ABBREVIATIONS LEGEND

AB	ANCHOR BOLT	MCJ	MASONRY CONTROL JOINT
ACI	AMERICAN CONCRETE INSTITUTE	MECH	MECHANICAL
AFF	ABOVE FINISH FLOOR	MFR	MANUFACTURER
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	MIN	MINIMUM
AISI	AMERICAN IRON AND STEEL INSTITUTE	MISC	MISCELLANEOUS
ARCH	ARCHITECTURAL	MO	MASONRY OPENING
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	MTL	METAL
AWS	AMERICAN WELDING SOCIETY	NC	NOT IN CONTRACT
BFF	BELOW FINISH FLOOR	NO	NUMBER
BFS	BOTTOM OF FOOTING STEP	NS	NEAR SIDE
BL	BLOCK LINTEL	NTS	NOT TO SCALE
BO	BOTTOM OF	OC	ON CENTER
BOM	BOTTOM OF MASONRY	OH	OPPOSITE HAND
BOS	BOTTOM OF STEEL	PDF	POWDER DRIVEN FASTENERS
BRG	BEARING	PCF	POUNDS PER CUBIC FEET
CIP	CAST-IN-PLACE	P/C	PRECAST
C.J.	CONTROL JOINT (WALL)	PL	PLATE
CL	CENTER LINE	PLF	POUNDS PER LINEAR FOOT
CLR	CLEAR	PSI	POUNDS PER SQUARE INCH
CMU	CONCRETE MASONRY UNIT	QTY	QUANTITY
COL	COLUMN	RD	ROOF DRAIN
CONC	CONCRETE	REF	REFERENCE
CONS	CONSTRUCTION	REINF	REINFORCING
CONT	CONTINUOUS	REQD	REQUIRED
DIA	DIAMETER	REV	REVERSE
EIFS	EXTERIOR INSULATION AND FINISH SYSTEM	RO	ROUGH OPENING
EJ	EXPANSION JOINT	RTU	ROOF TOP UNIT
EL	ELEVATION	SDI	STEEL DECK INSTITUTE
ELEC	ELECTRICAL	SIM	SIMILAR
EQ	EQUAL	SJI	STEEL JOIST INSTITUTE
EW	EACH WAY	SPECS	SPECIFICATIONS
FDN	FOUNDATION	T/B	TOP AND BOTTOM
FF	FINISH FLOOR	T&C	TENSION AND COMPRESSION
FS	FOOTING STEP	TFS	TOP OF FOOTING STEP
FTG	FOOTING	THK	THICK
GA	GAUGE	TO	TOP OF
GC	GENERAL CONTRACTOR	TOC	TOP OF CONCRETE
GYP BD	GYPSUM BOARD	TOF	TOP OF FOOTING
HORIZ	HORIZONTAL	TOSB	TOP OF GRADE BEAM
HSA	HEADED STUD ANCHOR	TOJ	TOP OF JOIST
INFO	INFORMATION	TOM	TOP OF MASONRY
JST	JOIST	TOP	TOP OF PAVING
JT	JOINT	TOS	TOP OF STEEL
KSI	KIPS PER SQUARE INCH	TRANS	TRANSVERSE
LBS	POUNDS	TYP	TYPICAL
LLH	LONG LEG HORIZONTAL	UNO	UNLESS NOTED OTHERWISE
LLV	LONG LEG VERTICAL	VERT	VERTICAL
LONG	LONGITUDINAL	W	WIDTH
MAX	MAXIMUM	WP	WORK POINT
		WS	WALL STEP
		WWF	WELDED WIRE FABRIC

SYMBOLS LEGEND

ELEVATION DESCRIPTION — ELEVATION SYMBOL

CUT SYMBOL

SECTION CUT (DETL SHEET)

ELEVATION DETAIL (DETL SHEET)

GRID DESIGNATION

POINT LOAD SYMBOL

REVISION DESIGNATION

PLAN NOTE SYMBOL (22)

SPOT ELEVATION (100'-0")

