPECTION INCLUDING GLAZED SYSTEM (INCLUDING BUT NOT LIMITED TO WINDOW UNITS, CURTAIN WALLS AND STOREFRONTS) WHICH EXCEEDS TEN (10) FEET IN HEIGHT, AND STAIRS.

REINFORCED CONCRETE

1. ALL CONCRETE SHALL BE CONTROLLED CONCRETE AND ALL CONCRETING PRACTICES SHALL CONFORM TO ACI 318-11, "AMERICAN CONCRETE INSTITUTE, BUILDING CODE FOR REINFORCED CONCRETE." CONCRETE DETAILS SHALL BE IN ACCORDANCE WITH ACI-135, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" UNLESS OTHERWISE NOTED ON THE DRAWINGS. CONCRETE TESTS FOR THE PRELIMINARY DESIGN MIX PREPARED BY AN APPROVED LABORATORY MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL. CONTRACTOR SHALL PLACE NO CONCRETE WITHOUT THE APPROVED DESIGN MIX.
2. THE CONTRACTOR MUST SUBMIT REINFORCING SHOP DRAWINGS TO THE ENGINEER FOR REVIEW AND APPROVAL. NO REINFORCED CONCRETE CONSTRUCTION IS TO BE STARTED UNTIL THE ENGINEER REVIEWS AND APPROVES THE SHOP DRAWINGS.
3. INSPECTION FOR ALL STRUCTURAL PORTIONS OF THE PROJECT SHALL BE PROVIDED AS REQUIRED PER THE APPLICABLE BUILDING CODE AND PROJECT SPECIFICATIONS. THE ENGINEER MAY VISIT THE SITE TO PROVIDE CONSTRUCTION ASSISTANCE OR TO GENERALLY OBSERVE THE PROGRESS OF CONSTRUCTION. SUCH VISITS ARE NOT TO BE CONSTRUED AS MEETING THE AFORESAID INSPECTION REQUIREMENTS UNLESS THE ENGINEER SPECIFICALLY STATES SO IN WRITING.
4. A RIGID TEMPLATE SHALL BE USED TO INSTALL ALL ANCHOR BOLTS.
5. UNLESS OTHERWISE NOTED ON PLAN, ALL CONCRETE SHALL BE NORMAL WEIGHT WITH 28 DAYS COMPRESSIVE STRENGTH OF 3,000 PSI.
6. ALL REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60 (fy=60,000 PSI) ALL REINFORCING TO BE WELDED SHALL CONFORM TO ASTM A-706 AND THAT WELDING SHALL BE IN ACCORDANCE WITH AWS D1.4, STRUCTURAL WELDING CODE. THE REINFORCING BARS SUPPLIER SHALL PROVIDE THE ENGINEER WITH AN AFFIDAVIT OF THE PRODUCER OF STEEL CERTIFYING THAT THE STEEL MEETS THE REQUIREMENTS OF THE ASTM.
7. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. ROLLED MESH IS NOT ACCEPTABLE.
8. ALL REINFORCEMENT SHALL BE SECURELY HELD IN PLACE WHILE PLACING CONCRETE. IF REQUIRED, THE CONTRACTOR SHALL PROVIDE ADDITIONAL BARS OR STIRRUPS NECESSARY TO SUPPORT ALL BARS AS REQUIRED TO COMPLETE HIS WORK.
9. ALL REINFORCING BARS SHALL BE LAPPED AS SPECIFICALLY DETAILED ON THE DRAWINGS. WHERE NOT SPECIFICALLY INDICATED, ALL REINFORCING BARS SHALL BE LAPPED USING CLASS B TENSION SPLICE LENGTHS, PER THE FOLLOWING TABLE:

REINFORCING BAR LAP SPLICE LENGTHS

BAR SIZE (NO.)	CLASS B TENSION LAP SPLICES		BAR SIZE (NO.)	CLASS B TENSION LAP SPLICES	
	TOP BARS	ALL OTHERS		TOP BARS	ALL OTHERS
3	2'-4"	1'-10"	7	6'-5"	5'-3"
4	3'-1"	2'-5"	8	7'-5"	6'-0"
5	3'-11"	3'-0"	9	8'-5"	6'-9"
6	4'-8"	3'-7"	10	9'-10"	7'-7"

NOTE : TOP BARS ARE DEFINED AS HORIZONTAL BARS SO PLACED THAT 12 IN. OR MORE OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE SPLICE.

