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CONCRETE PILE NOTES

- PRE STRESSED CONCRETE PILING SHALL BE MANUFACTURED IN ACCORDANCE WITH ASTM D 1143 (PILES UNDER AXIAL COMPRESSIVE LOAD), PCI MNL-116 (MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRE CAST AND PRE STRESSED CONCRETE PRODUCTS), PCI STD-112 (STANDARD PRE STRESSED CONCRETE PILES, SQUARE, OCTAGONAL AND CYLINDER), AND PCI MNL-120 (PCI DESIGN HANDBOOK-PRECAST AND PRE STRESSED CONCRETE).
- PRE STRESSING STEEL SHALL BE SEVEN-WIRE STRESS RELIEVED STRAND CONFORMING TO ASTM A 416 OR STRESS-RELIEVED WIRE CONFORMING TO ASTM A421, TYPE WA. THE MINIMUM ULTIMATE STRENGTH SHALL BE 250,000 PSI. PRE STRESSING STEEL SHALL BE FREE FROM GREASE, OIL, WAX, PAINT, SOIL, DIRT, LOOSE RUST, KINKS, BENDS OR OTHER DEFECTS.
- NON-PRESTRESSING REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60.
- STEEL FOR TIES AND SPIRALS SHALL CONFORM TO ASTM A 82.
- FOR PILES EXPOSED TO SEA OR BRACKISH WATER, THE CONCRETE MIX DESIGN AND THE CONCRETE MATERIALS SELECTED SHALL ENSURE PRODUCTION OF EXTREMELY DENSE CONCRETE FREE OF SHRINKAGE CRACKS AND HONEYCOMB WITH A MINIMUM OF PERMEABILITY. THE MAXIMUM WATER CEMENT RATIO (BY WEIGHT) SHALL BE 0.40. THE CONCRETE SHALL BE AIR ENTRAINED WITH A MINIMUM OF 4-12 PERCENT AND A MAXIMUM OF 6 PERCENT AIR ENTRAINMENT.
- THE CONCRETE MIX SHALL HAVE AN ULTIMATE COMPRESSIVE STRENGTH OF 5,000 PSI AT 28 DAYS.
- ENDS AND CORNERS OF SQUARE PILES SHALL BE CHAMFERED.
- DRIVE 12" SQUARE PILES IN ONE PIECE TO A MINIMUM EMBEDDED LENGTH OF 50 FEET TO DEVELOP A MINIMUM LOAD CARRYING CAPACITY OF 20 TONS.
- IF INDICATED MINIMUM EMBEDMENT LENGTH IS REACHED PRIOR TO OBTAINING THE ULTIMATE PILE BEARING CAPACITY, CONTINUE DRIVING UNTIL CAPACITY IS OBTAINED.
- EMBEDMENT LENGTH IS DEFINED AS THE DEPTH OF PILE FROM THE GROUND SURFACE AT THE LOCATION OF EACH PILE TO THE TIP OF THE PILE. GIVEN THE PILE LOCATIONS AND THE TOPOGRAPHY, OVERALL PILE LENGTHS MAY VARY.
- PROVIDE STEEL DRIVING HELMET OR CAP INCLUDING A PILE CUSHION BETWEEN TOP OF PILE AND DRIVING HELMET OR CAP TO PREVENT IMPACT DAMAGE TO PILE.
- USE A DRIVING HAMMER CAPABLE OF DEVELOPING THE ULTIMATE PILE CAPACITY CONSIDERING HAMMER IMPACT VELOCITY, RAM WEIGHT, STIFFNESS OF M/HAMMER AND PILE CUSHIONS, CROSS SECTION, LENGTH, AND TOTAL LENGTH OF PILE, AND CHARACTER OF SUBSURFACE MATERIAL TO BE ENCOUNTERED.
- SUBSOIL INFORMATION IS AVAILABLE FROM THE OWNER.
- DRIVE PILES WITH A VARIATION OF NOT MORE THAN 2 PERCENT FROM VERTICAL FOR PLUMB PILE. MAINTAIN AND CHECK AXIAL ALIGNMENT OF PILE AND LEADERS AT ALL TIMES.
- PLACE BUTTS WITHIN FOUR (4) INCHES OF LOCATION INDICATED.
- JETTING AND/OR PRE-DRILLING OF PILE SHALL NOT BE PERMITTED.
- CUT OFF PILES WITH A SMOOTH LEVEL CUT USING PNEUMATIC TOOLS, SAWING, OR OTHER SUITABLE METHODS. EXTEND PRE STRESSING STANDS/WIRES INTO PILE CAP AND TIES TO REINFORCING STEEL. PROVIDE GROUTED DOWELS AS INDICATED.

