SECTION 5A – STRUCTURAL STEEL (NON PEMB)

A. STRUCTURAL STEEL DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE "MANUAL OF STEEL CONSTRUCTION" OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION AND SHALL CONFORM TO THE LATEST OSHA REQUIREMENTS. SHOP DRAWINGS SHALL GIVE COMPLETE WELDING INFORMATION, BOTH SHOP AND FIELD, USING AWS SYMBOLS.

B. MATERIALS REQUIREMENTS, U.N.O.:
1. WELDING ELECTRODES SHALL CONFORM TO AWS A5.1 OR A5.5 E–70XX. (LOW – HYDROGEN FOR SMAW WELDING) ALL WELDING PROCEDURES SHALL BE LOW–HYDROGEN PROCESSES. ELECTRODES SHALL BE STORED AFTER OPENING TO MAINTAIN HYDROGEN CONTENTS.
2. BOLTS ARE TO BE 3/4" DIAMETER HIGH STRENGTH BOLTS CONFORMING TO ASTM A–325, U.N.O.
3. STRUCTURAL STEEL W–SHAPES SHALL BE ASTM A–992 GRADE 50. MISCELLANEOUS SHAPES (CHANNELS AND ANGLES) MAY CONFORM TO ASTM A–36 IN LIEU OF ASTM A–992 GRADE 50. TUBE STEEL MEMBERS SHALL CONFORM TO ASTM A–500B. PIPES TO BE ASTM A–53.
4. COMPRESSIBLE–WASHER–TYPE DIRECT TENSION INDICATOR DEVICES (DTI'S) SHALL CONFORM TO ASTM F959, AND SHALL BE BY J&M TURNER, INC., OR APPROVED EQUIVALENT. TWIST–OFF– TYPE TENSION CONTROL BOLTS (TCB'S) SHALL CONFORM TO ASTM F1852.
5. ANCHOR RODS SHALL CONFORM TO ASTM A36.
6. GROUT BELOW BASE PLATES SHALL BE NONSHRINK, HIGH STRENGTH, NONMETALLIC GROUT, WITH A MINIMUM (28) DAY COMPRESSIVE STRENGTH OF 6000 PSI.
7. SHEAR STUDS SHALL CONFORM TO ASTM A108 FOR LOW CARBON STEEL WITH A MINIMUM YIELD STRENGTH OF 50,000 PSI.

C. IN GENERAL, CONNECTIONS SHALL BE FIELD BOLTED. ALL BOLTS DESIGNATED "SLIP CRITICAL" OR "FULLY TIGHTENED" SHALL BE TIGHTENED TO THE MINIMUM PRETENSION VALUE SHOWN IN TABLE J3.1 OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS. IN ADDITION, CONNECTIONS DESIGNATED "SLIP CRITICAL" SHALL HAVE PROPERLY PREPARED FAYING SURFACES TO MEET CLASS A SURFACE CONDITION, U.N.O. "SLIP CRITICAL" CONNECTIONS SHALL INCLUDE ALL BOLTS IN MOMENT CONNECTIONS. "FULLY TIGHTENED" CONNECTIONS SHALL INCLUDE ALL BOLTS LOADED IN DIRECT TENSION (SUCH AS HANGERS), BRACED FRAME CONNECTIONS, AND MEMBERS THAT ARE PART OF THE MAIN LATERAL RESISTING SYSTEM. DIRECT TENSION INDICATOR (DTI) WASHERS OR TENSION CONTROL BOLTS (TCB'S) SHALL BE USED AT THESE CONDITIONS. ALL OTHER BOLTS SHALL BE, AT MINIMUM, SNUG TIGHT. WELDED CONNECTIONS SHALL BE MADE WITH E70 ELECTRODES, UNLESS OTHERWISE RECOMMENDED BY AWS.