10. UNLESS OTHERWISE NOTED ON STRUCTURAL DRAWINGS, PROVIDE MINIMUM CONCRETE COVER FOR REINFORCING BARS AS FOLLOWS:
    CAST AGAINST EARTH ---------- 3"
    EXPOSED TO EARTH OR WEATHER
    #5 AND SMALLER ---------- 1½"
    #6 AND LARGER ---------- 2"
    NOT EXPOSED TO EARTH OR WEATHER
    SLAB AND WALL
    #11 AND SMALLER ---------- ¾"
11. PROVIDE CONTRACTION AND CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI-318 BUILDING CODE. SUBMIT SHOP DRAWINGS SHOWING JOINT LOCATIONS ALONG WITH THE SEQUENCE OF POURS FOR THE ENGINEER'S REVIEW.
12. NO HORIZONTAL CONSTRUCTION JOINTS WILL BE PERMITTED IN BEAMS, WALLS AND SLABS UNLESS SPECIFICALLY SHOWN ON THE DRAWINGS OR APPROVED IN WRITING BY THE ENGINEER.
13. CONCRETE SLAB ON GRADE CONTRACTION JOINT SAW CUTTING SHALL BE DONE WITH SOFT-CUT HIGH SPEED SAW AND SHALL BE COMPLETED WITHIN 4 HOURS AFTER THE SLAB HAS BEEN CAST.
14. ALL CONCRETE PAD AND FILL SLABS SHALL BE REINFORCED WITH 6x6-W2.1xW2.1 MINIMUM, PLACED MID DEPTH OF CONCRETE, U.N.O. ROLLED MESH IS NOT ACCEPTABLE.
15. THE RESULTS FOR ALL COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR.

WOOD AND TIMBER FRAMING

1. FABRICATION AND ERECTION OF STRUCTURAL LUMBER SHALL CONFORM TO THE AMERICAN WOOD COUNCIL (AWC) SPECIFICATIONS.
2. ALL LUMBER DETAILS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF NDS AND THE STATE OF GEORGIA BUILDING CODE.
3. UNTREATED STRUCTURAL LUMBER SHALL BE AS SPECIFIED BELOW, UNLESS OTHERWISE NOTED:
   A. LUMBER SHALL BE GRADED IN ACCORDANCE WITH THE RULES OF ONE OF THE FOLLOWING ASSOCIATIONS: "GRADING RULES FOR SOUTHERN PINE LUMBER" OF THE SOUTHERN PINE INSPECTIONS BUREAU. "STANDARD GRADING AND DRESSING RULES NO. 16" OF THE WEST COAST LUMBER INSPECTION. "GRADING RULES FOR WESTERN LUMBER" PUBLISHED BY WESTERN WOOD PRODUCTS ASSOCIATION.
   B. REDWOOD SHALL CONFORM TO REQUIREMENTS OF "STANDARD SPECIFICATIONS FOR GRADES OF CALIFORNIA REDWOOD LUMBER" OF THE REDWOOD INSPECTION SERVICE.
4. WOOD FRAMING MEMBERS SHALL BE SOUTHERN PINE NO.2 WITH MIN. Fb = 975 PSI; E = 1,400,000 PSI U.O.N.
5. MOISTURE CONTENT OF LUMBER SHALL BE NO LESS THAN 7 PERCENT NOR GREATER THAN 19 PERCENT AT TIME OF ERECTION.
6. MICROLLAM LVL (2.0E) BEAM SHALL HAVE MIN. Fb = 2,600 PSI; E = 2,000,000 PSI U.O.N.
7. UNLESS OTHERWISE NOTED, ALL EXTERIOR BEARING WALL SHALL BE 2x6 STUDS SPACED AT 16" ON CENTER WITH DOUBLE TOP PLATES.
8. UNLESS OTHERWISE NOTED, ALL MEMBER CONNECTIONS SHALL HAVE STANDARD GALVANIZED METAL FRAMING ANCHORS OR CLIPS CONNECTION MEMBERS CARRYING ANY COMBINATION OF DEAD, LIVE OR WIND LOADS. PROVIDE MINIMUM 20 GA. HOT-DIPPED GALVANIZED (G60) FRAMING ANCHORS AT EACH POINT OF BEARING IN ACCORDANCE WITH ASTM A 153M OR ASTM A 641, SUPPLEMENTARY REQUIREMENTS
9. ALL WOOD MEMBERS SHALL BE PRESSURE TREATED U.O.N.
10. WOOD FRAMING SHALL BE BUILT IN ACCORDANCE WITH A.I.T.C. RECOMMENDATIONS.
11. FASTENERS: ASTM A153 HOT-DIP ZINC COATING, OR AISI TYPE 304 STAINLESS STEEL.
12. NAILS, WIRE AND BRADS: FS FF-N-105.
13. POWER DRIVEN FASTENERS: NATIONAL EVALUATION REPORT NER-272.
14. WOOD SCREWS: ANSI B18.6.1.
15. LAG BOLTS: ANSI B18.2.1.
16. BOLTS: STEEL BOLTS, ASTM A307, GRADE A; WITH ASTM A563 HEX NUTS AND FLAT WASHERS.
17. ANCHORING AND NAILING STANDARDS:
    A. PUBLISHED REQUIREMENTS OF MANUFACTURER OF METAL FRAMING ANCHORS.
    B. "TABLE 2304.9.1 – FASTENING SCHEDULE", OF THE IBC 2012.

