DESIGN PARAMETERS

1. BUILDING CODE — IBC 2012 WITH GEORGIA AMENDMENTS
   RISK CATEGORY — II
2. LIVE LOADS
   A. ROOF — 20 PSF
3. ROOF SNOW LOAD
   A. GROUND SNOW LOAD, Pg — 5 PSF
   B. FLAT ROOF SNOW LOAD (INCLUDES 5 PSF RAIN-ON-SNOW SURCHARGE, Pf — 8.5 PSF
   C. SNOW EXPOSURE FACTOR, Ce — 1.0
   D. SNOW LOAD IMPORTANCE FACTOR, I — 1.0
   E. THERMAL FACTOR, Ct — 1.0
4. WIND DESIGN DATA
   A. BASIC WIND SPEED (3 SECOND GUST), Vult / Vasd — 115 MPH / 90 MPH
   B. WIND EXPOSURE CATEGORY — C
   C. INTERNAL PRESSURE COEFFICIENT, GCpi — +/- 0.18
   D. WIDTH OF END ZONE — 6.9 FT
5. EARTHQUAKE DESIGN DATA
   A. SEISMIC IMPORTANCE FACTOR, Ie — 1.0
   B. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, Ss — 19.9%
   C. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S1 — 9.3%
   D. SITE CLASS — D
   E. DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETER, Sds — 0.212
   F. DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETER, Sd1 — 0.148
   G. SEISMIC DESIGN CATEGORY — C
6. MINIMUM LATERAL LOAD ON INTERIOR PARTITIONS — 5 PSF

GENERAL NOTES

GENERAL

1. STRUCTURAL ELEMENTS ARE NON-SELF SUPPORTING AND REQUIRE INTERACTION WITH OTHER ELEMENTS FOR STABILITY AND RESISTANCE TO LATERAL FORCES. FRAMING AND WALLS SHALL BE TEMPORARILY BRACED BY THE CONTRACTOR UNTIL PERMANENT BRACING, FLOOR AND ROOF DECKS, AND WALLS HAVE BEEN INSTALLED AND CONNECTIONS BETWEEN THESE ELEMENTS HAVE BEEN MADE.
2. THE CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION, UNLESS NOTED OTHERWISE. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATION OF CONSTRUCTION AND SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO.
3. USE ONLY DIMENSIONS INDICATED IN THE CONTRACT DOCUMENTS. DO NOT SCALE CONTRACT DOCUMENTS OR USE ANY DIMENSIONS TAKEN FROM ELECTRONIC DRAWING FILES.
4. ASSUME EQUAL SPACING IF NOT INDICATED IN CONTRACT DOCUMENTS.
5. ARCHITECTURAL, MECHANICAL AND ELECTRICAL COMPONENTS AND SYSTEMS SHALL BE DESIGNED AND CONSTRUCTED TO RESIST SEISMIC FORCES AS DETERMINED IN CHAPTER 13 OF ASCE 7.
6. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ADEQUATE TEMPORARY SUPPORT AND STABILITY OF EXISTING STRUCTURE DURING ALL PHASES OF CONSTRUCTION.
7. DIMENSIONS AND DETAILS OF THE EXISTING STRUCTURE ARE BASED UPON DOCUMENTS PROVIDED BY THE OWNER. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS AND REPORT TO THE ENGINEER-OF-RECORD ANY VARIATIONS FROM THE DATA SHOWN HEREIN FOR POSSIBLE REDESIGN.
8. BEFORE OR CONCURRENT WITH EXCAVATIONS FOR THE FOUNDATIONS ADJACENT TO THE EXISTING BUILDING, PROVIDE ADEQUATE SUPPORT TO THE EXISTING SUBBASE OF THE EXISTING SLAB AND THE FOUNDATIONS TO PREVENT UNDERMINING.
9. DURING WELDING OR ANY OTHER CONSTRUCTION ACTIVITY THAT GENERATES SPARKS OR INTENSE HEAT, THE CONTRACTOR SHALL PROVIDE ADEQUATE FIRE PROTECTION TO THE EXISTING STRUCTURE AND CONTENTS.