D. STEEL QUALITY CONTROL:
1. WELDER QUALIFICATIONS: QUALIFY WELDING PROCESSES AND WELDING OPERATORS IN ACCORDANCE WITH AWS STANDARD QUALIFICATION PROCEDURE". OPERATORS SHALL CARRY PROOF OF QUALIFICATIONS ON THEIR PERSONS.
2. TEST REPORTS: COPIES OF STEEL PRODUCER'S REPORT OF MILL ANALYSIS AND TENSILE AND BEND TESTS FOR STRUCTURAL STEEL MADE NO MORE THAN (60) DAYS BEFORE SHIPMENT.
3. CERTIFICATES: TESTING LABORATORY'S CERTIFICATE THAT STRUCTURAL STEEL HAS BEEN FURNISHED AND INSTALLED IN ACCORDANCE WITH CONTRACT DOCUMENTS. TESTING LABORATORY SHALL INSPECT CONNECTIONS IN ACCORDANCE WITH REFERENCES AS FOLLOWS. COPIES OF TEST RESULTS AND INSPECTION REPORTS SHALL BE SENT DIRECTLY TO THE ENGINEER.
   a. WELDED CONNECTIONS SHALL CONFORM TO AWS D1.1. TESTING AGENCY SHALL INSPECT ALL COMPLETE PENETRATION WELDS AND ALL BUTT WELDS MADE BY FABRICATOR. PERFORM ULTRASONIC OR RADIOGRAPHIC INSPECTIONS OF ALL FULL PENETRATION WELDS MADE IN THE FIELD. IF THE FABRICATOR USES THE FULL VALUE FOR FILLET WELDS, AS SPECIFIED IN THE REFERENCES, INSPECT 15% OF THESE WELDS. VISUALLY INSPECT ALL (100%) FIELD WELDS.
   b. BOLTED CONNECTORS: INSPECT AT LEAST 10% OF ALL "SLIP CRITICAL" OR "FULLY TIGHTENED" HIGH STRENGTH BOLTS WHICH ARE WELL SCATTERED THROUGHOUT THE STRUCTURE. IF LESS THAN 95% OF THE TESTED BOLTS MEET DESIGN TENSION OR IF ANY BOLT IS LESS THAN 85% OF DESIGN TENSION, THEN ALL BOLTS SHALL BE REWORKED. INSPECT 50% OF ALL REWORKED BOLTS, REPEAT THIS PROCESS UNTIL THE ABOVE REQUIREMENTS ARE MET. DIRECT TENSION INDICATORS OR TENSION CONTROL BOLTS MAY BE USED TO TEST 100% OF ALL "SLIP CRITICAL" OR "FULLY TIGHTENED" HIGH STRENGTH BOLTS.

E. A PRE–STEEL ERECTION CONFERENCE SHALL BE HELD BY THE CONTRACTOR WITH SUBCONTRACTORS, AND TESTING LAB PERSONNEL. ARCHITECT AS WELL AS ENGINEER SHALL BE INVITED BUT SHALL NOT BE REQUIRED TO ATTEND. CONFERENCE SHALL BE HELD WELL IN ADVANCE OF CONSTRUCTION TO INSURE PROPER INTERPRETATION OF DESIGN INTENT. SUBMIT QUESTIONS RESULTING FROM CONFERENCE IN WRITING TO ENGINEER. STEEL ERECTOR SHALL FIELD VERIFY CORRECTNESS OF FOUNDATION, ANCHOR BOLTS, OR OTHER EXISTING WORK AFFECTING THE STEEL BEFORE STARTING ERECTION.

F. SUBMIT SHOP DRAWINGS FOR FABRICATION AND ERECTION OF ALL STEEL MEMBERS IN ACCORDANCE WITH AISC STANDARDS NOTED ABOVE.

G. FABRICATOR SHALL DESIGN ALL CONNECTIONS NOT SPECIFICALLY DETAILED ON DRAWINGS. REGARDLESS OF PROVISION TO THE CONTRARY IN THE AISC CODE OF STANDARD PRACTICE FOR BUILDINGS AND BRIDGES, ALL CONNECTIONS DESIGNED BY FABRICATOR SHALL BE HIS RESPONSIBILITY AND REVIEW OF SHOP DRAWINGS BY THE ENGINEER SHALL NOT RELIEVE FABRICATOR OF THIS RESPONSIBILITY.