ABBREVIATIONS

ADDL	=	ADDITIONAL	L.P.	=	LOW POINT
ARCH.	=	ARCHITECTURAL	LT.	=	LIGHT
BAL.	=	BALANCE	L.W.	=	LIGHT WEIGHT
B.L.	=	BRICK LEDGE	MAX.	=	MAXIMUM
BOT.	=	BOTTOM	M.C.	=	MOMENT RESISTING CONNECTION
C.J.	=	CONTRACTION JOINT	MFR.	=	MANUFACTURER
C	=	CENTER LINE	MIN.	=	MINIMUM
CA	=	COLUMN ABOVE	N.F.	=	NEAR FACE
CB	=	COLUMN BELOW	NO.	=	NUMBER
C.C.	=	CENTER TO CENTER	NTS	=	NOT TO SCALE
CL.	=	CLEAR	O.C.	=	ON CENTER
COL.	=	COLUMN	OPNG.	=	OPENING
CONC.	=	CONCRETE	P.A.F.	=	POWDER ACTUATED FASTENERS
CONN.	=	CONNECTION	P.C.	=	PRECAST CONCRETE
CONSTR.	=	CONSTRUCTION	PL	=	PLATE
CONT.	=	CONTINUOUS	P.T.	=	PRESSURE TREATED
DET.	=	DETAIL	REINF.	=	REINFORCEMENT
DIA.	=	DIAMETER	REQD.	=	REQUIRED
DWG.	=	DRAWING	SCHED.	=	SCHEDULE
DWLS	=	DOWELS	SECT.	=	SECTION
EA.	=	EACH	SIM.	=	SIMILAR
E.E.	=	EACH END	S.J.	=	SOFT JOINT
E.F.	=	EACH FACE	S.O.G.	=	SLAB ON GRADE
E.J.	=	EXPANSION JOINT	SQ.	=	SQUARE
EL.	=	ELEVATION	S.S.	=	STAINLESS STEEL
ELECT.	=	ELECTRICAL	ST.	=	STEEL
E.W.	=	EACH WAY	STD.	=	STANDARD
E.O.S.	=	EDGE OF STRUCTURAL SLAB	STIFF.	=	STIFFENER
E.O.D.	=	EDGE OF DECK	S.W.	=	SHORT WAY
EQUIV.	=	EQUIVALENT	SYM.	=	SYMMETRICAL
FIN.	=	FINISHED	T&B	=	TOP & BOTTOM
FL.	=	FLOOR	T.O.F.	=	TOP OF FOOTING
FTG.	=	FOOTING	T.O.ST.	=	TOP OF STEEL
HORIZ.	=	HORIZONTAL	TYP.	=	TYPICAL
H.D.G.	=	HOT DIP GALVANIZED	U.O.N.	=	UNLESS OTHERWISE NOTED
H.P.	=	HIGH POINT	VERT.	=	VERTICAL
JT.	=	JOINT	V.I.F.	=	VERIFY IN FIELD
JS.	=	JOIST SUBSTITUTES	W.P.	=	WORKING POINT
L.L.H.	=	LONG LEG HORIZONTAL	WT.	=	WEIGHT
L.L.V.	=	LONG LEG VERTICAL	W.W.F.	=	WELDED WIRE FABRIC