MASONRY NOTES

- ALL MASONRY WORK SHALL BE PERFORMED IN ACCORDANCE WITH ACI 530, LATEST EDITION.
- COORDINATE LOCATION, SIZES OF MASONRY, AND LOCATION OF OPENINGS IN MASONRY WITH ARCHITECTURAL.
- MATERIALS**

CONCRETE BLOCK:	ASTM C90 GRADE 1 TYPE N1
MORTAR:	ASTM C270 TYPE S
GROUT:	ASTM C476
REINFORCING BARS:	ASTM A615 GRADE 60
JOINT REINFORCING:	ASTM A991, LADDER TYPE
EXTERIOR JT REINF.:	GALV. PER ASTM A 153
INTERIOR JT. REINF.:	GALV. PER ASTM A 641
- THE MINIMUM COMPRESSIVE STRENGTH OF THE MASONRY (FM) SHALL BE 2,000 PSI. UON. THIS STRENGTH SHALL BE OBTAINED IN ACCORDANCE WITH THE ABOVE REFERENCED SPECIFICATIONS FOR MASONRY STRUCTURES.
- MORTAR SHALL BE TYPE S/N PORTLAND CEMENT LIME OR MORTAR CEMENT PROPORTIONED IN ACCORDANCE WITH ASTM C 270. MINIMUM COMPRESSIVE STRENGTH 1800 PSI
- ALL CELLS SHALL HAVE A CLEAN INTERIOR SURFACE WITHOUT FINIS, SHELF, OR OTHER PROJECTIONS THAT MAY RESTRICT THE PLACEMENT OF GROUT.
- FILL ALL REINFORCED CELLS OF CMU AND CELLS BELOW GRADE WITH COARSE GROUT (PEA GRAVEL CONCRETE, FC=2500 PSI), MINIMUM AGGREGATE SIZE 3/8" AND A 6" SLUMP.
- ALL GROUTING SHALL BE IN ACCORDANCE WITH ASTM C-476 AND GROUTED SOLID FROM BOTTOM TO TOP IN LIFTS NOT TO EXCEED 4'-0".
- REINFORCING BARS SHALL CONFORM TO ASTM 615, GRADE 60.
- ALL BARS SHALL BE TIED AT SPLICES AND LAPPED A MINIMUM OF 48 BAR DIAMETERS, UNO.
- PLACE VERTICAL REINFORCING BARS IN CENTER OF CELLS AND SECURE AGAINST DISPLACEMENT DURING GROUTING.
- IN ADDITION TO BOND BEAMS, CMU WALLS SHALL BE REINFORCED HORIZONTALLY WITH JOINT REINFORCEMENT @ 16" O.C., LADDER TYPE, GALVANIZED STEEL CONSTRUCTION, 3/16" SIDE RODS WITH 9 GAUGE CROSS TIES. LAP JOINT REINFORCEMENT ENDS 9" MINIMUM.
- PLACE JOINT REINFORCEMENT IN FIRST AND SECOND COURSES ABOVE AND BELOW OPENINGS AND EXTEND 16" (MINIMUM) TO EACH SIDE OF OPENING.
- PLACE JOINT REINFORCEMENT CONTINUOUS IN EVERY TWO COURSES BELOW TOP OF WALLS, BELOW ROOF LEVEL, AND ABOVE FLOOR LEVEL.
- STEP JOINT REINFORCEMENT WITH BLOCK COURSING AND EXTEND ENDS 16" (MINIMUM) BELOW STEPS.
- ALL BOND BEAM REINFORCEMENT SHALL BE CONTINUOUS AT ALL CORNERS AND INTERSECTING WALLS.
- BOND BEAM REINFORCING BARS SHALL BE PLACED AT 12" ABOVE THE BOTTOM OF THE BLOCK AND HELD SECURELY IN POSITION DURING GROUTING.
- HORIZONTAL BARS ARE TO BE ATTACHED TO SPECIAL REINFORCEMENT AT END OF WALLS WITH A STANDARD 180° HOOK.
- ALL DOWELS AT STEM WALL TO BE ATTACHED DIRECTLY TO THE HORIZONTAL STEEL IN THE FOOTING.
- PROVIDE CORNER JOINTS AT 24" O.C. THROUGH EACH SIDE OF EACH OPENING IN CMU WALLS. FILL JOINTS WITH WEATHERPROOF ELASTIC SEALANT MEETING STANDARDS OF ASTM C920.
- AT VERTICAL CORNER JOINTS, PROVIDE ROUND SMOOTH BARS WITH ONE END GREASED TO ALLOW FOR LATERAL MOVEMENT.
- STRUCTURAL GROUT SHALL BE COARSE-12" WYTHE, FINE-8" WYTHE WITH 8 TO 10" SLUMP COMPRESSIVE STRENGTH 2500 PSI @ 28 DAYS. SEE ASTM C476 FOR REQUIREMENTS.
- FOR ALL GROUTING PROVIDE 3"x4" CLEAN-OUT OPENINGS OF ALL REINFORCED CELLS. CLEAN OUT PLUGS FILLED IN AFTER CELL INSPECTION AND BEFORE POUR.
- ALLOW MORTAR 3 TO 5 DAYS TO CURE DEPENDING ON WEATHER.

