1. BUILDING CODE - 2012 INTERNATIONAL BUILDING CODE w/ LOCAL AMENDMENTS

2. DESIGN LOADS:

A. FLOOR LIVE LOADS

a.	CORRIDORS, RAMPS, STAIRS & EXITWAYS	100 PSF
b.	PARTITIONS	15 PSF
c.	PROJECTION BOOTHS	50 PSF
d.	STADIUM SEATING (NON-REDUCED)	60 PSF
e.	CONCENTRATED LOAD ON ELEVATED FLRS	2000 LBS
f.	CONCENTRATED LOAD ON STAIR TREADS	300 LBS
g.	SCREEN FRAME CATWALKS FOR MAINTENANCE ACCESS	40 PSF

B. ROOF LIVE LOAD 20 PSF

C. ROOF SNOW LOAD

a.	GROUND SNOW LOAD, P_g	5 PSF
b.	FLAT ROOF SNOW LOAD, P_f	5.5 PSF
c.	SNOW EXPOSURE CATEGORY, C_e	1.0
d.	RISK CATEGORY	III
e.	SNOW LOAD IMPORTANCE FACTOR, I_s	1.1
f.	THERMAL FACTOR, C_t	1.0
g.	DRIFTING	PER CODE

D. WIND LOADS

a.	BASIC WIND SPEED (3 SECOND GUST), V(ULT)	115 MPH
b.	RISK CATEGORY	III
c.	WIND EXPOSURE	C
d.	INTERNAL PRESSURE COEFFICIENT	± 0.18

E. SEISMIC LOADS

a.	RISK CATEGORY	III
b.	SPECTRAL ACCELERATION	S_s = 0.193g, S_1= 0.091g
c.	SEISMIC IMPORTANCE FACTOR	
1.	I_e	1.25
2.	I_e (STAIRS AND EXITS)	1.5
3.	I_e (ALL OTHERS)	1.0
4.	I_e (PERPENDICULAR TO SCREEN)	1.0
5.	I_e (PARALLEL TO SCREEN)	1.25
d.	SPECTRAL RESPONSE COEFFICIENTS S_{DS}	0.206
	S_{D1}	0.146
e.	SITE CLASS	D
f.	SEISMIC DESIGN CATEGORY	C
g.	SEISMIC BASE SHEAR	N/A*

* RENOVATION DOES NOT IMPACT FORCES IN THE EXISTING LATERAL FORCE RESISTING ELEMENTS BY MORE THAN THE CODE ALLOWABLE 10%, SEE EXEMPTION 3404.4 OF THE 2012 IBC.

F. DEAD LOADS

a.	BUILT-UP RISER STRUCTURE	ACTUAL WEIGHT

G. THE STRUCTURAL SYSTEMS SHOWN ON THESE DOCUMENTS HAVE BEEN DESIGNED FOR THE FINAL IN PLACE USAGE OF THE STRUCTURE BASED ON THE INTENDED OCCUPANCY AND CODE REQUIREMENTS. WHILE GENERAL CONSTRUCTABILITY HAS BEEN CONSIDERED, THE STRUCTURAL SYSTEMS HAVE NOT BEEN DESIGNED TO ACCOMMODATE SPECIFIC CONSTRUCTION MEANS AND METHODS THAT MIGHT BE UTILIZED BY THE CONTRACTOR.

H. REFER TO SHEET S4.1 FOR SCREEN STRUCTURE DESIGN LOAD INFORMATION.

3. STATEMENT OF SPECIAL INSPECTIONS

A. THIS STATEMENT OF SPECIAL INSPECTIONS IS IN ACCORDANCE WITH SECTION 1704.3 OF THE 2012 INTERNATIONAL BUILDING CODE (IBC 2012). THE INTENT OF THIS SECTION IS THAT ALL SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE PROVISIONS OF CHAPTER 17 OF THE 2012 IBC. UNLESS SPECIFICALLY NOTED OTHERWISE, ADDITIONAL SPECIAL INSPECTIONS MAY BE REQUIRED BY LOCAL CODE OR BUILDING OFFICIAL, AND IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ANY ADDITIONAL REQUIREMENTS ABOVE AND BEYOND THE REQUIRED SPECIAL INSPECTIONS INDICATED.