WOOD TRUSS

1. TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH APPLICABLE PROVISIONS OF LATEST EDITION OF NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (NDS), AMERICAN FOREST AND PAPER ASSOCIATION (AFPA), AND NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION (ANSI/TPI 1), TRUSS PLATE INSTITUTE (TPI), AND CODE OF JURISDICTION.
2. ALL TRUSSES ARE TO BE DESIGNED FOR UPLIFT LOADS REQUIRED BY THE 2012 INTERNATIONAL BUILDING CODE. ALSO, TRUSSES TO BE DESIGNED TO SUPPORT THE MECHANICAL EQUIPMENT IF SHOWN ON THE MECHANICAL DRAWINGS.
3. PREENGINEERED TRUSSES SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA.
4. ALL HARDWARE (BOLTS, HANGERS, STRAPS, ETC.) REQUIRED FOR CONNECTIONS BETWEEN PREENGINEERED TRUSSES SHALL BE DESIGNED AND SPECIFIED BY THE TRUSS DESIGN ENGINEER.
5. FABRICATOR SHALL FURNISH DESIGN DRAWINGS BEARING SEAL AND REGISTRATION NUMBER OF A STRUCTURAL ENGINEER LICENSED IN GEORGIA. DRAWINGS SHALL BE APPROVED BY ARCHITECT PRIOR TO FABRICATION.
6. TRUSS DESIGN DRAWINGS SHALL INCLUDE THE FOLLOWING LOADS AS APPLICABLE:
   A. TOP CHORD LL = 20 PSF TOP CHORD DL = 10 PSF
   B. BOT CHORD LL = 0 PSF BOT CHORD DL = 5 PSF
   C. WIND LOAD = 115 MPH (2012 IBC FIGURE 1609A)
7. LUMBER USED SHALL BE IDENTIFIED BY GRADE MARK OF A LUMBER INSPECTION BUREAU OR AGENCY APPROVED BY BOARD OF REVIEW OF AMERICAN LUMBER STANDARDS COMMITTEE, AND SHALL BE SIZE, SPECIES AND GRADE IN ACCORDANCE WITH DESIGN DRAWINGS.
8. CONNECTOR PLATES SHALL MEET OR EXCEED ASTM A653 REQUIREMENTS FOR STRUCTURAL STEEL.
9. TRUSSES SHALL BE FABRICATED AS SET FORTH IN ANSI/TPI 1-2007 IN ACCORDANCE WITH THE DESIGN DRAWINGS BY AN ESTABLISHED FABRICATOR.
10. TRUSSES SHALL BE HANDLED DURING FABRICATION, DELIVERY AND AT JOB SITE SO AS NOT TO BE SUBJECTED TO EXCESSIVE LATERAL BENDING.
11. INSTALLATION SHALL BE IN ACCORDANCE WITH HANDLING, INSTALLING AND BRACING WOOD TRUSSES, HIB-91, TPI. TRUSSES SHALL BE SET AND SECURED LEVEL AND PLUMB, AND IN CORRECT LOCATION.
12. TRUSSES SHALL BE SUFFICIENTLY BRACED DURING INSTALLATION TO PREVENT TOPPLING OR DOMINOING. INSTALL ALL BRACING BEFORE PLACING CONCENTRATED LOADS ATOP TRUSSES.
13. CUTTING AND ALTERING OF TRUSSES IN NOT PERMITTED.
14. ALL PREENGINEERED TRUSS SHOP DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE DURING THE TIMES OF INSPECTION AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED AND APPROVED BY THE PROJECT STRUCTURAL ENGINEER-OR-RECORD.
15. TRUSSES SHOWN ILLUSTRATE PROFILE OF TRUSS. MINIMUM TOP AND BOTTOM CHORD SIZE – 2x6, AND MINIMUM WEB MEMBER – 2x4, UNLESS NOTED OTHERWISE OR AS DETERMINED BY DETAILED DESIGN.
16. PRESS PLATES SHALL BE PLACED ON EACH SIDE OF TRUSS AT EACH JOINT.
17. TRUSSES AND RAFTERS SHALL BE CONNECTED TO TOP BEARING PLATE WITH SEISMIC TIE DOWN ANCHORS. SIZE AND NUMBER OF TIES TO BE AS REQUIRED TO RESIST FORCE SPECIFIED BY TRUSS DESIGNER. SIMPSON H3 SEISMIC TIES SPECIFIED ON STRUCTURAL SECTIONS AND DETAILS ARE THE MINIMUM SIZE REQUIRED, ADDITIONAL TIES MAY BE REQUIRED TO RESIST FORCE SPECIFIED BY TRUSS DESIGNER. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION WITH TRUSS DESIGN ENGINEER.