CONCRETE

1. MINIMUM COMPRESSIVE STRENGTH (f'c) AT THE END OF 28 DAYS SHALL BE AS FOLLOWS:
   A. FOOTINGS — 3000 PSI
   B. SLABS-ON-GRADE — 3000 PSI
2. THE MAXIMUM WATER-TO-CEMENT RATIO SHALL BE 0.44; MAXIMUM SIZE AGGREGATE SHALL BE 3/4 INCH; SLUMP LIMIT SHALL BE 4 INCHES PLUS OR MINUS 1 INCH. CONCRETE SHALL BE NORMAL WEIGHT (145 PCF), UNLESS NOTED OTHERWISE.
3. EXTERIOR CONCRETE AND CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL BE AIR-ENTRAINED. THE AIR CONTENT SHALL BE 6 PERCENT PLUS OR MINUS 1.5 PERCENT.
4. MATERIALS OR ADMIXTURES SHALL NOT CONTAIN ANY CALCIUM CHLORIDE.
5. REINFORCING STEEL SHALL MEET THE FOLLOWING:
   A. DEFORMED BARS — ASTM A615, GRADE 60
   B. WELDED WIRE FABRIC — ASTM A185
6. WHERE DOWELS ARE INDICATED BUT NOT SIZED, PROVIDE DOWELS THAT MATCH SIZE AND LOCATION OF MAIN REINFORCING STEEL AND LAP SPLICE WITH THE MAIN REINFORCING STEEL. REINFORCING BARS SHALL BE SPLICED AS NOTED IN THE REINFORCING LAP SCHEDULE.
7. REFER TO ACI 318 LATEST EDITION FOR CONCRETE COVER, ACI 315 LATEST EDITION FOR DETAILING PRACTICES AND FABRICATION, AND ACI 301 LATEST EDITION FOR STANDARD PRACTICE FOR MIXING AND PLACING CONCRETE.
8. "C.J." INDICATES SAW CUT CONTRACTION JOINT OR DOWELED CONSTRUCTION JOINT IN SLAB-ON-GRADE. REFERENCE CAST-IN-PLACE CONCRETE SPECIFICATION FOR ACCEPTED SAW CUT METHODS. SLAB POURS SHALL BE SEPARATED BY A DOWELED CONSTRUCTION JOINT. CONTRACTION/CONSTRUCTION JOINTS SHALL BE LOCATED AS SHOWN ON PLANS OR AS DIRECTED BY THE ENGINEER-OF-RECORD.
9. PROVIDE CORNER BARS THAT MATCH AND LAP CONTINUOUS REINFORCEMENT SIZE AND QUANTITY AT INTERSECTIONS AND CORNERS OF WALLS AND FOUNDATIONS.
10. ANCHORS INSTALLED IN HARDENED CONCRETE SHALL ONLY BE USED WHERE SPECIFIED ON THE CONTRACT DRAWING. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REINFORCING. HOLES SHALL BE DRILLED, DRY AND CLEANED AND ANCHORS INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED WRITTEN INSTRUCTIONS AND APPLICABLE ESR REPORT. REFERENCE DETAILS FOR ANCHOR SIZE AND EMBEDMENT. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED ON THE CONTRACT DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY THE PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION AND LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL MEET THE FOLLOWING MINIMUM YIELD STRESS (Fy):

	YIELD	ASTM SPECIFICATION
A. W, WT SHAPES:	50 KSI	A992
B. BARS, PLATES, CHANNELS, ANGLES:	36 KSI	A36
C. SQUARE, RECTANGULAR HSS:	46 KSI	A500, GRADE B
D. ROUND HSS:	42 KSI	A500, GRADE B
E. STRUCTURAL STEEL PIPE:	35 KSI	A53, GRADE B
F. ANCHOR RODS:	36 KSI	F1554
G. ALL-THREAD RODS:	36 KSI	A36