ROOF COLUMN

MEZZANINE OR AUDITORIUM COLUMN

METAL STUD WALL/TRACK

METAL STUD BEARING WALL/TRACK

FULL CAPACITY MOMENT CONNECTION

RANGER CONNECTION OR CONCENTRATED LOAD

COLUMN SCHEDULE

MARK	SIZE	BASEPLATE L x N x THK	DETAIL
C1A	HSS 3 1/2" x 3 1/2" x 1/4"	9 1/2" x 9 1/2" x 3/4"	17-S512
C1B	HSS 3 1/2" x 3 1/2" x 5/16"	9 1/2" x 9 1/2" x 3/4"	17-S512
C1C	HSS 3 1/2" x 3 1/2" x 1/4"	4 1/4" x 10 1/4" x 1"	2-S512
C2A	HSS 3 1/2" x 3 1/2" x 1/4"	4 1/4" x 10 1/4" x 7/8"	2-S512
C2B	HSS 3 1/2" x 3 1/2" x 5/16"	4 1/4" x 10 1/4" x 7/8"	2-S512
C2C	HSS 3 1/2" x 3 1/2" x 3/8"	4 1/4" x 10 1/4" x 7/8"	2-S512
C2D	HSS 4" x 4" x 5/16"	10" x 10" x 3/4"	17-S512
C2E	HSS 4" x 4" x 3/8"	10" x 10" x 3/4"	17-S512
C2F	HSS 4" x 4" x 1/2"	10" x 10" x 3/4"	17-S512
C2G	HSS 4 1/2" x 4 1/2" x 5/16"	10 1/2" x 10 1/2" x 3/4"	17-S512
C2H	HSS 4 1/2" x 4 1/2" x 3/8"	10 1/2" x 10 1/2" x 3/4"	17-S512
C2I	HSS 4 1/2" x 4 1/2" x 1/2"	10 1/2" x 10 1/2" x 7/8"	17-S512
C2J	HSS 5" x 5" x 3/8"	11" x 11" x 7/8"	17-S512
C2K	HSS 5" x 5" x 1/2"	11" x 11" x 3/4"	17-S512

SPECIAL INSPECTIONS

SCOPE OF WORK	INSPECTION TASK	FREQUENCY OF INSP. CONTINUOUS	FREQUENCY OF INSP. PERIODIC	BUILDING CODE REFERENCE
CONCRETE	REINFORCING STEEL & PLACEMENT		X	ACI 318: 3.5, 7.1 - 7.5 & IBC 1910.4
	BOLTS INSTALLED IN CONCRETE		X	ACI 318: 8.1.3, 21.2.8 & IBC 1905.1.8, 1905.1.9 (ACI 355.2, HILTI)
	VERIFICATION OF REQ'D MIX		X	ACI 318: 1.2.2, 4.2, 5.2, 5.3, 5.4, 5.6, CH. 19, 26.4.3, 26.4.4 & IBC 1904.2, 1905.1.7
	CONCRETE SAMPLING	X		ASTM C172, C31 & ACI 318: 5.6, 5.8 & IBC 1910.10
	CONCRETE PLACEMENT	X		ACI 318: 5.9, 5.10 & IBC 1910.8, ACI 318: 5.11 - 5.13 & IBC 1910.9
	CURING TEMPERATURE & TECHNIQUES		X	ACI 318: 5.11 - 5.13 & IBC 1910.9
STEEL	MATL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS & WASHERS		X	IBC 1705.2.1
	HIGH-STRENGTH BOLTED CONNECTIONS		X	IBC 1705.2.1
	WELDING VERIFICATION OF STEEL		X	IBC 1705.2.1
	WELDING MATERIAL IDENTIFICATION		X	IBC 1705.2.1
	WELDING PARTIAL PENETRATION GROOVE WELDS	X		IBC 1705.2.1
	FILLET WELDING 5/16" & LARGER WELDS		X	IBC 1705.2.1
	STEEL FRAME		X	IBC 1705.2.1

ALL TYPE SPECIAL INSPECTIONS SHALL BE RETAINED BY THE OWNER TO COMPLETE SPECIAL INSPECTIONS PRIOR TO AND DURING CONSTRUCTION TO CONFORM TO THE REQUIREMENTS OF THE BUILDING CODE AND THE LOCAL CODE AUTHORITIES. ITEMS REQUIRING SPECIAL INSPECTIONS ARE TABULATED ABOVE. REFER TO CODE CHAPTER 17 FOR SPECIFIC REQUIREMENTS FOR EACH INSPECTION