18. TRUSS DRAWINGS SHALL SHOW MEMBER FORCES FOR GRAVITY AND UPLIFT CONDITIONS.
19. WHERE ATTACHED CEILINGS (i.e. ATTACHED DIRECTLY TO BOTTOM CHORD) OCCUR, THEY MAY BE CONSIDERED CAPABLE OF PROVIDING LATERAL SUPPORT TO TRUSS BOTTOM CHORD. AT LOCATIONS, WHERE ATTACHED CEILINGS DO NOT OCCUR, BOTTOM CHORD SHALL BE BRACED AT 5'-6" O.C. MAXIMUM.
20. ALL WOOD BEARING SHALL HAVE FULL CONTACT BETWEEN MEMBERS.
21. TRUSS SYSTEM DESIGN ENGINEER SHALL PROVIDE CONNECTIONS FOR TRUSS GIRDERS AT SUPPORTS AND AT ENDS. DO NOT BEAR TRUSS GIRDERS OVER LINTELS AT OPENINGS U.O.N.
22. ROOF TRUSSES, TRUSS SYSTEM, AND WOOD ROOF FRAMING ARE THE RESPONSIBILITY OF THE CONTRACTOR AND HIS DESIGN TEAM.
23. AT LOCATIONS WHERE ENDS OF ROOF TRUSSES LAP CONNECT TRUSSES TOGETHER WITH 10d NAILS @ 6" O.C. VERTICALLY, UNLESS NOTED OTHERWISE.
24. ROOF TRUSSES AND WOOD ROOF FRAMING SHALL BE SUPPORTED BY STRUCTURAL MEMBERS AND LOAD BEARING WALLS SHOWN ON ROOF FRAMING PLAN.
25. THE NUMBER AND SIZE OF NAILS OR STAPLES CONNECTING WOOD MEMBERS SHALL NOT BE LESS THEN THOSE SPECIFIED IN FASTENING SCHEDULE 2304.9.1 OF THE 2012 INTERNATIONAL BUILDING CODE. FOR ATTACHMENT OF WOOD STRUCTURAL PANELS TO FLOOR AND ROOF FRAMING USE 10d NAILS @ 6" O.C. UNLESS NOTED OTHERWISE.
26. TRUSSES TO BE DESIGNED TO TRANSFER NO LATERAL (HORIZONTAL) LOADS DUE TO DEAD OR LIVE LOADS (DOWNWARD) TO TOP OF STUD WALLS AND TOP FLANGE OF BEAMS. TRUSSES TO BE DESIGNED TO TRANSFER LATERAL (WIND AND SEISMIC) FORCES ONLY TO TOP OF WALL AND TOP OF BEAMS.
27. TRUSS DESIGN ENGINEER SHALL PROVIDE CONNECTIONS FOR ALL TRUSS TO TRUSS CONNECTIONS.
28. CONTRACTOR SHALL PROVIDE ADDITIONAL WALL STUDS AT LOCATIONS WHERE TRUSS GIRDERS BEAR ON WALLS.
29. TRUSSES AND TRUSS RAFTERS SHALL SPAN IN DIRECTION INDICATED ON ROOF FRAMING PLAN. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING WITH TRUSS SUPPLIER.
30. TRUSS DEFLECTION DUE TO TOTAL LOAD (D.L. AND L.L.) SHALL NOT EXCEED [illegible] FOR THE WHOLE TRUSS AND EACH CHORD MEMBER.
31. ALL TRUSS CHORD AND WEB MEMBERS AT PANEL POINTS SHALL BE ASSUMED TO BE PINNED AND NOT FIXED.
32. AT PIGGYBACK TRUSSES, TRUSS SYSTEM DESIGNER SHALL DESIGN AND DETAIL CONNECTIONS BETWEEN UPPER AND LOWER TRUSS. TRUSS DESIGNER SHALL DESIGN AND DETAIL LATERAL CHORD BRACING BETWEEN PIGGYBACK TRUSSES.
33. SPACING BETWEEN TRUSS PANEL POINTS AT TRUSS CHORDS SHALL NOT EXCEED 8'-0" AT TRUSS TOP CHORDS AND 10'-0" AT TRUSS BOTTOM CHORDS.
34. TRUSS MANUFACTURER/FABRICATOR SHALL SUPPLY SHOP DRAWINGS THAT INCLUDE TRUSS LAYOUT PLAN, TRUSS ELEVATIONS, TRUSS TO TRUSS CONNECTIONS, TRUSS TO BEARING CONNECTIONS AND DETAILS, DIAPHRAGM SHEAR TRANSFER BRACING, LATERAL WEB BRACING, BOTTOM CHORD BRACING, AND ALL TEMPORARY ERECTION BRACING. SHOP DRAWINGS AND/OR CALCULATIONS SHALL SHOW THE LIVE AND DEAD LOADS AT THE TOP AND BOTTOM CHORD AND THE STRESSES IN EACH TRUSS MEMBER.