2. BOLTS FOR STEEL BEAM AND COLUMN CONNECTIONS SHALL BE 3/4-INCH DIAMETER ASTM A325-N HIGH-STRENGTH BOLTS UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS. ALL BOLTED CONNECTIONS ARE BEARING TYPE UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS. ALL BOLTS SHALL BE TIGHTENED SNUG TIGHT UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS .
3. WELDING SHALL MEET AWS D1.1 STRUCTURAL WELDING CODE LATEST REVISION. ELECTRODES SHALL BE 70 KSI LOW HYDROGEN.
4. PROVIDE DOUBLE NUTS AND DOUBLE WASHERS FOR STEEL COLUMN ANCHOR BOLTS TO ALLOW FOR ADJUSTMENT IN BASE PLATE ELEVATION. PROVIDE 1 1/2 INCH NON-SHRINK GROUT UNDER BASE PLATE AFTER ERECTION. USE 2 1/2 INCH NON-SHRINK GROUT WHEN COLUMN ANCHOR BOLTS ARE 1 1/4 INCH DIAMETER OR LARGER. NON-SHRINK GROUT SHALL BE NON-METALLIC WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS.
5. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR INCLUDING THE COSTS FOR ALL MISCELLANEOUS STEEL SHOWN IN THE CONTRACT DOCUMENTS. THESE COSTS SHALL INCLUDE, BUT ARE NOT LIMITED TO, MISCELLANEOUS STEEL ITEMS SHOWN ON THE STRUCTURAL, ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.

STEEL DECK

1. ROOF DECK
   A. ROOF DECK SHALL BE GALVANIZED TYPE "B". DEPTH SHALL BE AS SHOWN IN THE CONTRACT DOCUMENTS.
   B. ROOF DECK IS REQUIRED TO ACT AS A DIAPHRAGM. CONNECTIONS SHALL BE IN ACCORDANCE WITH STEEL DECK INSTITUTE SPECIFICATIONS. REFER TO THE ROOF DIAPHRAGM CONNECTION DIAGRAM FOR ATTACHMENT.
   C. DECKING SHALL BE CONTINUOUS OVER A MINIMUM OF (3) SPANS UNLESS NOTED OTHERWISE IN THE CONTRACT DOCUMENTS.
   D. NO HANGING LOADS SHALL BE ATTACHED TO ROOF DECK.