B. SPECIAL INSPECTIONS SHALL BE PERFORMED AS SPECIFIED IN THE SCHEDULE OF SPECIAL INSPECTIONS, SEE SCHEDULE 1/S0.2.

C. THE CONTRACTOR SHALL REQUEST SPECIAL INSPECTION OF THE ITEMS LISTED IN THE AFOREMENTIONED SCHEDULE PRIOR TO THOSE ITEMS BECOMING INACCESSIBLE AND UNOBSERVABLE DUE TO PROGRESSION OF THE WORK. THE CONTRACTOR SHALL PROVIDE SAFE ACCESS TO THE JOB SITE AND ITEMS TO BE INSPECTED. SAFE ACCESS INCLUDES, BUT IS NOT LIMITED TO, LADDERS, SCAFFOLDING AND/OR CONTRACTOR OPERATED LIFTS AS REQUIRED FOR SITE OBSERVATION.

D. SPECIAL INSPECTOR SHALL PROVIDE BI-WEEKLY SPECIAL INSPECTION REPORTS AND SHALL DISTRIBUTE THESE REPORTS TO THE BUILDING OFFICIAL, OWNER, CONTRACTOR, ARCHITECT, STRUCTURAL ENGINEER-OF-RECORD AND MECHELEC/PLUMBING ENGINEER-OF-RECORD. SPECIAL INSPECTION REPORTING SHALL BE IN ACCORDANCE WITH SECTION 1704.2.4 OF THE 2012 IBC.

E. ALL DISCREPANCIES NOTED DURING INSPECTIONS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR. IF LEFT UNCORRECTED, THESE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE APPROPRIATE DESIGN PROFESSIONALS AND/OR BUILDING OFFICIAL. THE INSPECTOR IS NOT AUTHORIZED TO APPROVE DEVIATIONS FROM THE CONTRACT DRAWINGS.

4. STRUCTURAL ENGINEER SITE OBSERVATIONS

A. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND, EXCEPT WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE.

B. THE ENGINEER SHALL NOT HAVE CONTROL NOR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSION OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

C. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF PMA ENGINEERING IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN GENERAL ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.

5. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS PRIOR TO FABRICATION OF NEW STRUCTURAL COMPONENTS. CONTRACTOR SHALL CARRY ADEQUATE CONTINGENCY TO COVER UNKNOWN CONDITIONS OR WHERE NEW DIMENSIONS DO NOT MATCH THE EXISTING STRUCTURE. CONTRACTOR SHALL NOTIFY ARCHITECT AND ENGINEER IF THE STRUCTURE DIFFERS FROM THESE CONTRACT DOCUMENTS.

6. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OTHER PERTINENT INFORMATION RELATED TO THE STRUCTURAL WORK AND COORDINATE AS REQUIRED. THESE STRUCTURAL DRAWINGS ARE INTENDED TO BE UTILIZED AS A COMPLETE SET OF DOCUMENTS THAT REPRESENT THE BUILDINGS STRUCTURAL SYSTEMS. NO SINGLE SHEET OR SERIES OF SHEETS IS INTENDED TO "STAND ALONE". THESE STRUCTURAL DRAWINGS ARE INTENDED TO BE INCLUDED IN A COMPLETE SET OF CONSTRUCTION DOCUMENTS, INCLUDING BUT NOT LIMITED TO: ARCHITECTURAL DRAWINGS AND MECHANICAL/ELECTRICAL/ PLUMBING DRAWINGS. CONTRACTOR SHALL VERIFY COORDINATION OF THESE DRAWINGS WITH CONTENTS OF ABOVE DRAWING SETS SPECIFIED AND ONLY PROCEED WITH BIDDING AND CONSTRUCTION AFTER SUCH HAS TAKEN PLACE.

7. DETAILS LABELED "TYP" OR "TYPICAL" ARE TO BE APPLIED AT LOCATIONS THAT ARE SAME OR SIMILAR TO THOSE SPECIFICALLY INDICATED. WHERE A DETAIL IS NOT INDICATED, THE DETAIL SHALL BE THE SAME AS FOR SIMILAR CONDITIONS OR AS SHOWN IN THE "TYPICAL DETAILS".