H. UNLESS OTHERWISE NOTED, ALL BEAM CONNECTIONS SHALL BE STANDARD FRAMED, SEATED END, OR SINGLE–PLATE SHEAR CONNECTIONS AS SHOWN IN THE AISC MANUAL OF STEEL CONSTRUCTION. UNLESS REACTIONS ARE NOTED ON THE DRAWINGS, CONNECTIONS SHALL DEVELOP AT LEAST ONE–HALF OF THE TOTAL UNIFORM LOAD CAPACITY OF THE BEAM. CONNECTIONS SHALL BE DESIGNED AS BEARING–TYPE CONNECTIONS WITH THREADS IN THE SHEAR PLANE, UNLESS OTHERWISE NOTED. IN NO CASE SHALL THE LENGTH OF FRAMED CONNECTIONS BE LESS THAN ONE–HALF THE "T" DISTANCE OF THE BEAM WEB.

I. ALL BRACING CONNECTIONS SHALL DEVELOP EITHER THE FORCE NOTED ON THE DRAWINGS OR THE ALLOWABLE TENSILE FORCE IN THE MEMBER IF NOT NOTED ON THE DRAWINGS. BRACING CONNECTIONS SHALL BE DESIGNED AND DETAILED SO THAT ALL FORCE COMPONENTS WILL BE TRANSMITTED DIRECTLY TO THE CENTER OF GRAVITY OF INTERSECTING MEMBERS. WHERE THIS IS NOT POSSIBLE, CONNECTIONS SHALL BE DESIGNED FOR ALL RESULTING ECCENTRICITIES. BOLTED BRACING CONNECTIONS SHALL BE CONNECTED WITH A MINIMUM OF TWO BOLTS. DESIGN OF GUSSET PLATES AT BRACING CONNECTIONS IS THE RESPONSIBILITY OF THE FABRICATOR. MINIMUM GUSSET PLATE THICKNESS SHALL BE ½", U.N.O.

SECTION 5A – (CONTINUED)

J. MINIMUM WELD SIZE SHALL BE 3/16" UNLESS OTHERWISE NOTED. WHERE NOT NOTED OTHERWISE, WELD SHALL BE ALL AROUND. INCREASE WELD SIZE TO MEET AISC REQUIREMENTS.

K. SINGLE SHEAR PLATES SHALL BE 3/8" MINIMUM THICKNESS.

L. THE GENERAL CONTRACTOR SHALL VERIFY THE REQUIRED CAMBER IN THE FIELD PRIOR TO ERECTION OF EACH MEMBER. ANY MILL CAMBER SHALL BE PLACED UP.

M. SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ENGINEER AS TO LOCATION AND TYPE OF SPLICE TO BE MADE. ANY MEMBER HAVING A SPLICE NOT SHOWN AND DETAILED ON SHOP DRAWINGS WILL BE REJECTED.

N. STRUCTURAL STEEL SHALL BE PUNCHED FOR WOOD BLOCKING AND NAILERS IN ACCORDANCE WITH ARCHITECTURAL DETAILS.

O. THIS STRUCTURE DEPENDS ON THE DIAPHRAGM AND BRACING MEMBERS SHOWN. THE CONTRACTOR IS TO PROVIDE LATERAL BRACING IN EACH DIRECTION DURING THE ERECTION PHASE. SUCH BRACING SHALL REMAIN IN PLACE UNTIL ALL DIAPHRAGM AND LATERAL BRACING ELEMENTS ARE IN PLACE IN THEIR ENTIRETY AND HAVE BEEN APPROVED BY THE STRUCTURAL ENGINEER.

P. ALL STRUCTURAL STEEL, EXCEPT FOR GALVANIZED STEEL, STEEL TO RECEIVE SPRAY ON FIREPROOFING, AND THAT IN CONTACT WITH FRESH CONCRETE, SHALL RECEIVE ONE SHOP COAT OF THE FABRICATOR'S STANDARD GRAY PRIMER. ALL BOLTED AND WELDED CONNECTIONS EXCEPT GALVANIZED CONNECTIONS AND THOSE IN CONTACT WITH FRESH CONCRETE SHALL BE PAINTED WITH THE SAME GRAY PRIMER (OR SPECIAL PRIMER IN PROCESS AREAS) FOLLOWING APPROVAL OF THE CONNECTION BY THE TESTING AGENCY. TOUCH UP GALVANIZED CONNECTIONS WITH A ZINC RICH GALVANIZING PAINT. STEEL SHALL BE BARE AT AREAS TO BE FIREPROOFED. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR FINISH PAINT.