COPIES OF PERIODIC AND FINAL SPECIAL INSPECTION REPORTS AND ANY ACCOMPANYING DATA SHALL BE SENT DIRECTLY FROM APPROVED TESTING AND INSPECTION AGENCY TO STRUCTURAL ENGINEER OF RECORD IMMEDIATELY AFTER PREPARATION - NO EXCEPTIONS

LT. GA. FRAMING FASTENERS & CONNECTORS

Notes: refer to sections and details for location and quantity of connectors

CONNECTOR	SUBSTRATE	DESCRIPTION	PRODUCT
SCREWS	METAL TRACK	#10-16 x 5/8" PAN HEAD	BUILDEX "TEKS" COMPASS (ESR-1976)
			"DARTS GRABBER SELF DRILLING" (ESR-1271)
	STUD-TO-STUD	#10-16 x 5/8" HEX HEAD	BUILDEX "TEKS" COMPASS (ESR-1976)
			"DARTS GRABBER SELF DRILLING" (ESR-1271)
	METAL-TO-STRUC. STEEL	#12-24 x 1-1/4" HEX HEAD, #5 TIP	BUILDEX "TEKS" (ESR-1976)
			"HILTI" KWIK-PRO (ESR-2116)
	STRUCTURAL PANEL CONCRETE SUBFLOOR TO L.T. GA. FRAMING	NO. 6-18 X 1 5/8" PATH MIN. 0.36" BUGLE HEAD	GRABBER "STRUCTURAL CONCRETE PANEL SCREW" ITEM # GHS6158LBSN62 (ESR-1271)
P.D.F.'s	CONCRETE OR GROUTED CMU	0.157"Ø X 1 1/4"	"HILTI" X-U (ESR-2269)
	STRUCTURAL STEEL	0.157"Ø X 5/8"	"HILTI" X-U (ESR-2269)

Fasteners by other manufacturers may be acceptable. Submit product data for review and approval prior to use

GENERAL STRUCTURAL NOTES

Design Loads:

1. BUILDING CODE — 2012 IBC W/GEORGIA AMENDMENTS

2. DEAD LOADS
   - MEZZANINE — 11 PSF
   - STADIUM SEATING — 12 PSF

3. LIVE LOADS
   - AUDITORIUM/FIXED SEATING — 60 PSF
   - LOBBY — 100 PSF
   - MEZZANINE — 50 PSF
   - STAIRS — 100 PSF

4. SNOW LOADS — N/A

5. WIND LOADS
   - A. WIND SPEED — N/A
   - B. RISK CATEGORY — N/A
   - C. EXPOSURE CLASSIFICATION — N/A
   - D. APPLICABLE INTERNAL PRESSURE COEFFICIENT — N/A
   - E. COMPONENTS AND CLADDING — N/A

6. EARTHQUAKE LOADS
   - A. IMPORTANCE FACTOR — 1.25
   - B. RISK CATEGORY — III
   - C. S_S — 0.184
   - D. S_1 — 0.060
   - E. SITE CLASS — D
   - F. S_{DS} — 0.196
   - G. S_{D1} — 0.143
   - H. SEISMIC DESIGN CATEGORY — C
   - I. BASIC SEISMIC FORCE RESISTING SYSTEM — LIGHT FRAMED WALL SYSTEMS USING FLAT STRAP BRACING
   - J. DESIGN BASE SHEAR — 0.061BW
   - K. C_S — 0.063
   - L. R — 4
   - M. ANALYSIS PROCEDURE — EQUIVALENT LATERAL FORCE METHOD

General:

1. These notes supplement the specification which should be referred to for additional information. Any conflict and/or contradiction of terms or requirements between plans, specifications and these General Notes will be resolved by the Architect/Engineer in favor of the more stringent or superior quality requirements.
2. The General Contractor shall be responsible for verifying all dimensions and elevations shown on the plans and for coordinating all dimensions and elevations shown on the structural drawings with those shown on the architectural and mechanical drawings. If errors or discrepancies in the dimensions occur, it shall be the superintendent's responsibility to bring all discrepancies to the attention of the Architect before proceeding with the work. Field verify conditions of all adjacent existing construction prior to beginning work or ordering materials. Report any discrepancies with information contained in these documents immediately to Engineer.
3. The Contractor shall verify all architectural, mechanical, and electrical openings, sizes and locations, with the structural drawings.
4. Structural details are general in nature and are valid for all similar areas and conditions, unless specifically shown or noted otherwise.
5. The Contractor shall provide all temporary bracing and shoring as required during construction to ensure the safety of all individuals involved.
6. Furnish all labor, materials and equipment necessary to complete the work shown or implied by these drawings.
7. A copy of all third party special inspection's testing and inspection reports shall be on-site for each related inspection performed by the building department or its designee.
8. Refer to specifications for additional requirements.