8. THE NEW CONSTRUCTION IS NOT STRUCTURALLY STABLE UNTIL ALL CONNECTIONS, FRAMING, DIAPHRAGMS, PERMANENT BRACING AND METAL DECKING ARE COMPLETE AND HAVE ACHIEVED DESIGN STRENGTH. CONTRACTOR IS SOLELY RESPONSIBLE FOR MAINTAINING STRUCTURAL STABILITY DURING ERECTION AND CONSTRUCTION. TEMPORARY BRACING SYSTEMS ARE NOT TO BE REMOVED UNTIL STRUCTURAL WORK IS COMPLETE.

9. REINFORCING STEEL:

A. ALL REINFORCING STEEL SHALL BE ASTM A615 GRADE 60, EXCEPT WELDED REINFORCING WHICH SHALL BE ASTM A706 GRADE 60.

B. ALL WELDED WIRE FABRIC SHALL BE ASTM A185 AND ARE COLD DRAWN WIRE.

C. ALL ACCESSORIES FOR SUPPORTING REINFORCING SHALL BE GALVANIZED OR HAVE PLASTIC-COATED FEET.

D. REINFORCING SHALL BE DETAILED, FABRICATED, PLACED, AND SUPPORTED IN ACCORDANCE WITH ACI 315, LATEST EDITION.

E. STANDARD COVERAGE OF REINFORCING, UNLESS OTHERWISE SHOWN, SHALL BE AS FOLLOWS:

CAST AGAINST EARTH PERMANENTLY EXPOSED TO WEATHER	3"
EXPOSED TO EARTH OR WEATHER (FORMED)	2"
NOT EXPOSED TO EARTH OR WEATHER: SLABS, WALLS	3/4"

10. CONCRETE:

A. CAST-IN-PLACE CONCRETE CONSTRUCTION SHALL COMPLY WITH THE APPLICABLE BUILDING CODE REQUIREMENTS, INDUSTRY GUIDES AND REFERENCE STANDARDS INCLUDING, BUT NOT LIMITED TO:

a. ACI 301 - SPECIFICATIONS FOR STRUCTURAL CONCRETE
b. ACI 305R - GUIDE TO HOT WEATHER CONCRETING
c. ACI 306R - GUIDE TO COLD WEATHER CONCRETING
d. ACI 318 - STRUCTURAL CONCRETE BUILDING CODE
e. ACI 347 - GUIDE TO FORMWORK FOR CONCRETE
f. ACI SP-66 - ACI DETAILING MANUAL
g. AWS D1.4 - STRUCTURAL WELDING CODE - REINFORCING STEEL
h. CRSI - MANUAL OF STANDARD PRACTICE

B. ALL CONCRETE, UNLESS OTHERWISE NOTED, SHALL DEVELOP A 28 DAY COMPRESSIVE STRENGTH OF 4,000 PSI AND HAVE MAXIMUM WATER/CEMENT RATIO OF 0.53.

C. IT IS THE INTENT OF THESE CONCRETE SPECIFICATIONS THAT THE CONTRACTOR SUPPLY CONCRETE MIXES WITH A MINIMUM AMOUNT OF WATER IN ORDER TO LIMIT PLASTIC SHRINKAGE CRACKING IN FRESHLY PLACED CONCRETE. IT IS EXPECTED THAT PRODUCING WORKABILITY FOR CONCRETE MIXES WILL REQUIRE THE ADDITION OF WATER-REDUCING AND/OR SUPER-PLASTICIZING CHEMICAL ADMIXTURES.

D. CONCRETE SLUMP SHALL BE A MAXIMUM OF 4" +/- 1" (ASTM C-143) AS DELIVERED IN THE FIELD. CONTRACTOR MAY USE CHEMICAL ADMIXTURES TO ATTAIN A MAXIMUM SLUMP OF 8" FOR WORKABILITY.

E. NO WATER MAY BE ADDED TO THE CONCRETE MIX ON SITE.

F. FLY ASH MAY BE USED AT A RATE NOT TO EXCEED 25% OF THE TOTAL CEMENT CONTENT.

G. CHAMFER ALL EXPOSED CORNERS OF CONCRETE 3/4".

H. PRIOR TO PLACING CONCRETE IN ANY LOCATION, IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO HAVE THOROUGHLY CHECKED AND COORDINATED ALL DIMENSIONS, ELEVATIONS, OPENINGS, RECESSES, AND BLOCKOUTS SHOWN ON THE ARCHITECTURAL, STRUCTURAL, AND MECHANICAL DRAWINGS. IN THE EVENT ERRORS, CONFLICTS, OR OMISSIONS EXIST, IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO CONTACT THE ARCHITECT OR ENGINEER FOR NECESSARY CORRECTIVE ACTION.