GENERAL NOTES

COLD FORMED METAL FRAMING

1. ALL COLD FORMED METAL FRAMING SHALL HAVE A MINIMUM THICKNESS OF 33 MILS (20 GA) AND SHALL BE SPACED AT A MAXIMUM OF 16 INCHES ON CENTER UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS AND SHALL MEET THE MINIMUM STRUCTURAL PROPERTIES FROM THE AMERICAN IRON AND STEEL INSTITUTE - NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING LATEST EDITION. MINIMUM FLANGE WIDTH OF FRAMING MEMBERS SHALL BE 1 5/8 INCH AND THE LIP LENGTH OF THE C-SHAPE PORTION SHALL BE A MINIMUM OF 1/2 INCH.
2. WALL STUDS AS BACKING TO MASONRY VENEER SHALL SHALL HAVE A MINIMUM THICKNESS OF 43 MILS (18 GA).
3. METAL FRAMING SHALL BE IN ACCORDANCE WITH THE FOLLOWING, UNLESS NOTED OTHERWISE:
   A. 54 MILS (16 GA) AND HEAVIER — ASTM A1003, GRADE 50 TYPE H (ST50H)
   B. 43 MILS (18 GA) AND LIGHTER — ASTM A1003, GRADE 33 TYPE H (ST33H)
   C. ACCESSORIES, TRACK AND OTHER MEMBERS — ASTM A1003, GRADE 33 TYPE H (ST33H), MINIMUM
4. DO NOT WELD 33 MILS (20 GA) AND LIGHTER FRAMING, UNLESS SPECIFICALLY NOTED IN THE CONTRACT DOCUMENTS.
5. METAL FRAMING SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN RECOMMENDATIONS. HORIZONTAL BRACING FOR WALL STUDS SHALL BE PLACED AT 48 INCHES ON CENTER OR AS PER MANUFACTURER'S WRITTEN RECOMMENDATIONS IF LESS THAN 48 INCHES ON CENTER. HORIZONTAL BRIDGING FOR JOISTS SHALL BE PLACED AT 8'-0" ON CENTER OR AS PER MANUFACTURER'S WRITTEN RECOMMENDATIONS IF LESS THAN 8'-0" ON CENTER. APPLIED FINISH MATERIALS SHALL NOT BE CONSIDERED BRIDGING OR FLANGE BRACING UNLESS NOTED OTHERWISE IN THE CONTRACT DOCUMENTS.
6. ALL AXIALLY LOADED WALL STUDS SHALL HAVE FULL FLANGE BEARING AGAINST UPPER AND LOWER TRACK WEB PRIOR TO ATTACHMENT TO TRACK. SPLICES IN AXIALLY LOADED WALL STUDS ARE NOT ALLOWED.
7. TRACK SHALL BE 54 MILS (16 GA) MINIMUM FOR WALL STUDS 54 MILS (16 GA) OR LIGHTER. TRACK SHALL MATCH WALL STUD THICKNESS FOR WALL STUDS 68 MILS (14 GA) AND HEAVIER. TRACKS SHALL BE ANCHORED AS FOLLOWS:
   TO STEEL - HILTI X-U, 0.157 INCH DIAMETER KNURLED SHANK FASTENERS AT 12 INCHES ON CENTER (ESR-2269) OR APPROVED EQUAL, UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS.
   TO CONCRETE - HILTI X-U, 0.157 INCH DIAMETER KNURLED SHANK FASTENERS AT 8 INCHES ON CENTER WITH 1 1/2 INCH EMBEDMENT (ESR-2269) OR APPROVED EQUAL, UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS.
8. CONNECTIONS SHALL CONSIST OF ANY OF THE FOLLOWING AS NOTED IN THE CONTRACT DOCUMENTS:
   A. SELF-DRILLING SCREWS OF TYPE AND SIZE AS SHOWN IN THE CONTRACT DOCUMENTS.
   B. WELDS SHALL BE PERFORMED BY OPERATORS QUALIFIED IN ACCORDANCE WITH SECTION 6.0 OF AWS D1.3, SHEET METAL.

ABBREVIATIONS

A.B.	ANCHOR BOLTS
A.F.F.	ABOVE FINISHED FLOOR
ACI	AMERICAN CONCRETE INSTITUTE
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
ARCH.	ARCHITECTURAL
B.L.	BLOCK LINTEL
B.O.	BOTTOM OF
B.O.D.	BOTTOM OF DECK
BAL.	BALANCE
BLDG.	BUILDING
BRG.	BEARING
C.J.	CONTRACTION JOINT
C.L.	CENTER LINE
CLR.	CLEAR
CMU	CONCRETE MASONRY UNIT
COL.	COLUMN
CONC.	CONCRETE
CONST.	CONSTRUCTION
CONT.	CONTINUOUS
D.B.A.	DEFORMED BAR ANCHOR
DIA.	DIAMETER
DWG.	DRAWING
E.F.	EACH FACE
E.J.	EXPANSION JOINT
E.O.D.	EDGE OF DECK
E.O.S.	EDGE OF SLAB
E.W.	EACH WAY
ELEV.	ELEVATION
EQ.	EQUAL
EXIST.	EXISTING
F.F.E.	FINISHED FLOOR ELEVATION
F.S.	FAR SIDE
FDN.	FOUNDATION
FTG.	FOOTING
G.B.	GRADE BEAM
GA.	GAGE
GALV.	GALVANIZED
H.S.A.	HEADED STUD ANCHOR
HORIZ.	HORIZONTAL
IBC	INTERNATIONAL BUILDING CODE
INFO.	INFORMATION
J.B.E.	JOIST BEARING ELEVATION
JT.	JOINT
K	UNIT OF 1,000 POUNDS (KIP)
KSI	KIPS PER SQUARE INCH