Q. NOTHING SHALL BE SUSPENDED FROM ROOF DECK.

R. DETAILING, FABRICATION, AND ERECTION SHALL COMPLY WITH ALL APPLICABLE OSHA REGULATIONS, INCLUDING ADDITIONAL CONNECTORS, PLATES, HOLES, ETC. NOT DEPICTED ON THESE DRAWINGS.

SECTION 5B – PRE–ENGINEERED METAL BUILDING (PEMB)

A. PRE–ENGINEERED METAL BUILDING SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF GEORGIA. COLUMN REACTIONS SHALL BE SUBMITTED WITH ANCHOR BOLT DRAWINGS FOR REVIEW PRIOR TO CONSTRUCTION OF FOUNDATION. IF REACTIONS EXCEED THOSE USED FOR FOUNDATION DESIGN, FOUNDATIONS MUST BE REVISED AS NECESSARY. DESIGN LOADS SHALL BE COMPUTED IN ACCORDANCE WITH SECTION 1 OF THESE NOTES.

B. FURNISH ALL LABOR, MATERIALS AND ENGINEERING SERVICES REQUIRED TO COMPLETE THE METAL BUILDING, ROOF DECK, AND WALL PANELS, INCLUDING ANCHOR BOLTS, COLUMNS, BEAMS, GIRTS, BRACING, MOUNTING ACCESSORIES, ROOF INSULATION, METAL TRIM, FASCIA, GUTTERS, INSULATION, AND OTHER COMPONENTS REQUIRED FOR A COMPLETE JOB.

C. ANCHOR BOLTS, AND ANCHOR BOLT SETTING PLAN AND ANCHOR BOLT TEMPLATES SHALL BE PROVIDED BY THE METAL BUILDING SYSTEM SUPPLIER.

D. THE STAMP OF A REGISTERED ENGINEER IS REQUIRED ON ALL ERECTION DRAWINGS AND DESIGN CALCULATIONS.

E. STRUCTURAL SYSTEMS SHALL BE DESIGNED TO CONFORM TO THE ENGINEERING STANDARDS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION AND THE AMERICAN IRON AND STEEL INSTITUTE AND THE REQUIREMENTS OF THESE DOCUMENTS.

F. CONTRACTOR SHALL PROVIDE ERECTION INFORMATION AND DRAWINGS AS REQUIRED TO DESCRIBE AND DEFINE SYSTEM. DRAWINGS SHALL INCLUDE ANCHOR BOLT SETTING PLAN AND PIECE MARKS ON ALL MAJOR PARTS FOR EASY FIELD INDENTIFICATION.

G. SUBMIT LETTER OF DESIGN CERTIFICATION FOR THE STRUCTURAL FRAMING AND LIGHT GAUGE METAL FRAMING OF THE METAL BUILDING SYSTEM. LETTER OF CERTIFICATION TO BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER.

H. PRIMARY MEMBERS FABRICATED FROM PLATE, PLATE COILS, STRIP MILL PLATE OR FLAT BAR STOCK SHALL HAVE FLANGES AND WEBS JOINED ON ONE SIDE OF THE WEB BY A CONTINUOUS WELDING PROCESS. MINIMUM YIELD STRENGTH : 50,000 PSI UNLESS OTHERWISE APPROVED.

I. SECONDARY MEMBERS, PURLINS, GIRTS, EAVE STRUTS SHALL BE COLD FORMED FROM STEEL WHICH HAS A MINIMUM YIELD STRENGTH OF 55,000 PSI, UNLESS OTHERWISE APPROVED.

J. TRANSVERSE WIND/SEISMIC FORCES SHALL BE TRANSFERRED TO THE FOUNDATION THROUGH THE USE OF PORTAL FRAMES IN COMBINATION WITH "X" BRACING IN THE PLANE OF THE ROOF. LONGITUDINAL WIND/SEISMIC FORCES SHALL BE TRANSFERRED TO THE FOUNDATION THROUGH THE USE OF PORTAL FRAMES IN COMBINATION WITH "X" BRACING IN THE PLANE OF THE ROOF. SUBMIT ERECTION DRAWINGS WITH BRACING AND FRAME LOCATIONS INDICATED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION OF METAL BUILDING.