I. EMBEDDED ITEMS ARE TO BE FURNISHED AND INSTALLED BY THE CONTRACTOR PRIOR TO PLACING CONCRETE.

J. ANCHOR RODS SHALL BE HELD IN PLACE WITH A RIGID TEMPLATE.

11. STRUCTURAL STEEL:

A. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN, LATEST EDITION AND AISC CODE OF STANDARD PRACTICE.

B. ALL STRUCTURAL STEEL FOR WIDE FLANGE & WT SHAPES SHALL BE A992 GRADE 50 UNLESS NOTED OTHERWISE ON THE PLANS. ALL ANGLES, PLATES, AND CHANNELS AND SHALL BE ASTM A36 UNLESS NOTED OTHERWISE. ALL RECTANGULAR AND ROUND HSS SHAPES SHALL BE ASTM A500, GRADE B.

C. ALL STRUCTURAL STEEL WELDS IN THE SHOP OR THE FIELD SHALL BE PERFORMED BY A QUALIFIED WELDER AND SHALL CONFORM TO THE CURRENT REQUIREMENTS OF A.W.S.

D. SHOP WELDED AND FIELD BOLTED CONNECTIONS ARE PREFERRED, UNLESS OTHERWISE SHOWN.

E. FILLET WELDS NOT SPECIFICALLY SIZED IN THESE DOCUMENTS SHALL BE THE MINIMUM SIZE IN ACCORDANCE WITH AWS D1.1 LATEST EDITION, DEPENDENT ON THE THINNER PART JOINED, BUT NO LESS THAN 3/16".

F. WELDING ELECTRODES SHALL BE E70XX.

G. STEEL ROOF DECK SHALL BE 1-1/2", WIDE RIB, 18 GAGE, WITH NESTED SIDELAPS. DECK SHALL BE G60 GALVANIZED.

H. ALL OPENINGS IN ROOF OVER 6" IN ANY DIRECTION SHALL HAVE A 3x3x1/4 ANGLE FRAME (4 SIDES) BETWEEN ROOF FRAMING MEMBERS, UNLESS OTHERWISE NOTED ON DRAWINGS.

I. PROVIDE 4x4x1/4 ANGLE BETWEEN JOISTS AT EACH EDGE OF RTU. COORDINATE LOCATION WITH MECHANICAL.

12. POST-INSTALLED ANCHORAGE:

A. DESIGN OF ALL POST-INSTALLED ANCHORAGE SHALL BE IN ACCORDANCE WITH ACI-318 APPENDIX D AND SHALL CONSIDER CRACKED CONCRETE CONDITIONS.

B. ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED BY TRAINED PERSONNEL PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) AS SHOWN IN THE CORRESPONDING ICC-ESR REPORT AND INCLUDED IN THE ANCHOR PACKAGING

C. THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL POST-INSTALLED ANCHORAGE ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.

D. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. EXISTING REINFORCING BARS SHALL NOT BE CUT UNLESS NOTED OTHERWISE ON THE DRAWINGS. THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS BY FERROSCAN, GPR, X-RAY OR OTHER MEANS PRIOR TO INSTALLATION OF ANCHORS.

E. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.

F. EMBEDMENT DEPTH FOR MECHANICAL EXPANSION ANCHORS SHALL BE DEFINED AS THE DISTANCE FROM THE SURFACE OF THE LOAD-BEARING BASE MATERIAL TO THE DEEPEST PART OF THE ANCHOR WHICH TENSION LOAD IS TRANSFERRED TO THE CONCRETE, MEASURED PRIOR TO APPLYING TORQUE TO THE ANCHOR.

G. EMBEDMENT DEPTH FOR ADHESIVE AND SCREW TYPE ANCHORS SHALL BE DEFINED AS THE DISTANCE FROM THE SURFACE OF THE LOAD-BEARING BASE MATERIAL TO THE DEEPEST PART OF THE ANCHOR AFTER THE ANCHOR HAS BEEN INSTALLED INTO THE HOLE.

H. SUBMITTALS ARE THE CONTRACTOR'S RESPONSIBILITY AND MUST INCLUDE EVALUATION REPORTS FROM THE INTERNATIONAL CODE COUNCIL.