Concrete:

1. All concrete and reinforcing work shall conform to the latest edition of the American Concrete Institute's "Building Code Requirements for Structural Concrete" (ACI 318) and "Specifications for Structural Concrete for Buildings" (ACI 301).
2. Proposed concrete mix design shall be submitted to the Engineer for approval prior to construction. Concrete mix designs shall meet the following requirements:

Light Weight Concrete Normal Weight Concrete	28-day Strength	Maximum Water/Cement Ratio	Maximum Air Entrainment
	3000 psi	.50	6%
	4000 psi	.45	4"-1" 6%

3. Lightweight concrete fill or metal deck shall be "sand-lightweight" concrete and contain large aggregate that conforms to all material that conforms to ASTM C330. The concrete shall have a maximum 28 day, dry weight of 117 pcf.
4. The Contractor desiring to place concrete with a slump above allowable limits to facilitate placement or pumping shall be permitted to use an appropriate approved admixture - no water shall be added at the project site unless approved by the Engineer's permission. All admixtures shall be approved in writing by the Engineer.
5. The Contractor shall reject any concrete that exceeds the slump limits noted above or exceeds the maximum allowable mixing time noted in the specification.
6. Fly ash shall not be permitted in any concrete mix, with the exception of foundation concrete.
7. No aluminum shall be embedded in concrete.
8. All concrete is reinforced unless specifically noted as unreinforced. Reinforce all concrete not otherwise shown with the same reinforcing as in similar sections or areas.
9. During hot weather (80 degrees F and above), the Contractor shall comply with the recommendations of ACI 305 "Hot Weather Concrete". During cold weather (40 degrees and below), the Contractor shall comply with the recommendations of ACI 306 "Cold Weather Concreting".
10. The concrete mix designs are to be submitted as a formal submittal to the Engineer of Record for review and acceptance. After acceptance of the mix design by the Engineer of Record, the accepted designs must be forwarded to the Special Inspector PRIOR to concrete being delivered to the site.
11. Refer to Section 3 of the specifications for additional requirements.

Reinforcing Steel:

1. Materials shall comply with the following:
   a. Reinforcing steel, all bars, shall be ASTM A615, Grade 60.
   b. Welded wire mesh reinforcing shall be ASTM A185 cold drawn wire.
2. Detail bars in accordance with the "Manual of Standard Practice for Detailing Reinforced Structures" ACI 315, and "Building Code Requirements for Structural Concrete," ACI 318.
3. Accessories shall be as specified in the latest edition of "Concrete Reinforcing Steel Institute Handbook." Maximum spacing of accessories shall be 4'-0". All accessories to have galvanized or plastic coated feet.
4. Reinforcing shall be continuous and where splices are required they shall be lapped a minimum of 45 bar diameters for bars NOT considered top bars. Top bars bars with more than 12 inches of concrete below, shall be lapped 56 bar diameters at splices unless noted otherwise on the drawings. Welded wire fabric shall be lapped a minimum of six inches.
5. Standard concrete cover of bars unless otherwise noted shall be:
   a. Where earth formed — 3 inches
   b. Walls and slabs (not exposed to earth or weather) — 3/4 inch
   c. Walls and slabs (exposed to earth or weather):
      - #5 and smaller — 1 1/2 inches
      - #6 and larger — 2 inches
   d. Other — 2 inches
   All coverage nominal bar diameter minimum.
6. Provide 6x6/W2.1xW2.1 welded wire fabric for all concrete slabs-on-deck unless otherwise noted. Place wire in center of slab.
7. Where reinforcing bar or deformed anchors are required to be welded to other steel members ATOG "Weldable" grade reinforcing bars shall be used. A615 grade reinforcing shall not be welded. Other types of weldable deformed anchors may be substituted with written permission from the Engineer.
8. Refer to Section 3 of the specifications for additional requirements.