K. FOOTINGS HAVE BEEN SIZED BASED ON ESTIMATED COLUMN REACTIONS. THE ENGINGINEERED METAL BUILDING SUPPLIER SHALL PROVIDE FINAL COLUMN REACTIONS FOR REVIEW. IF COLUMN REACTIONS ARE DIFFERENT FROM THOSE ORIGINALLY ESTIMATED, FOOTINGS WILL BE REQUIRED TO BE REDESIGNED. CONTRACTOR IS TO INSURE THAT THIS REVIEW PROCESS IS COMPLETE PRIOR TO PLACING FOOTINGS.

L. PEMB FRAMING SHOWN ON THESE DRAWINGS IS CONCEPTUAL. FINAL DESIGN IS THE RESPONSIBILITY OF THE METAL BUILDING DESIGN ENGINEER. COMPLETE DRAWINGS AND CALCULATIONS FOR THE METAL BUILDING SYSTEM SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION.

M. DEFLECTION CRITERIA:
1. BASE BID:
   a. BUILDING FRAMES H/180
   b. GIRTS OR SPANDRELS L/180
2. ADD ALTERNATE No. 1:
   a. BUILDING FRAMES LATERALLY SUPPORTING MASONRY H/360
   b. BUILDING FRAMES AT METAL SIDING H/180
   c. GIRTS OR SPANDRELS LATERALLY SUPPORTING MASONRY L/500
   d. GIRTS OR SPANDRELS LATERALLY SUPPORTING METAL SIDING L/180
3. ADD ALTERNATE No. 2:
   a. BUILDING FRAMES H/300
   b. GIRTS OR SPANDRELS LATERALLY SUPPORTING MASONRY L/500
   c. GIRTS OR SPANDRELS LATERALLY SUPPORTING METAL SIDING L/180

SECTION 5C LIGHT GAUGE STEEL

A. SECTIONS AND DETAILS SHOWN ON THE DRAWINGS ARE FOR CONCEPT ONLY. ACTUAL MEMBER SIZE, SPACING, GAGE, AND CONNECTION DETAILS SHALL BE DESIGNED BY METAL STUD ENGINEER (COMPONENT ENGINEER). METAL STUDS SHALL BE DESIGNED PER "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STRUCTURAL STEEL MEMBERS" FOR ALL APPLICABLE LOADS.

B. DESIGN OF LIGHT GAGE METAL FRAMING NOT SPECIFICALLY DETAILED ON DRAWINGS SHALL BE PERFORMED BY A LICENSED STRUCTURAL ENGINEER IN THE STATE IN WHICH THE PROJECT WILL BE CONSTRUCTED. DESIGN CALCULATIONS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL PRIOR TO FABRICATION. DESIGN CALCULATIONS SHALL BE SIGNED AND SEALED BY THE DESIGN ENGINEER.

C. COMPLETE SHOP DRAWINGS FOR THE CONSTRUCTION OF LIGHT GAUGE METAL FRAMING SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF GEORGIA AND SHALL BE AVAILABLE AT THE JOB SITE AT TIMES OF INSPECTION. SHOP DRAWINGS SHALL BE SUBMITTED PRIOR TO FABRICATION SHOWING WALL SECTIONS COORDINATED WITH DRAWINGS SHOWING FRAMING, ACCESSORIES, ANCHORAGE AND CONNECTION DETAILS.

D. MATERIAL SPECIFICATIONS FOR LIGHT–GAGE STEEL:
16 GA. OR HEAVIER: ASTM A–446, Fy = 50 KSI MIN.
18 GA. OR LIGHTER: ASTM A–446, Fy = 33 KSI MIN.
GALVANIZING: MINIMUM G–60 COATING

E. CONNECTION MATERIAL GAGE
MATCH STUD GAGE U.N.O. CLIP ANGLES SHALL BE 14 GA. MINIMUM.

F. BUILT–UP MEMBERS
FASTEN TOGETHER WITH 1" LONG STITCH WELDS OR #12 SCREWS AT 12" o.c. MAXIMUM, EACH FLANGE, AND EACH TRACK.