ABBREVIATIONS

LBS.	POUNDS
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG.	LONGITUDINAL
MAX.	MAXIMUM
MECH.	MECHANICAL
MFR.	MANUFACTURER
MIN.	MINIMUM
MISC.	MISCELLANEOUS
N.I.C.	NOT IN CONTRACT
N.S.	NEAR SIDE
N.T.S.	NOT TO SCALE
NO.	NUMBER
O.C.	ON CENTER
O.D.	OUTSIDE DIAMETER
O.H.	OPPOSITE HAND
P.A.F.	POWER ACTUATED FASTENER
P.M.E.J.	PREMOLDED EXPANSION JOINT
PCF	POUNDS PER CUBIC FOOT
PLF	POUNDS PER LINEAR FOOT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
QTY.	QUANTITY
R.O.	ROUGH OPENING
RE:	REFER
REINF.	REINFORCING
REQD.	REQUIRED
RTU	ROOF TOP UNIT
S.D.S.	SELF-DRILLING SCREWS
SCHED.	SCHEDULE
SIM.	SIMILAR
SPECS.	SPECIFICATIONS
STD.	STANDARD
STL.	STEEL
T&B	TOP AND BOTTOM
T.O.	TOP OF
T.O.P.	TOP OF PIER
T.O.W.	TOP OF WALL
TRANS.	TRANSVERSE
TYP.	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
W.P.	WORK POINT
W.W.F.	WELDED WIRE FABRIC
WT.	WEIGHT

SPECIAL INSPECTIONS

1. THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS PER SECTION 1704 OF THE IBC. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN THE PROJECT SPECIFICATIONS.
2. SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO COMPLETION OF THAT PHASE OF WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF WORK.
3. THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SPECIAL INSPECTOR REGARDING INDIVIDUAL INSPECTION FOR ITEMS LISTED ON THE STATEMENT OF SPECIAL INSPECTIONS AND AS NOTED ON THE BUILDING DEPARTMENT APPROVED PLANS. ADEQUATE NOTICE AND ACCESS TO APPROVED PLANS SHALL BE PROVIDED SO THAT THE SPECIAL INSPECTOR HAS TIME TO BECOME FAMILIAR WITH THE PROJECT.
4. FABRICATORS OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1704.2 OF THE IBC.

STATEMENT OF SPECIAL INSPECTIONS (IBC 2012)

			FREQUENCY OF INSPECTION	
		REQUIRED VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC
II.		CONCRETE CONSTRUCTION (IBC TABLE 1705.3)		
1.		INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.	---	X
2.		INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.	---	X
3.		VERIFYING USE OF REQUIRED DESIGN MIX.	---	X
4.		AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	---
5.		INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	---
6.		INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	---	X
7.		VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	---	X
8.		INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	---	X
IV.		SOILS (IBC TABLE 1705.6)		
1.		VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	---	X
2.		VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	---	X
3.		PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	---	X
4.		VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	---
5.		PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	---	X
		* CONTINUOUS SPECIAL INSPECTION: SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PREFORMED.		
		* PERIODIC SPECIAL INSPECTION: SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PREFORMED.		

STRUCTURAL STEEL SPECIAL INSPECTIONS (AISC 360-10, SPECIFICATION CHAPTER N)