STRUCTURAL STEEL NOTES

- ALL DESIGN AND CONSTRUCTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AISC MANUAL OF STEEL CONSTRUCTION (FOURTEENTH EDITION).
- MATERIAL:**

A. W-SHAPE	ASTM A992, Fy=50KSI
B. CHANNELS, ANGLES, & PLATE	ASTM A36, Fy=36KSI
C. RECTANGULAR HSS	ASTM 1095 OR A500 GRADE B, Fy=46KSI
D. HIGH-STRENGTH BOLT	ASTM A325 N
E. ANCHOR ROD	ASTM F1554
F. THREADED ROD	ASTM A36
- NO SECOND HAND MATERIAL PERMITTED
- MAKE ALL FIELD MEASUREMENTS REQUIRED TO VERIFY DIMENSIONS.
- CONNECTIONS:**

A. 3/4" - A325 BOLTS (UNLESS OTHERWISE NOTED ON PLANS	
B. FIELD WELD: E50 OR E70 SERIES. ELECTRODES SHALL BE SUITABLE FOR THE POSITIONS AND CONDITIONS OF INTENDED USE.	
- UNLESS SHOWN OTHERWISE, PROVIDE STANDARD FRAMED OR SEATED BEAM CONNECTIONS AS SHOWN IN PART 4 OF THE AISC "MANUAL OF STEEL CONSTRUCTION".
- UNLESS GREATER REACTIONS ARE INDICATED ON THE PLANS, PROVIDE CONNECTIONS DEVELOPING AT LEAST ONE HALF OF THE TOTAL UNIFORM LOAD CAPACITY TABULATED IN THE TABLES OF THE AISC MANUAL FOR THE GIVEN SECTION AND SPAN OF THE BEAM.
- IN NO CASE SHALL THE LENGTH OF THE FRAMED CONNECTIONS BE LESS THAN ONE HALF THE "T" DIMENSION OF THE BEAM.
- UNLESS NOTED OTHERWISE, BEAMS ON MASONRY WALLS SHALL BEAR A LENGTH EQUAL TO THE BEAM DEPTH, 8 INCHES MINIMUM.
- ERECTION: PROVIDE ADEQUATE EQUIPMENT TO PERFORM THE WORK WITHOUT DAMAGE TO PROPERTY AND PROVIDE COMPLETE SAFETY TO PUBLIC, WORKMEN AND PROPERTY.
- SHOP PAINT: STRUCTURAL STEEL SHALL BE PAINTED IN ACCORDANCE WITH THE SPECIFICATION SECTION 09900, UNLESS NOTED AS HOT-DIP GALVANIZED.
- FIELD PAINT: TOUCH UP ALL BOLTS AND WELDS WITH SHOP PAINT.
- CONTRACTOR SHALL SUBMIT ERECTION PLANS AND DETAIL SHOP DRAWINGS FOR REVIEW BY ENGINEER BEFORE FABRICATION, INCLUDING STEEL CONNECTIONS, STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTORS SCOPE OF SERVICE. SHOP DRAWINGS OF SUCH CONNECTION SHALL BE SUBMITTED FOR REVIEW. DESIGN OF CONNECTIONS DOES NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS.
- THE CONTRACTOR SHALL NOT CUT OR ALTER IN ANY WAY THE STRUCTURAL MEMBERS WITHOUT THE APPROVAL OF THE ENGINEER.