Structural Steel:

1. All structural steel shall conform to the following:
   a. Structural Steel — ASTM A36, Grade 36
   b. High Strength Structural Steel — ASTM A992, Grade 50
   c. Miscellaneous Steel — ASTM A36, Grade 36
   d. HSS Structural Tubing — ASTM A500, Grade B
   e. Steel Pipe — ASTM A53, Grade B

   All structural steel wide flange and WT shapes shall be ASTM A992, Grade 50 unless otherwise.
2. All beam connections shall be designed by the steel fabricator under the supervision of a Professional Engineer registered in the state of Georgia unless specifically noted otherwise on these drawings. Connections shall be welded or bolted per the latest edition of the AISC Steel Construction Manual. Bolts shall be ASTM A325, Type N unless specifically noted otherwise on drawings. If any connection load is not designated on the drawings, then it shall be designed for not less than 5/8 of the total uniformly distributed load capacity of the beam, noted as slip-critical, all bolts shall be tightened to a "snug tight" condition so that the fillers are firmly in contact, and shall be visually inspected by a testing agency.
3. For beam connections to tube columns, provide 3/8" thick shear plates with 4 inch column, welded all around both sides; alternately, connections using W section or double angles shall be welded to side of column.
4. Unless noted otherwise, all column anchor bolts shall have a minimum projection of 3d.
5. Unless noted otherwise, all structural steel shall be painted with one coat of red oxide primer conforming to specifications. Field touch-up all abraded areas and welded areas.
6. Welding shall conform to the latest publication of applicable codes set forth by the American Welding Society. Welding of unidentified materials shall not be allowed.
7. Composite metal floor deck for mezzanine slabs shall be 1 1/2" deep, Fy = 50 ksi, galvanized, attached to all supporting steel with 5/8" puddle welds at 12" o.c. spacing for spans to 10'-4", use 20 ga. deck; at 11'-0" and up to 12'-11" use 18 ga. deck.
8. Composite metal floor deck for seating platforms shall be 2" deep, Fy = 50 ksi, galvanized, attached to all steel supporting beams with 5/8" puddle welds at 12" o.c. For spans to 11'-6", use 20 ga. deck; if greater than 11'-6" and up to 11'-11" use 18 ga. See details for attachment to light gauge framing.
9. Shear connectors for composite floor systems shall be 3/4"Ø x 4" long headed studs, welded to beams to develop full shear capacity of the steel section of the stud. Unless noted otherwise studs shall be spaced evenly along the center of the beam. If the stud spacing exceeds 18" the metal deck shall be welded to the beams so that attachment points do not exceed 6". If the number of studs noted is such that the spacing exceeds 24" add additional studs so that spacing is equal to 24".
10. See architectural drawings for all miscellaneous steel, roller holes, attachments, etc.
11. Provide 2 layers 15# felt for bond breaker around steel columns where in contact with slab.
12. Guardrails, guards and handrails shall be designed by a Professional Engineer licensed in the State of Georgia, retained by the contractor. Sealed and signed shop drawings that have been reviewed and stamped by the Architect/Engineer shall be available at the job site during the times of inspection.
13. Grout below all column base plates and other bearing members shall be 7500 psi non-shrink, grout, thoroughly compacted and installed as recommended by the grout manufacturer.
14. All steel members shall be made in an approved fabricator's shop; the approved fabricator shall submit the Certificate of Compliance to the Engineer of Record and the Building Inspector prior to erection.
15. Steel deck used for sound isolation slab shall be 9/16", 22 ga., galvanized, type C steel deck by Vilcraft or approved equal. Steel deck shall be placed with ribs bearing on sound isolation pads spaced at 12"x12" grid pattern.
16. Refer to Section 5 of the specifications for additional requirements.