MASONRY

1. MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530-05/ASCE 5-05) AND "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1-11/ASCE 6-11) PUBLISHED BY THE AMERICAN CONCRETE INSTITUTE, EXCEPT AS MODIFIED BY THE CONTRACT DOCUMENTS.
2. CONCRETE BLOCK UNITS SHALL BE NORMAL WEIGHT, HOLLOW, LOAD BEARING UNITS AND COMPLY WITH ASTM C90, GRADE N. ALL BLOCK SHALL BE TYPE 1, MOISTURE CONTROLLED UNITS. MASONRY NET AREA COMPRESSIVE STRENGTH (f'm) SHALL BE MINIMUM 1,900 PSI AND SHALL BE VERIFIED EITHER BY PRISM TESTS CONDUCTED PER ASTM C 1314 OR BY UNIT COMPRESSIVE STRENGTH TESTS PERFORMED PER ASTM C 140.
3. ALL MORTAR IS TO BE IN ACCORDANCE WITH ASTM C270. UNLESS NOTED OTHERWISE, MORTAR FOR EXTERIOR AND FOUNDATION WALL MASONRY SHALL BE TYPE "S", PORTLAND CEMENT/LIME MORTAR. MORTAR FOR INTERIOR LOAD BEARING MASONRY SHALL BE TYPE S. MORTAR FOR INTERIOR NON-LOADBEARING MASONRY SHALL BE TYPE N OR S.
4. ALL REINFORCED STEEL SHALL BE IN ACCORDANCE WITH ASTM A615. NEW BILLET STEEL DEFORMED BARS SHALL BE GRADE 60.
5. ALL MASONRY REINFORCED STEEL SHALL BE INSTALLED IN ACCORDANCE WITH ACI 315, "DETAILS AND DETAILING OF CONCRETE REINFORCING".
6. MASONRY JOINT REINFORCEMENT IS TO COMPLY WITH ASTM A951, MILL GALVANIZED CARBON STEEL WIRE FOR INTERIOR WALLS AND HOT-DIP GALVANIZED, CARBON STEEL WIRE FOR EXTERIOR WALLS. HORIZONTAL JOINT REINFORCEMENT IS TO BE MINIMUM 9 GAUGE TRUSS TYPE (DUR-O-WAL DA3100 OR EQUIVALENT) WITH SINGLE PAIR OF SIDE RODS AND CROSS RODS SPACED NO MORE THAN 16" O.C. LAP REINFORCEMENT A MINIMUM OF 6". PLACE MORTAR ON JOINT REINFORCEMENT WHERE THE CAVITY WALL IS 4" OR MORE. HORIZONTAL JOINT REINFORCEMENT SHALL BE PROVIDED WITHIN 16" OF THE TOP AND BOTTOM OF WALLS.
7. PLACE HORIZONTAL BARS IN 8" DEEP BOND BEAM UNITS AT TOP OF ALL WALLS AND AT INTERMEDIATE LEVELS AS INDICATED ON THE DRAWINGS.
8. ALL ANCHOR BOLTS AND REINFORCING IN MASONRY SHALL BE COMPLETELY GROUTED. GROUT FOR COLLAR JOINTS IN WALLS SHALL BE MORTAR. GROUT FOR REINFORCING BARS AND ANCHOR BOLTS SHALL BE SAND AGGREGATE CEMENT GROUT. MASONRY CELLS TO BE GROUTED SHALL BE CLEAR OF MORTAR FINS, DEBRIS OR OTHER OBSTRUCTIONS, SUCH THAT THE FLOW OF GROUT IS NOT INHIBITED.
9. ALL GROUT FOR GROUTING MASONRY BLOCK CORES SHALL BE MIN. 3,000 PSI SAND AGGREGATE CEMENT GROUT, MEETING ASTM C476. SLUMP FOR THIS GROUT SHALL BE 8". MAXIMUM GROUT LIFT TO BE 5 FT. ALLOW A MINIMUM OF 24 HOURS FOR MASONRY TO SET PRIOR TO GROUTING. ROD OR VIBRATE GROUT DURING PLACEMENT TO INSURE SOLID GROUTING. ALLOW AT LEAST 15 MINUTES BETWEEN SUCCESSIVE LIFTS.
10. ALL MASONRY AND COLLAR JOINTS BELOW FINISHED FLOOR ELEVATION ARE TO BE GROUTED SOLID.
11. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF MASONRY CONTROL JOINTS.
12. USE A FULL MORTAR BED FOR FIST STARTER COURSE. USE FACE SHELL MORTAR BEDS ABOVE.
13. ALL REINFORCING BARS ARE TO BE LAPPED IN ACCORDANCE WITH ACI 530-11 SPLICE LENGTH TO BE 48 X BAR DIAMETER MINIMUM BUT NOT LESS THAN 24".
14. VERTICAL AND HORIZONTAL REINFORCEMENT SHALL BE PROVIDED AS SHOWN ON THE DRAWINGS. PLACE VERTICAL REINFORCING BARS AT CORNERS, JAMBS OF OPENINGS, AND IN WALLS AS INDICATED ON THE DRAWINGS. VERTICAL WALL REINFORCING BARS SHALL EXTEND CONTINUOUS FROM FOUNDATION TO EMBED AT LEAST 6" INTO TOP BOND BEAM.
15. REINFORCING STEEL AROUND PERIMETER OF OPENINGS SHALL EXTEND NOT LESS THAN 40 X BAR DIAMETER OR 24", WHICHEVER IS GREATER BEYOND CORNER OF OPENING.
16. DURING ERECTION, COVER TOP OF WALLS WITH WATERPROOF SHEETING AT END OF EACH DAY'S WORK. COVER PARTIALLY COMPLETED STRUCTURES WHEN WORK IS NOT IN PROGRESS.
17. DO NOT APPLY UNIFORM FLOOR OR ROOF LOADING FOR AT LEAST 12 HOURS AFTER BUILDING MASONRY WALLS.
18. DO NOT APPLY CONCENTRATED LOADS FOR AT LEAST 3 DAYS AFTER BUILDING MASONRY WALLS OR COLUMNS.
19. ALL CMU IS TO BE LAID WITH RUNNING BONDS WITH VERTICAL JOINTS LOCATED AT CENTER OF MASONRY UNITS IN THE ALTERNATE COURSE BELOW.
20. PROVIDE CONTINUITY AT ALL WALL CORNERS BY CONFORMING TO ONE OF THE FOLLOWING:
    A. FIFTY PERCENT OF THE MASONRY UNITS AT THE INTERSECTION INTERLOCK.
    B. WALLS SHALL BE REGULARLY TOOTHED WITH 8 IN. MAXIMUM OFFSETS AND ANCHORED BY STEEL CONNECTORS WITH A MINIMUM 1/4"x28" INCLUDING 2" LONG 90 DEGREE BEND AT EACH END T FORM A U OR Z SHAPE. ANCHORS ARE TO BE SPACED VERTICALLY AT 4' O.C.
21. THE ADHESIVE REQUIRED FOR ANCHORAGE INTO MASONRY UNITS SHALL BE HILTI HY 70 HYBRID. OR EQUAL. FOLLOW THE MANUFACTURERS RECOMMENDATIONS FOR INSTALLATION PROCEDURES.
22. ANCHORS USED FOR MASONRY ATTACHMENT SHALL BE STANDARD HAS THREADED ROD MATERIAL MEETING THE REQUIREMENTS OF ASTM A193 GRADE B7. ANCHOR RODS SHALL BE FURNISHED WITH CHAMFERED ENDS.