I. CONCRETE ANCHORS:

a. MECHANICAL ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193.

b. ADHESIVE ANCHORING SYSTEMS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308.

13. LIGHT GAGE METAL FRAMING:

A. ALL LIGHT GAGE METAL FRAMING AND CONNECTIONS SHALL BE FABRICATED, AND ERECTED IN ACCORDANCE WITH AISI "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AND SDI-WM MLSFA "LIGHT WEIGHT STEEL FRAMING SYSTEMS MANUAL."

B. ALL LIGHT GAGE METAL FRAMING IDENTIFICATION SHALL CONFORM TO THESE DOCUMENTS IS IN ACCORDANCE WITH THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA).

C. ALL STRUCTURAL MEMBERS SHALL BE FORMED FROM STEEL HAVING A GALVANIZED COATING MEETING THE REQUIREMENTS OF ASTM A653. STEEL MATERIAL SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI (EXCEPT TRACKS USED AT 54 MIL THICKNESS OR HEAVIER SHALL HAVE A MINIMUM YIELD STRESS OF 50 KSI).

D. ALL CONNECTIONS SHALL BE MADE WITH A MINIMUM OF (4) #10-16 SCREWS OR EQUIVALENT WELDS (UNLESS NOTED OTHERWISE ON THE DRAWINGS). FASTENERS FOR FIRE-RETARDANT-TREATED PLYWOOD SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. IN THE ABSENCE OF MANUFACTURER'S RECOMMENDATIONS FASTENER COATING SHALL BE HOT DIPPED GALVANIZED OR ZINC COATED IN ACCORDANCE WITH ASTM B 695, CLASS 55 MINIMUM.

E. ALL SCREWS SHALL BE OF THE DIAMETER AND SIZE INDICATED ON THE DRAWINGS, AND SHALL BE THOSE MANUFACTURED AND TESTED BY ITW BUILDEX. MINIMUM SPACING AND EDGE DISTANCE FOR SCREWS SHALL BE THE GREATER OF EITHER 1/2 INCH OR 3 X SHANK DIAMETER, UNLESS NOTED OTHERWISE ON THE DRAWINGS. SCREW PENETRATION THROUGH JOINED MATERIALS SHALL NOT BE LESS THAN THREE EXPOSED THREADS. SELECT SCREWS WITH AN ADEQUATE CUTTING TIP TO ACCOMMODATE THE TOTAL THICKNESS TO BE DRILLED. DRILLING MUST BE COMPLETED BEFORE THE THREADS ENGAGE THE MATERIAL. WHERE SCREW ATTACHMENTS ARE MADE TO FRAMING COMPONENTS OF DIFFERENT THICKNESSES, THE THINNEST COMPONENT MUST BE PENETRATED FIRST. (UNLESS NOTED OTHERWISE) ALL SCREWS ARE TYPE "T1".

F. WELDING SHALL BE DONE IN ACCORDANCE WITH AWS D1.3 - LATEST EDITION, STRUCTURAL WELDING CODE, SHEET STEEL.

G. SUGGESTED WELD METAL AND PROCESS FOR SHOP WELDING ARE: 70 KSI WELD METAL STRENGTH (MINIMUM). SUGGESTED METHODS FOR FIELD WELDING: 1-5/8" (UNLESS NOTED OTHERWISE) E70XX (MINIMUM) ELECTRODE - SMAW; OR "GASLESS" MIG.

H. MINIMUM WELD THROAT THICKNESS (t) MUST MATCH OR EXCEED THE BASE STEEL THICKNESS OF THE THINNEST CONNECTED PART UNLESS NOTED OTHERWISE.

I. SEQUENCING OF WELDS SHALL BE SO AS TO AVOID DISTORTION OF MEMBERS. REPLACE ALL MEMBERS WHEN BURN THROUGH OCCURS DURING WELDING.

J. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR AS REQUIRED ON ANGULAR FIT AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED.

K. NO SPLICES IN STUDS, JOISTS, OR OTHER LOAD CARRYING MEMBERS MAY BE MADE WITHOUT PRIOR ENGINEERING REVIEW AND SPECIFIC DETAILS FOR ANY SUCH SPLICE(S).