- GROUT UNDER ALL BASE PLATES AND BEARING PLATES WITH NON-SHRINK TYPE GROUT, MINIMUM COMPRESSIVE STRENGTH OF 7000 PSI AT 28 DAYS.
- ALL WELDED CONSTRUCTION SHALL BE IN ACCORDANCE WITH AWS D1.1 AND AISC. ALL WELDING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS AND IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY STANDARD D1.1, LATEST EDITION.
- FOR STEEL MEMBERS AND EMBEDMENT EXPOSED TO WEATHER, PROVIDE HOT-DIPPED GALVANIZED STEEL. BOLTS CONNECTING ANY GALVANIZED MEMBER SHALL BE HOT-DIP GALVANIZED.
- WHERE NO CAMBER IS INDICATED, BEAMS SHALL BE INSTALLED SO THAT, ANY NATURAL CAMBER IS UPWARD.

METAL STUD FRAMING NOTES

- INSTALL ALL LIGHT GAGE STEEL FRAMING IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- LIGHT GAGE METAL FRAMING SHALL BE GALVANIZED IN ACCORDANCE WITH A653 G60. GALVANIZING DAMAGED BY INSTALLATION OPERATIONS OF WELDING SHALL BE REPAIRED USING GALVANIZING REPAIR PAINT CONFORMING TO SSPC-PAINT 20 OR DOD-P-21035
- ERECT LIGHT GAGE METAL FRAMING IN ACCORDANCE WITH ASTM C-955.
- CONTRACTOR TO PROVIDE DOUBLE STUDS AT EACH SIDE OF OPENING, TRIPLE STUDS AT EACH CORNER, AND BOX HEADERS OVER EVERY OPENING.
- CONTRACTOR TO PROVIDE MINIMUM 27MILL STEEL ASTM A1003 GRADE 33 SHEATHING FOR ALL EXTERIOR WALLS FOR THE EXTERIOR FACE. SHEATHING IS TO BE APPLIED PERPENDICULAR TO FRAMING WITH STRAP BLOCKING BEHIND THE HORIZONTAL JOINT AND SOLID BLOCKING BETWEEN THE FIRST TWO END STUDS. SCREW SPACING AS SHOWN IN DETAIL.
- CONTRACTOR TO PROVIDE 5/8" GYPSUM BOARD FOR ALL EXTERIOR WALLS FOR THE INTERIOR FACE. GYPSUM BOARD IS TO BE APPLIED PERPENDICULAR TO THE FRAMING WITH STRAP BLOCKING BEHIND THE HORIZONTAL JOINT AND SOLID BLOCKING BETWEEN THE FIRST TWO END STUDS. SCREW SPACING TO BE 6" O.C. ON THE EDGES AND IN THE FIELD USING #6-20x3/4" SCREWS.
- CONTRACTOR TO PROVIDE 5/8" GYPSUM BOARD FOR ALL INTERIOR WALLS FOR LOAD BEARING OR SHEAR WALLS. GYPSUM BOARD IS TO BE APPLIED PERPENDICULAR TO THE FRAMING WITH STRAP BLOCKING BEHIND THE HORIZONTAL JOINT AND SOLID BLOCKING BETWEEN THE FIRST TWO END STUDS. SCREW SPACING TO BE 6" O.C. ON THE EDGES AND IN THE FIELD USING #6-20x3/4".
- MINIMUM MATERIALS SPECIFICATIONS**