		Verification and Inspection
		QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR
A.		THE FABRICATOR'S QUALITY CONTROL INSPECTOR (QCI) SHALL INSPECT THE FOLLOWING:
	1.	SHOP WELDING, HIGH-STRENGTH BOLTING, AND DETAILS PER SECTION N5
	2.	SHOP CUT AND FINISHED SURFACES PER SECTION M2
	3.	SHOP HEATING FOR STRAIGHTENING, CAMBERING AND CURVING PER SECTION M2.1
	4.	TOLERANCES FOR SHOP FABRICATION PER SECTION 6 OF THE CODE OF STANDARD PRACTICE
B.		THE ERECTOR'S QUALITY CONTROL INSPECTOR (QCI) SHALL INSPECT THE FOLLOWING:
	1.	FIELD WELDING, HIGH-STRENGTH BOLTING, AND DETAILS PER SECTION N5
	2.	STEEL DECK AND HEADED STEEL STUD ANCHOR PLACEMENT AND ATTACHMENT PER SECTION N6
	3.	FIELD CUT SURFACES PER SECTION M2.2
		FIELD HEATING FOR STRAIGHTENING PER SECTION M2.1
		TOLERANCES FOR FIELD ERECTION PER SECTION 7.13 OF THE CODE OF STANDARD PRACTICE
		QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), APPLICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE
		QUALITY ASSURANCE (QA) INSPECTION OF FABRICATED ITEMS SHALL BE MADE AT THE FABRICATOR'S PLANT. QA INSPECTION OF THE ERECTED STEEL SYSTEM SHALL BE MADE AT THE PROJECT SITE
	1.	QA INSPECTIONS, EXCEPT NDT, MAY BE WAIVED WHEN THE WORK IS PERFORMED IN A FABRICATING SHOP OR BY AN ERECTOR APPROVED BY THE AHJ TO PERFORM THE WORK WITHOUT QA
	2.	NDT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. WHEN THE FABRICATOR PERFORMS THE NDT, THE QA AGENCY SHALL REVIEW THE FABRICATOR'S NDT REPORTS
	3.	AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE FABRICATOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS
	4.	AT COMPLETION OF ERECTION, THE APPROVED ERECTOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE ERECTOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS
	5.	THE QUALITY ASSURANCE INSPECTOR (QAI) SHALL REVIEW MATERIAL TEST REPORTS AND CERTIFICATIONS AS LISTED IN SECTION N3.2 FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS
		INSPECTION TASKS LISTED IN THE TABLES BELOW SHALL BE PERFORMED BY BOTH THE FABRICATOR'S/ERECTOR'S QUALITY CONTROL INSPECTOR (QCI) AND THE QUALITY ASSURANCE INSPECTOR (QAI) UNLESS NOTED OTHERWISE:

		PERFORM - PERFORM THESE TASKS FOR EACH WELDED JOINT, MEMBER OR CONNECTION.		
		OBSERVE - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.		
		INSPECTION TASKS PRIOR TO WELDING (AISC 360-10, TABLE N5.4-1	PERFORM	OBSERVE
1.		WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	X	---
2.		MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	X	---
3.		MATERIAL IDENTIFICATION (TYPE/GRADE)	---	X
4.		WELDER IDENTIFICATION SYSTEM (a)	---	X
5.		FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)		
	A.	JOINT PREPARATION	---	X
	B.	DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	---	X
	C.	CLEANLINESS (CONDITION OF STEEL SURFACES)	---	X
	D.	TACKING (TACK WELD QUALITY AND LOCATION)	---	X
	E.	BACKING TYPE AND FIT (IF APPLICABLE)	---	X
6.		CONFIGURATION AND FINISH OF ACCESS HOLES	---	X
7.		FIT-UP OF FILLET WELDS	---	X
	A.	DIMENSIONS (ALIGNMENT, GAPS AT ROOT)	---	X
	B.	CLEANLINESS (CONDITION OF STEEL SURFACES)	---	X
	C.	TACKING (TACK WELD QUALITY AND LOCATION)	---	X
8.		CHECK WELDING EQUIPMENT (b)	---	X
	(a)	THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.		
	(b)	THIS INSPECTION TASK IS THE SOLE RESPONSIBILITY OF THE FABRICATOR'S/ERECTOR'S QUALITY CONTROL INSPECTOR (QCI)		
		INSPECTION TASKS DURING WELDING (AISC 360-10, TABLE N5.4-2)	PERFORM	OBSERVE
1.		USE OF QUALIFIED WELDERS	---	X
2.		CONTROL AND HANDLING OF WELDING CONSUMABLES	---	X
	A.	PACKAGING	---	X
	B.	EXPOSURE CONTROL	---	X
3.		NO WELDING OVER CRACKED TACK WELDS	---	X
4.		ENVIRONMENTAL CONDITIONS	---	X
	A.	WIND SPEED WITHIN LIMITS	---	X
	B.	PRECIPITATION AND TEMPERATURE	---	X
5.		WELDING PROCEDURE SPECIFICATION (WPS) FOLLOWED		
	A.	SETTINGS ON WELDING EQUIPMENT	---	X
	B.	TRAVEL SPEED	---	X
	C.	SELECTED WELDING MATERIALS	---	X
	D.	SHIELDING GAS TYPE / FLOW RATE	---	X
	E.	PREHEAT APPLIED	---	X
	F.	INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.)	---	X
	G.	PROPER POSITION (F, V, H, OH)	---	X
6.		WELDING TECHNIQUES	---	X
	A.	INTERPASS AND FINAL CLEANING	---	X
	B.	EACH PASS WITHIN PROFILE LIMITATIONS	---	X
	C.	EACH PASS MEETS QUALITY REQUIREMENTS	---	X