L. TRACK MEMBERS SHALL MATCH DEPTH AND MIL THICKNESS (GAGE) OF STUDS UNLESS NOTED OTHERWISE.

M. BRIDGING SHALL BE INSTALLED, IN THE FORM OF COLD ROLLED CHANNELS AND CONNECTION CLIPS, IN ALL WALLS ON THIS PROJECT. SEE TYPICAL LIGHT GAGE METAL FRAMING DETAILS ON THE DRAWINGS FOR SPECIFIC REQUIREMENTS.

N. ALL LIGHT GAGE METAL FRAMING SHOWN ON THESE DOCUMENTS IS BASED ON THE PHYSICAL AND STRUCTURAL PROPERTIES OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA). ANY SUBSTITUTIONS MUST MEET THE MINIMUM PROPERTIES PROVIDED BY THESE MEMBERS, AND MUST BE APPROVED IN WRITING BY THE ENGINEER.

14. SUBMITTALS:

A. CODE COMPLIANT STRUCTURAL DESIGN OF THE FOLLOWING ITEMS IS DEFERRED TO THE GENERAL CONTRACTOR.

a. TEMPORARY BRACING AND SHORING

B. DEFERRED SUBMITTALS SHALL INCLUDE SUBSTANTIATING STRUCTURAL CALCULATIONS AND SHALL BEAR THE SIGNED WET STAMP OF A REGISTERED PROFESSIONAL ENGINEER WHO IS LEGALLY AUTHORIZED TO PRACTICE IN THE JURISDICTION WHERE PROJECT IS LOCATED AND WHO IS EXPERIENCED IN PROVIDING ENGINEERING SERVICES OF THE KIND INDICATED. DEFERRED SUBMITTALS SHALL BEAR THE APPROVAL STAMP OF THE PROJECT ENGINEER OF RECORD.

C. ALL SHOP DRAWINGS AND SUBMITTALS MUST BE REVIEWED AND APPROVED BY THE CONTRACTOR PRIOR TO SUBMITTAL. ENGINEER'S REVIEW OF SHOP DRAWINGS IS LIMITED TO CHECKING FOR GENERAL CONFORMANCE WITH DESIGN DRAWINGS AND STRENGTH OF COMPONENTS AND MATERIALS. CONTRACTOR IS RESPONSIBLE FOR ANY CHANGES FROM THE DESIGN DRAWINGS, QUANTITIES, DIMENSIONAL ERRORS OR OMISSIONS IN THE SHOP DRAWINGS.

D. ALL SHOP DRAWINGS MUST BE ORIGINAL DOCUMENTS AND SHALL NOT BE REPRODUCTIONS OF THESE CONTRACT DOCUMENTS.

E. SUBMIT SHOP DRAWINGS DETAILING FABRICATION OF EACH MEMBER AND ITS CONNECTIONS. DETAIL DRAWINGS ARE TO BE PREPARED UNDER THE SUPERVISION OF A LICENSED PROFESSIONAL ENGINEER.

F. CONTRACTOR SHALL SUBMIT STRUCTURAL SHOP DRAWINGS FOR THE FOLLOWING:

a. CONCRETE MIX DESIGN AND MATERIALS
b. CONCRETE REINFORCING STEEL
c. STRUCTURAL STEEL
d. STEEL DECK & ATTACHMENT METHODS AND MATERIALS
e. LIGHT GAGE METAL FRAMING & CONNECTIONS

1 PLFS0.1 STRUCTURAL GENERAL NOTES

PLFS0.1 GENERAL NOTES
PLFS0.2 TYPICAL DETAILS
PLFS0.3 TYPICAL DETAILS
PLFS0.4 TYPICAL DETAILS
PLFS0.5 TYPICAL DETAILS
PLFS4.1 SCREEN STRUCTURE FRONT ISOMETRIC
PLFS4.2 SCREEN STRUCTURE BACK ISOMETRIC
PLFS4.3 SCREEN STRUCTURE SUPPORT DETAILS
PLFS4.4 SCREEN STRUCTURE ISOMETRIC DETAILS
PLFS4.5 SCREEN SPEAKER MOUNTING DETAILS
PLFS4.6 SCREEN STRUCTURE SHAKER DETAILS