DESIGNATION:	600s200-43
SPACING:	24"
YIELD STRENGTH:	50 KSI
BRIDGING:	CONTINUOUS @ 4'-0" O.C.
- SHEAR WALL**

DESIGNATION:	600s200-43
SPACING:	16"
YIELD STRENGTH:	50 KSI
BRIDGING:	CONTINUOUS @ 4'-0" O.C.
- TRACKS**

DESIGNATION:	600s200-43
YIELD STRENGTH:	50 KSI
- CONNECTIONS:**

TRACKS TO CONCRETE:	(2) 0.145"Ø PDF SPACED @ 16" O.C. W/ MIN 13/4" PENETRATION OR HILTI DX PAF SPACED @ 16" O.C. W/ 0.177"Ø SHANK AND 17/16" PENETRATION.
TRACKS TO STEEL:	(2) 0.145"Ø PDF SPACED @ 16" O.C. W/ KNURLED SHANK.
STUDS TO TRACKS:	(2) 0.145"Ø PDF SPACED @ 16" O.C. W/ KNURLED SHANK OR 1" WELDED EACH STUD FLANGE TO TRACK LEG.

FLOOD PROOFING LOADS

- FLOOD INSURANCE RATE MA9 (FIRM) 45019C05121 - 11/17/04
- FLOOD ZONE: AE14 (ELEVATION 14.0 FT)
- DESIGN FLOOD ELEVATION = 15'-0" (INCLUDING 1-FT OF FREEBASE)
- DESIGN STILL WATER ELEVATION = 3.5 FT.
- FINISH FLOOR ELEVATION 9'-0".

WATERPROOFING / FLOODPROOFING

- PRIOR TO THE APPLICATION OF WATERPROOFING MATERIALS ON CONCRETE WALLS, HOLES AND RECESSES RESULTING FROM THE REMOVAL OF FORM TIES SHALL BE SEALED WITH A BITUMINOUS MATERIAL OR OTHER APPROVED METHODS OR MATERIALS.
- WALL WATERPROOFING SHALL BE APPLIED FROM THE BOTTOM OF THE WALL TO NOT LESS THAN 12 INCHES ABOVE THE BASE FLOOD ELEVATION. THE REMAINDER OF THE WALL SHALL BE DAMP PROOFED IN ACCORDANCE WITH SECTION 1805.2.2 OF THE 2015 IBC.
- FLOOR WATERPROOFING SHALL BE ACCOMPLISHED BY PLACING A MEMBRANE AS SPECIFIED IN WATERPROOFING SPECIFICATION WITH JOINTS NOT LAPPED LESS THAN 6 INCHES OR OTHER APPROVED MATERIALS UNDER THE SLAB. JOINTS SHALL BE LAPPED AND SEALED IN ACCORDANCE WITH THE MANUFACTURER'S INSTALLATION INSTRUCTIONS. (REF. IBC 1805.3.1)
- WALL WATERPROOFING SHALL CONSIST OF POLYMER MODIFIED CEMENT. (REF. A-DWGS)
- FLOOD BARRIERS SHALL BE DESIGNED TO MEET THE REQUIREMENTS OF ASCE-24 BASED ON THE DESIGN CRITERIA PROVIDED IN THESE DRAWINGS. SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY THE SPECIALTY ENGINEER, REGISTERED IN SOUTH CAROLINA, SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL.