2012 IBC TABLE 2304.9.1
FASTENING SCHEDULE

CONNECTION	FASTENING[a, m]	LOCATION
1. JOIST TO SILL OR GIRDER	3 – 8d COMMON (2½" x 0.131") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL
2. BRIDGING TO JOIST	3 – 8d COMMON (2½" x 0.131") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL EACH END
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST	2 – 8d COMMON (2½" x 0.131")	FACE NAIL
4. WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST	3 – 8d COMMON (2½" x 0.131")	FACE NAIL
5. 2" SUBFLOOR TO JOIST OR GIRDER	2 – 16d COMMON (3½" x 0.162")	BLIND & FACE NAIL
6. SOLE PLATE TO JOIST OR BLOCKING	16d (3½" x 0.135") AT 16" O.C. 3" x 0.131" NAILS AT 8" O.C. 3" 14 GAGE STAPLES AT 12" O.C.	TYPICAL FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	3 – 16d (3½" x 0.135") AT 16" 4 - 3" x 0.131" NAILS AT 16" 4 - 3" 14 GAGE STAPLES PER 16"	BRACED WALL PANELS
7. TOP PLATE TO STUD	2 – 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	END NAIL
8. STUD TO SOLE PLATE	4 – 8d COMMON (2½" x 0.131") 4 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL
	2 – 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	END NAIL
9. DOUBLE STUDS	16d (3½" x 0.135") AT 24" O.C. 3" x 0.131" NAILS AT 8" O.C. 3" 14 GAGE STAPLES AT 8" O.C.	FACE NAIL
10. DOUBLE TOP PLATES	16d (3½" x 0.135") AT 16" O.C. 3" x 0.131" NAILS AT 12" O.C. 3" 14 GAGE STAPLES AT 12" O.C.	TYPICAL FACE NAIL
DOUBLE TOP PLATES	8 – 16d COMMON (3½" x 0.162") 12 - 3" x 0.131" NAILS 12 - 3" 14 GAGE STAPLES	LAP SPLICE
11. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	3 – 8d COMMON (2½" x 0.131") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL
12. RIM JOIST TO TOP PLATE	8d (2½" x 0.131") AT 6" O.C. 3" x 0.131" NAIL AT 6" O.C. 3" 14 GAGE STAPLE AT 6" O.C.	TOENAIL
13. TOP PLATES, LAPS & INTERSECTIONS	2 – 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
14. CONTINUOUS HEADER, TWO PIECES	16d COMMON (3½" x 0.162")	16" O.C. ALONG EDGE
15. CEILING JOISTS TO PLATE	3 – 8d COMMON (2½" x 0.131") 5 - 3" x 0.131" NAILS 5 - 3" 14 GAGE STAPLES	TOENAIL
16. CONTINUOUS HEADER TO STUD	4 – 8d COMMON (2½" x 0.131")	TOENAIL
17. CEILING JOISTS, LAPS OVER PARTITIONS (SEE SECT. 2308.10.4.1, TABLE 2308.10.4.1)	3 – 16d COMMON (3½" x 0.162") MINIMUM, TABLE 2308.10.4.1 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
18. CEILING JOISTS TO PARALLEL RAFTERS (SEE SECT. 2308.10.4.1, TABLE 2308.10.4.1)	3 – 16d COMMON (3½" x 0.162") MINIMUM, TABLE 2308.10.4.1 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
19. RAFTER TO PLATE (SEE SECT. 2308.10.1, TABLE 2308.10.1)	3 – 8d COMMON (2½" x 0.131") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL
20. 1" DIAGONAL BRACE TO EACH STUD & PLATE	2 – 8d COMMON (2½" x 0.131") 2 - 3" x 0.131" NAILS 2 - 3" 14 GAGE STAPLES	FACE NAIL
21. 1"x8" SHEATHING TO EACH BEARING WALL	3 – 8d COMMON (2½" x 0.131")	FACE NAIL
22. WIDER THAN 1"x8" SHEATHING TO EACH BEARING	3 – 8d COMMON (2½" x 0.131")	FACE NAIL
23. BUILT-UP CORNER STUDS	16d COMMON (3½" x 0.162") 3" x 0.131" NAILS 3" 14 GAGE STAPLES	24" O.C. 16" O.C. 16" O.C.
24. BUILT-UP GIRDER & BEAMS	20d COMMON (4" x 0.192") 32" O.C. 3" x 0.131" NAIL AT 24" O.C. 3" 14 GAGE STAPLE AT 24" O.C.	FACE NAIL AT TOP & BOTTOM STAGGERED ON OPPOSITE SIDES
	2 - 20d COMMON (4" x 0.192") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL AT ENDS AND AT EACH SPLICE
25. 2" PLANKS	16d COMMON (3½" x 0.162")	AT EACH BEARING
26. COLLAR TIE TO RAFTER	3 - 10d COMMON (3" x 0.148") 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
27. JACK RAFTER TO HIP	3 - 10d COMMON (3" x 0.148") 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	TOENAIL
	2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
28. ROOF RAFTER TO 2-BY RIDGE BEAM	2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL
	2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
29. JOIST TO BAND JOIST	3 - 16d COMMON (3½" x 0.162") 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
30. LEDGER STRIP	3 - 16d COMMON (3½" x 0.162") 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL AT EACH JOIST
31. WOOD STRUCTURAL PANELS & PARTICLEBOARD[b] SUBFLOOR, ROOF & WALL SHEATHING (TO FRAMING)	3/8" AND LESS: 6d[c, l]; 2⅜" x 0.113 NAIL[n]; 1¾" 16 GAGE[o] 19/32" TO 3/4": 8d[d] OR 6d[e]; 2⅜" x 0.113 NAIL[p]; 2" 16 GAGE[p] 7/8" TO 1": 8d[c] 1⅛" TO 1¼": 10d[c] OR 8d[d]	
SINGLE FLOOR (COMBINATION SUBFLOOR-UNDERLAYMENT TO FRAMING)	3/4" AND LESS: 6d[e] 7/8" TO 1": 8d[e] 1⅛" TO 1¼": 10d[c] OR 8d[e]	
32. PANEL SIDING (TO FRAMING)	1/2" OR LESS: 6d 5/8": 8d	
33. FIBERBOARD SHEATHING[g]	1/2": NO. 11 GAGE ROOFING NAIL[h]; 6d COMMON NAIL (2"x0.113"); NO. 16 GAGE STAPLE[i] 25/32": NO. 11 GAGE ROOFING NAIL[h]; 8d COMMON NAIL (2½"x0.131"); NO. 16 GAGE STAPLE[i]	
34. INTERIOR PANELING	1/4": 4d[j] 3/8": 6d[k]	