2 PLFS0.1 STRUCTURAL SHEET INDEX

A.F.F.	ABOVE FINISH FLOOR	I.F.	INSIDE FACE
ALT	ALTERNATE	JST	JOIST
A.B.	ANCHOR BOLT	JT	JOINT
ARCH	ARCHITECTURAL PLANS	K	KIP (1000 LBS)
@	AT	LG	LONG
&	AND	LBS	POUNDS
BAL	BALANCE	LLH	LONG LEG HORIZONTAL
BLDG	BUILDING	LLV	LONG LEG VERTICAL
BM	BEAM	MANUF	MANUFACTURER
BOT	BOTTOM	MAS	MASONRY
BRG	BEARING	MAX	MAXIMUM
BTWN	BETWEEN	MIN	MINIMUM
CL	CENTER LINE	MISC	MISCELLANEOUS
C.G.S.	CENTER OF GRAVITY OF STRANDS	MK	MARK
CIP	CAST-IN-PLACE CONCRETE	N.S.	NEAR SIDE
CLR	CLEAR	N.T.S.	NOT TO SCALE
C.J.	CONTROL JOINT	O.C.	ON CENTER
COL	COLUMN	O.F.	OUTSIDE FACE
CMU	CONCRETE MASONRY UNIT	OPNG	OPENING
CONC	CONCRETE	OPP	OPPOSITE
CONT	CONTINUOUS	PC	PRECAST
CTR	CENTER	PSF	POUNDS PER SQUARE FOOT
DIA	DIAMETER	PSI	POUNDS PER SQUARE INCH
DEG	DEGREE	PT	POST TENSION
DIM	DIMENSION	RAD	RADIUS
DTL	DETAIL	REINF	REINFORCEMENT
DWG	DRAWING	REF	REFERENCE
E.F.	EACH FACE	RE:	REFERENCE
ELEV	ELEVATION	SCHED	SCHEDULE
EQ	EQUAL	SECT	SECTION
E.W.	EACH WAY	SHT	SHEET
EXIST	EXISTING	SIM	SIMILAR
EXP	EXPANSION	SPA	SPACING
EXT	EXTERIOR	SPECS	SPECIFICATION
FND	FOUNDATION	SQ	SQUARE
FIN	FINISHED	STD	STANDARD
FLR	FLOOR	STL	STEEL
F.S.	FAR SIDE	T&B	TOP & BOTTOM
FTG	FOOTING	T.O.	TOP OF...(ADD ITEM)
F.V.	FIELD VERIFY	TYP	TYPICAL
GA	GAUGE	U.N.O.	UNLESS NOTED OTHERWISE
GB.	GRADE BEAM	VAR	VARIES
GALV	GALVANIZED	VERT	VERTICAL
HORIZ	HORIZONTAL	w/	WITH
HT	HEIGHT	W.W.F.	WELDED WIRE FABRIC

3 PLFS0.1 STRUCTURAL ABBREVIATIONS

4 PLFS0.1 NOT USED

Order Plans @ WWW.LDI.inc.com

GEORGIA REGISTERED No. PE001789 PROFESSIONAL JEFFREY D. PARKER
PMA Engineering
6775 Shiloh Rd. E. Suite 100 Alpharetta, GA 30005 770.886.9999 www.pmaengineering.com
DATE: 5/4/2018

SHEET LEGEND

G0.0%	GENERAL INFORMATION & SCOPE OF WORK
S0.0%	SCHEDULES & NOTES
S0.1%	FOUNDATION & SLAB PLANS
A2.0%	EXTERIOR ELEVATIONS
A3.0%	ENLARGED PLANS
A3.0%	EXTERIOR ELEVATIONS
A3.0%	ENLARGED PLANS, ELEVATIONS, FINISH SCHEDULE, WINDOW PLANS
S3.0%	ROOF FRAMING PLANS
S3.0%	ROOF FRAMING SECTIONS
S3.1%	ROOF FRAMING PLAN
S4.1	STRUCTURAL
M1	MECHANICAL
E1	ELECTRICAL
P1	ELECTRICAL
AF1	AWNINGS DETAILS
FE1	OVERHEAD EQUIPMENT

AMC MADISON YARDS 8

AMC NEW BUILD
905 MEMORIAL DRIVE SE
ATLANTA, GA 30316

American Multi-Cinema, Inc.
One AMC Way
11500 Ash Street
Leawood, KS 66211

REV	DATE	DESCRIPTION

ORIGINAL ISSUE DATE: 05/21/2018

KEY PLAN:

SHEET TITLE:
GENERAL NOTES

SHEET NUMBER:
PLFS0.1