Anchors:

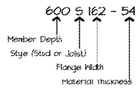
1. Expansion anchors shall be "Hilti" Kwik Bolt TZ (ESR-1917). Holes shall be drilled into concrete and not cored. If reinforcing is encountered during drilling the hole shall be moved to avoid damaging reinforcing. Unless noted otherwise anchor embedment shall fully develop the strength of the anchor and shall be a minimum of 2" less than the length of the anchor. Follow all manufacturer's recommendations including those for proper drill size, hole cleaning and anchor torque.
2. Unless noted otherwise, adhesive used to anchor rebar or threaded rods in concrete shall be "HILTI HIT-RE 500 V3 (ESR-3814). Holes shall be drilled into concrete and not cored. If reinforcing is encountered during drilling the hole shall be moved to avoid damaging reinforcing. Unless noted otherwise anchor embedment shall fully develop the strength of the anchor and shall be a minimum of 2" less than the length of the anchor. Follow all manufacturer recommendations including those for proper drill size and hole cleaning. When possible horizontal holes to receive threaded rods or rebar shall be slightly inclined to encourage flow of adhesive.
3. Powder driven fasteners for attachment of light gauge framing to concrete shall be "HILTI" X-U fasteners with .157" min. shank diameter (ESR-2269); and shall be embedded 1" minimum into concrete base.

Light Gauge Steel Connections:

1. All screw connections are based on AISI/SPEC section E4 which outlines the AISI specification provisions for screw connections.
2. For screws 1/2" minimum clearance must be maintained from all edges of the steel members. A 9/16" minimum on center spacing must be maintained between adjacent screws.
3. Powder drives fastener systems, expansion anchor systems, masonry screw systems, and adhesive anchor systems connections are based on literature published by Hilti Fastening Systems, Inc. Alternate manufacturer's fasteners of comparable specifications and load capacities are acceptable. Alternates are to be submitted to EOR for approval.
4. If required, all welded connections are to be performed in accordance with the latest version of AWS D1.3 specifications for Welding Sheet Steel in Structures. Consult AWS D1.3/D1.3M Welding Zinc Coated Steel and ANSI standard 7.4#1 for information regarding safe welding procedures.
5. Minimum weld throat thickness (t) must match or exceed the base steel thickness of the thinnest connected part unless noted otherwise.
6. In welding, the zinc coating on steel framing will be burned away therefore, a zinc rich paint must be applied to the weld area to provide corrosion resistance.
7. All steel members shall be made in an approved fabricator's shop; the approved fabricator shall submit the Certificate of Compliance to the Engineer of Record and the Building Inspector prior to erection.

Light Gauge Steel Framing:

1. Member references included herein are based on AISI standards. Products from any manufacturer that meet or exceed AISI geometrical and material standards are acceptable to be used for this project.



2. Metal studs shall meet the following criteria:
   a. ASTM A1003, Grade 33 Type H (ST 33H)
   b. ASTM A653, 33 Grade 33
   c. ASTM A1003, Grade 50 Type H (ST 50H)
   d. ASTM A653, 33 Grade 50 Class 1
3. Metal studs and joists shall have the following minimum sizes:
   a. Depth and Gauge — As noted on drawings
   b. Flange Width — 1 5/8"
   c. Lip Width — 1/2"
   d. Minimum Yield Strength 33 ksi up to 16 ga. members and 50 ksi for members heavier than 16 ga.
4. Sizes and thicknesses of all light gauge framing indicated on drawings are minimums required. No exceptions.
5. Tracks and Runners shall be same gauge as studs noted on drawing if heavier. Flange stiffeners shall be used on all tracks. Track shall be attached to concrete w/ powder driven fasteners x 12" o.c. U.N.O.
6. All structural light gauge framing shall be galvanized to meet the requirements of ASTM A525, G-60.
7. All light gauge steel framing shall be braced with bridging, bracing, track, and stiffeners, etc. as required to properly construct framing to manufacturer's recommendations. Contractor shall obtain a copy of the manufacturer's published construction details to use as a reference during construction.
8. Light gauge steel framing shown in auditorium sections is general configuration and minimum design requirements. Refer to note 7 below.
9. Light gauge steel framing and connections shall be designed by a Professional Engineer licensed in the state of Georgia, retained by the contractor. Sealed and signed shop drawings and calculations that have been reviewed and stamped by the Architect/Engineer shall be available at the job site during the times of inspection.
10. Connections of exterior and load bearing structural studs shall be made with 1/8 inch Pilot Welds all around unless noted otherwise.
11. Where gypsum sheathing is not required on both faces of steel studs, continuous bridging shall be installed as required by manufacturer.
12. All field cutting of studs must be done by sawing, shearing or plasma cutting. Other torch cutting methods of cold-formed members are unacceptable.
13. No notching or coping of studs is allowed unless stated herein.
14. Ends of studs must be seated firmly in and against web of stud track, which must have frill bearing on structure. Attach studs through track flanges w/ #10 self tapping screws each flange. A minimum of three exposed screw threads shall protrude beyond inner metal ply in installed position.
15. Splicing of stud studs is unacceptable unless specifically noted otherwise herein.
16. Framing fabricator must ensure punch out alignment when assembling lateral bracing and field cutting studs to length. Lateral bracing must be installed at the time wall studs are erected. Failure to install bracing at this time may compromise the integrity of the structure.
17. Design assumes all components to be in final, stabilized position and condition. Temporary bracing by others or other means of stabilization may be required until the framing is complete.
18. See a minimum of three studs at the intersection of all load bearing walls.
19. Provide light gauge steel lintel beam headers over trash archways as required, typical.
20. If additional holes are required in members contact a licensed Professional Engineer for written direction prior to cutting holes.
21. The design of the components and assemblies indicated herein are in accordance with the AISI "Specification for the Design of Cold Formed Steel Structural Members".
22. Light gauge structural components with thicknesses greater than those indicated herein may be substituted.
23. The framing contractor is advised to completely review the Contract Documents and compare to these drawings for conflicting and/or missing information prior to ordering material and/or beginning construction.
24. Refer to specifications for further requirements.