G. PROVIDE BRIDGING AT 4' MAXIMUM VERTICAL SPACING IN WALLS.

H. STUDS SHALL BE INSTALLED TO SEAT SQUARELY (WITHIN 1/16") AGAINST THE WEB PORTION OF THE TOP AND BOTTOM TRACKS. TRACKS SHALL REST ON A CONTINUOUS, UNIFORM BEARING SURFACE.

J. TEMPORARY BRACING SHALL BE PROVIDED AND LEFT IN PLACE UNTIL WORK IS PERMANENTLY STABILIZED.

K. SPLICING OF MEMBERS SPANNING BETWEEN SUPPORTS SHALL NOT BE PERMITTED.

L. VERTICAL ALIGNMENT (PLUMBNESS) OF STUDS SHALL BE WITHIN 1/960TH (1/8" IN 10'–0") OF THE SPAN.

M. HORIZONTAL ALIGNMENT (LEVELNESS) OF WALLS SHALL BE WITHIN 1/960TH (1/8" IN 10'–0") OF THEIR RESPECTIVE LENGTHS. SPACING OF STUDS SHALL NOT BE MORE THAN + 1/8" FROM THE DESIGNED SPACING PROVIDING THAT THE CUMULATIVE ERROR DOES NOT EXCEED THE REQUIREMENTS OF THE FINISHED MATERIALS.

N. PROVIDE DEEP TRACK ASSEMBLY AT TOPS OF ALL NON–LOAD BEARING STUD WALLS TO ALLOW FOR MOVEMENT OF STRUCTURE. ARCHITECT SHALL REVIEW IN PLACE METAL STUD CONSTRUCTION PRIOR TO THE INSTALLATION OF GYPSUM WALL BOARD OR SHEATHING.

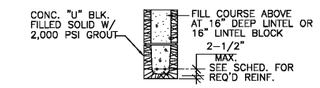
O. DEFLECTION OF LIGHT GAUGE STEEL WALL STUDS AND OTHER MEMBERS LATERALLY SUPPORTING MASONRY SHALL BE LIMITED TO L/600 ALL OTHER LIGHT GAUGE WALL STUDS AND FRAMING SHALL BE LIMITED TO L/360.

P. COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN TEAM – EOR RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA AND SHALL BE AVAILABLE AT THE JOB SITE DURING THE TIMES OF INSPECTION INCLUDING THE FOLLOWING COMPONENTS OF THIS PROJECT:
   LIGHT GAUGE STEEL FRAMING
   PRE–ENGINEERED METAL BUILDING SYSTEM.