		INSPECTION TASKS AFTER WELDING (AISC 360-10, TABLE N5.4-3)	PERFORM	OBSERVE
1.		WELDS CLEANED	---	X
2.		SIZE, LENGTH AND LOCATION OF WELDS	X	---
3.		WELDS MEET VISUAL ACCEPTANCE CRITERIA		
	A.	CRACK PROHIBITION	X	---
	B.	WELD/BASE-METAL FUSION	X	---
	C.	CRATER CROSS SECTION	X	---
	D.	WELD PROFILES	X	---
	E.	WELD SIZE	X	---
	F.	UNDERCUT	X	---
	G.	POROSITY	X	---
4.		ARC STRIKES	X	---
5.		k-AREA (a)	X	---
6.		BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	X	---
7.		REPAIR ACTIVITIES	X	---
8.		DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	X	---
9.		ULTRASONIC TESTING (UT) ON ALL CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS, IN MATERIALS 5/16 IN. THICK OR GREATER. (required in Risk Category III or IV)	X	---
10.		ULTRASONIC TESTING (UT) ON 10% OF CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS, IN MATERIALS 5/16 IN. THICK OR GREATER (required in Risk Category II)	X	---
11.		THERMALLY CUT SURFACES OF ACCESS HOLES SHALL BE TESTED USING MAGNETIC PARTICLE TESTING (MT) OR PENETRANT TESTING (PT), WHEN FLANGE THICKNESS EXCEEDS 2 IN. FOR ROLLED SHAPES, OR WHEN THE WEB THICKNESS EXCEEDS 2 IN. FOR BUILT-UP SHAPES.	X	---
		INSPECTION TASKS PRIOR TO BOLTING (AISC 360-10, TABLE N5.6-1)	PERFORM	OBSERVE
1.		MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	(QA) X	(QC) X
2.		FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	---	X
3.		PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH)	---	X
4.		PROPER BOLTING PROCEDURES SELECTED FOR JOINT DETAIL	[illegible]	X
5.		CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS.	[illegible]	X
6.		PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED (a)	(QCI) X	(QAI) X
7.		PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	---	X
	(a)	NOT APPLICABLE FOR SNUG TIGHT JOINTS		
		INSPECTION TASKS DURING BOLTING (AISC 360-10, TABLE N5.6-2) (a)	PERFORM	OBSERVE
1.		FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	---	X
2.		JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	---	X
3.		FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING (b)	---	X
4.		FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES (b)	---	X
	(a)	NOT APPLICABLE FOR SNUG TIGHT JOINTS		
	(b)	FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE TURN-OF-NUT METHOD WITH MATCHMARKING TECHNIQUES, THE DIRECT-TENSION-INDICATOR METHOD, OR THE TWIST-OFF-TYPE TENSION CONTROL BOLT METHOD, THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS		
		INSPECTION TASKS AFTER BOLTING (AISC 360-10, TABLE N5.6-3)	PERFORM	OBSERVE
1.		DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	X	---
		OTHER INSPECTION TASKS (AISC 360-10, SECTION N5.7)	PERFORM	OBSERVE
1.		INSPECT THE STEEL TO VERIFY COMPLIANCE WITH THE DETAILS SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION	X	---
2.		INSPECT THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED PRIOR TO PLACEMENT OF CONCRETE	X	---

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CONSTRUCTION DOCUMENTS
05/10/2018
job status

No.	Description	Date

revisions

GEORGIA REGISTERED No. PE037883 PROFESSIONAL ENGINEER SAMUEL HENRY RAY
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2018.05.09 11:28:05-05'00'
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GENERAL NOTES
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