METAL ROOF DECK NOTES

- ROOF DECK SHALL CONFORM TO STEEL DECK INSTITUTE DESIGN MANUAL, L.E.
- PROVIDE STEEL DECK WITH DEPTH INDICATED ON THE DRAWINGS AND MINIMUM THICKNESS OF 22 GAUGE, UON, TO CONFORM TO ASTM A583, GRADE 33, MINIMUM Fy = 33 KSI.
- DECKING IS TO BE GALVANIZED PER ASTM A653, G60. TOUCH UP GALVANIZED SURFACES WITH GALVANIZING REPAIR PAINT APPLIED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
- THE DESIGN, MANUFACTURE AND ERECTION OF STEEL ROOF DECK AND ITS ANCHORAGE SHALL, AT A MINIMUM, BE IN ACCORDANCE WITH CODES / STANDARDS NOTED.
- ROOF DECKING SHALL BE FASTENED AS FOLLOWS:

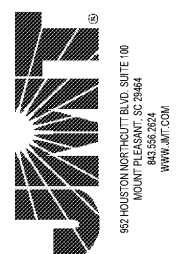
A. INTERIOR SUPPORTS:	3/4" PUDDLE WELD @ 12" O.C.
B. PERIMETER SUPPORTS:	3/4" PUDDLE WELD @ 12" O.C.
C. SIDE LAPS:	#10 TEXS @ 18" O.C.
- DECK IS SPECIFIED ON A THREE SPAN CONDITION. BURNISH HEAVY DUTY DECK FOR ONE OR TWO SPAN CONDITIONS.
- FASTEN DECK TO RESIST NET UPLIFT OF 85 PSF TYPICAL AND 115 PSF IN THE ENDS AND CORNER ZONES.
- FASTEN ROOF DECK TO RESIST A DIAPHRAGM SHEAR FORCE OF 375 PSF.

ROOF TRUSS NOTES

- COLD-FORMED STEEL TRUSSES SHALL BE DESIGNED FOR THE DESIGN LOADS SHOWN IN THE CONTRACT DOCUMENTS. REFER TO THE SPECIFICATIONS.
- COLD FORMED STEEL TRUSSES SHALL BE BRACED AND ERECTED IN ACCORDANCE WITH AISI "SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" OR "LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR COLD-FORMED STEEL STRUCTURAL MEMBERS".
- BRACING IN THE PLANE OF WEB MEMBERS:
 - THE TRUSS FABRICATOR SHALL PROVIDE AND LOCATE CONTINUOUS LATERAL BRACING FOR EACH TRUSS WEB MEMBER AS REQUIRED.
 - LATERAL BRACING SHALL BE RESTRAINED BY DIAGONAL BRACING. THIS BRACING IS TO BE CONTINUOUS.
 - A MINIMUM OF TWO ROWS OF DIAGONAL BRACING IS REQUIRED. ONE AT EACH VERTICAL WEB MEMBER CLOSURES TO BEARING LOCATIONS.
- THE BOTTOM CHORDS SHALL BE BRACED BY CONTINUOUS LATERAL BRACING SPACED AT 8 TO 10 FEET ATTACHED TO THE TOP OF THE BOTTOM CHORD. DIAGONALS PLACED AT 45° TO THE LATERAL BRACES SHALL BE LOCATED AT EACH END, IF BUILDING EXCEEDS 60 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.
- TOP CHORD BRACING:
 - IF DECKING IS APPLIED DIRECTLY TO TOP CHORD, PROPERLY LAPPED AND ATTACHED TO DEVELOP DIAPHRAGM ACTION, BRACING IS NOT REQUIRED.
 - IF PURLINS ARE USED, DIAGONAL TOP CHORD BRACING IS REQUIRED AT EACH END, IF BUILDING EXCEEDS 60 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.
- COLD FORMED ROOF TRUSSES ARE TO BE DESIGNED BY THE COLD FORMED STEEL FABRICATOR BY A PROFESSIONAL ENGINEER AND SEALED CALCULATIONS AND DRAWINGS ARE TO BE SUBMITTED FOR REVIEW.

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GENERAL NOTES

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BID DOCUMENTS