SECTION 6 – WOOD

A. GENERAL CONSTRUCTION NOTES:
1. ALL WOOD JOISTS AND BEAMS TO BE #2 SOUTHERN PINE, U.N.O. ALL 2x4 AND 2x6 LOAD BEARING AND EXTERIOR WALL WOOD STUDS ARE TO BE #2 S–P–F. OR SOUTHERN PINE STUD GRADE, OR BETTER.
2. ALL MULTIPLE STUD POSTS, ISOLATED OR WITHIN WALLS, SHALL BE #2 S–P–F. OR SOUTHERN PINE STUD GRADE, OR BETTER.
3. S–P–F (SOUTH) SHALL NOT BE SUBSTITUTED FOR S–P–F.
4. PROVIDE 1/2" DIAMETER x 6" EMBEDMENT HEADED ANCHOR BOLTS TO ALL CONTINUOUS PLATES AT LOAD BEARING AND EXTERIOR WALLS, AT CORNERS, AT EACH SIDE OF EACH OPENING AND AT 48" O.C., UNLESS NOTED AS CLOSER ON DRAWINGS.
5. NAIL MULTIPLE PLY BEAMS AND HEADERS WITH TWO ROWS 16d NAILS AT 12" O.C. TOP AND BOTTOM PER PAIR OF PLYS, U.N.O. SPLICES ARE NOT PERMITTED IN ANY PLY BETWEEN SUPPORTS. SEE DRAWINGS FOR BOLTED MULTIPLE PLY BEAMS AND HEADERS.
6. MULTIPLE STUD POSTS WITH (4) OR MORE STUDS SHALL BE NAILED TOGETHER WITH EACH STUD NAILED TO THE ADJACENT STUD W/(2) ROWS 16d NAILS AT 12"o.c. STAGGERED AT 6"o.c.
7. STUDS OR JOISTS SHALL NOT BE CUT TO INSTALL PLUMBING OR WIRING UNLESS METAL OR WOOD SIDE PIECES ARE PROVIDED TO STRENGTHEN THE MEMBER.
8. ANY WOOD THAT IS TO REMAIN EXPOSED TO WEATHER, OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE–TREATED #2 SOUTHERN PINE.
9. ALL LOAD BEARING WALL STUD SIZES SHALL BE IN ACCORDANCE WITH THE DRAWINGS.
10. TIMBER FASTENING SHALL BE PER IBC "MINIMUM FASTENING SCHEDULE" UNLESS NOTED AS GREATER ON DRAWINGS. CONNECTORS TO BE SIMPSON AS NOTED ON DRAWINGS. WHERE NO HANGER SIZE IS SPECIFIED, PROVIDE HANGER RECOMMENDED BY MANUFACTURER FOR JOIST SIZE SUPPORTED, AS A MINIMUM.
11. USE TYPE, SIZE AND QUANTITY OF FASTENERS IN CONNECTORS SPECIFIED BY CONNECTOR MANUFACTURER. WHERE FASTENER OPTIONS ARE GIVEN BY THE MANUFACTURER, INSTALL TYPE, SIZE AND QUANTITY OF FASTENERS REQUIRED TO ACHIEVE THE MAXIMUM RATED CONNECTOR CAPACITY.
12. BOLTS: BOLTS FOR WOOD CONSTRUCTION SHALL BE ASTM A–307. BOLT HOLES IN WOOD SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER. A METAL PLATE, METAL STRAP, OR WASHER NOT LESS THAN A STANDARD CUT WASHER SHALL BE BETWEEN THE WOOD AND THE BOLT HEAD AND BETWEEN THE WOOD AND THE NUT. THE THREADED PORTION OF BOLTS SUBJECT TO WOOD BEARING SHALL BE KEPT TO A PRACTICAL MINIMUM.

CMU LINTEL REINF. SCHEDULE		
OPENING WIDTH	REINF.	REMARKS
UP TO 3'–0"	2#3	
UP TO 4'–8"	2#4	
UP TO 6'–0"	2#5	
UP TO 8'–0"	2#6	
UP TO 12'–0"	2#5	16" DEEP
UP TO 14'–0"	2#8	16" DEEP



PROVIDE 16" MIN. BEARING EACH END.
VERTICAL REINFORCING TO EXTEND THROUGH LINTEL.

CMU LINTEL REINFORCING SCHEDULE (1 / S0–2)

NOT TO SCALE

STEEL BRICK LINTEL SCHEDULE:		
OPENING WIDTH	ANGLE SIZE	REMARKS
UP TO 4'–0"	∠ 3–1/2 X 3–1/2 X 5/16	
UP TO 6'–0"	∠ 4 X 3–1/2 X 5/16	LLV
UP TO 8'–0"	∠ 5 X 3–1/2 X 5/16	LLV
NOTE: PROVIDE 8" MINIMUM BEARING E.E. LINTEL.		

SPECIFIC DETAILS TAKE PRECEDENCE OVER SCHEDULED SIZES.

STEEL ANGLE VENEER LINTEL SCHEDULE (2 / S0–2)

NOT TO SCALE

ARCHITECT'S STAMP

STATE OF GEORGIA · GORDON N. SMITH, JR. · 7575 · LAGRANGE, GEORGIA · REGISTERED ARCHITECT

SIGNATURE REQUIRED

REGISTERED PROFESSIONAL ENGINEER · 17028 · 5/9/18 · THOMAS BRADY

SMITH DESIGN GROUP, INC.
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REVISIONS		
△	DATE	DESCRIPTION

PROJECT:

VERNON ROAD FIRE STATION

VERNON ROAD
LAGRANGE GEORGIA

TITLE:

GENERAL NOTES (CONTINUED)

MODIFIED DATE:

ISSUED DATE:
FOR BID AND PERMIT
7 MAY 2018

JOB NO:
1731

SHEET:
S0–2

SBFA JOB # 18034.01
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