FOR SI: 1 INCH = 25.4 MM.

a. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHEN OTHERWISE STATED.
b. NAILS SPACED AT 6 INCHES ON CENTER AT EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS EXCEPT 6 INCHES AT SUPPORTS WHERE SPANS ARE 48 INCHES OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTION 2305. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE COMMON, BOX OR CASING.
c. COMMON OR DEFORMED SHANK (6d - 2"x0.113"; 8d - 2½"x0.131"; 10d - 3"x0.148").
d. COMMON (6d - 2"x0.113"; 8d - 2½"x0.131"; 10d - 3"x0.148").
e. DEFORMED SHANK (6d - 2"x0.113"; 8d - 2½"x0.131"; 10d - 3"x0.148").
f. CORROSION-RESISTANT SIDING (6d - 1⅞"x0.106"; 8d - 2⅜"x0.128") OR CASING (6d - 2"x0.099"; 8d - 2½"x0.113") NAIL.
g. FASTENERS SPACED 3 INCHES ON CENTER AT EXTERIOR EDGES AND 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS, WHEN USED AS STRUCTURAL SHEATHING. SPACING SHALL BE 6 INCHES ON CENTER ON THE EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS FOR NONSTRUCTURAL APPLICATIONS.
h. CORROSION-RESISTANT ROOFING NAILS WITH 7/16-INCH-DIAMETER HEAD AND 1½-INCH LENGTH FOR ½-INCH SHEATHING AND 1¾-INCH LENGTH FOR 25/32-INCH SHEATHING.
i. CORROSION-RESISTANT STAPLES WITH NOMINAL 7/16-INCH CROWN AND 1⅛-INCH LENGTH FOR ½-INCH SHEATHING AND 1½-INCH LENGTH FOR 25/32-INCH SHEATHING. PANEL SUPPORTS AT 16 INCHES (20 INCHES IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS OTHERWISE MARKED).
j. CASING (1½"x0.080") OR FINISH (1½"x0.072") NAILS SPACED 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORT.
k. PANEL SUPPORTS AT 24 INCHES. CASING OR FINISH NAILS SPACED 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS.
l. FOR ROOF SHEATHING APPLICATIONS, 8d NAILS (2½"x0.113") ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL PANELS.
m. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16-INCH.
n. FOR ROOF SHEATHING APPLICATIONS, FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.
o. FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS FOR SUBFLOOR AND WALL SHEATHING AND 3 INCHES ON CENTER AT EDGES, 6 INCHES AT INTERMEDIATE SUPPORTS FOR ROOF SHEATHING.
p. FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.

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