Shop Draning Review:

1. Norton & Schmidt Consulting Engineers, LLC. will review shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to properly function as an integral element of the overall structural system detailed on these drawings. This review will not relieve the contractor from the responsibility for errors, correctness of details, or conformance with the contract. The contractor shall allow 10 working days for review of all structural submittals.
2. Prior to submittal of a shop drawing or related submittal the general contractor shall:
   a. Review each submission for compliance with the contract documents.
   b. Review each submission for conformance with the means, methods, techniques, sequences, and operations of construction and safety precautions and programs incidental thereto, all of which are the sole responsibility of the general contractor.
   c. Identify any variation from the contract documents.
   d. Review and approve each compliant submission. Non-compliant submissions shall be returned to the subcontractor and approved submissions shall be stamped thus.
3. The following is a list of required shop drawings and related submittals. The general contractor shall refer to the specifications for more information and a complete list of required submittals:
   a. Anchor bolt layout drawings.
   b. Concrete mix designs, tests and material certifications.
   c. Concrete reinforcing shop drawings, reinforcing material certifications.
   d. Structural steel shop drawings, material certifications, welder certifications, connection calculations and AISC certification documentation.
   e. Light gauge steel framing, calculations and shop drawings including standard details, bracing and bridging requirements, and material certifications; welder certifications.
   f. Steel deck
   g. Post installed anchors

Special Inspections:

1. Special inspections for light gauge steel framing construction shall be completed in strict accordance with Code Section 1705.12.3 (omit first sentence and Exception). Special inspection reporting shall be in strict accordance with Code Section 1704.2.4.
2. The contractor shall be responsible for notifying the Special Inspector a minimum of 72 hours in advance of required Special Inspections. Installation of construction materials that prevent accessibility to inspection areas must not proceed until Special Inspections have been completed and finalized.
3. Special Inspection reports shall be issued to the Owner, Contractor, Architect and Structural Engineer of Record immediately after completion. No exceptions.

Design Responsibility

I, Rodney M. Sommer, P.E., Registered Engineer and a principal of Norton & Schmidt Consulting Engineers, LLC., certify that the structural design drawings consisting of sheets S001, S101, S200, S201, S202, S203, S204, S501, S502, S503, S504, S505, S510, S511, S512 and S513 were prepared by or approved by me and that I am a duly licensed Engineer under the laws of the State of Georgia, license number PE021645, Exp. date 12/31/18. I disclaim any responsibility for all other drawings in the construction document package as they are the responsibility of other design professionals.

SHEET INDEX

S001 . . . . . GENERAL STRUCTURAL INFORMATION
S101 . . . . . ENTRY LEVEL FLOOR PLAN
S200 . . . . . SEISMIC LOAD PLAN
S201 . . . . . OVERALL PROJECTION LEVEL FRAMING PLAN
S202 . . . . . ENLARGED PROJECTION LEVEL FRAMING PLAN - AUD. 1
S203 . . . ENLARGED PROJECTION LEVEL FRAMING PLAN - AUD. 2, 3 & 4
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SHEET TITLE: GENERAL STRUCTURAL INFORMATION

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