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DRAWING NOTES

GENERAL REQUIREMENTS

1. THE STRUCTURE DESCRIBED BY THESE DOCUMENTS IS INTENDED TO WORK AS A COMPLETED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION INCLUDING TEMPORARY SHORING AND BRACING. THE CONTRACTOR IS ALSO RESPONSIBLE FOR COORDINATION OF HIS OR HER WORK WITH ALL OTHER TRADES, AND FOR PERFORMING ALL WORK IN A SAFE AND SATISFACTORY MANNER.
2. ENGINEER/ARCHITECT'S APPROVAL MUST BE OBTAINED IN WRITING FOR ALL DEVIATIONS AND SUBSTITUTIONS. THE ENGINEER/ARCHITECT IS NOT RESPONSIBLE FOR THE FAILURE OF THE CONTRACTOR TO BUILD THE STRUCTURE ACCORDING TO THE DOCUMENTS.
3. THE CONTRACTOR SHALL VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO DETAILING, FABRICATION AND CONSTRUCTION; AND SHALL NOTIFY THE ENGINEER/ARCHITECT OF ANY DISCREPANCIES.
4. SHOP DRAWINGS: SUBMIT NEWLY PREPARED INFORMATION DRAWN TO SCALE. INDICATE DEVIATIONS FROM CONTRACT DOCUMENTS. DO NOT REPRODUCE CONTRACT DOCUMENTS OR COPY STANDARD INFORMATION. DOCUMENTS REPRODUCED FROM FCE DOCUMENTS WITHOUT WRITTEN PERMISSION WILL BE REJECTED. COMPLIANCE WITH SPECIFIED REQUIREMENTS REMAINS CONTRACTOR'S RESPONSIBILITY.
5. FOR ADDITIONAL INFORMATION, SEE THE PROJECT SPECIFICATIONS. IN THE CASE OF A DISCREPANCY THE MORE STRINGENT REQUIREMENTS SHALL GOVERN UNLESS APPROVED OTHERWISE IN WRITING BY THE ENGINEER.
6. OWNER SHALL EMPLOY AND PAY A QUALIFIED INDEPENDENT TESTING AGENCY TO PERFORM TESTS AND INSPECTIONS SPECIFIED IN OTHER SECTIONS, AND THOSE REQUIRED BY AUTHORITIES HAVING JURISDICTION, INCLUDING ALL SPECIAL INSPECTIONS. CONTRACTOR IS RESPONSIBLE FOR SCHEDULING INSPECTIONS AND TESTS. RETESTING: OWNER SHALL PAY FOR RETESTING WHERE RESULTS OF INSPECTIONS AND TESTS PROVE UNSATISFACTORY AND INDICATE NONCOMPLIANCE WITH REQUIREMENTS. THE OWNER RESERVES THE RIGHT TO DEDUCT COSTS OF RETESTING FROM CONSTRUCTION CONTRACT COSTS.
7. SECTIONS SHOWN ON STRUCTURAL DRAWINGS PROVIDE TYPICAL DETAILING INFORMATION THAT SHALL BE APPLIED TO ALL SIMILAR AND LIKE CONDITIONS U.N.O. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH PROJECT REQUIREMENTS.

EARTHWORK

1. AN INDEPENDENT TESTING AGENCY SHALL BE RETAINED BY THE OWNER TO PERFORM TESTING OF EARTHWORK. ALL FOOTING AND SLAB SUB-GRADES SHALL BE INSPECTED, AND TESTED IF REQUIRED, BY THE TESTING AGENCY. ALL FILL PLACEMENT AND COMPACTION SHALL BE MONITORED BY THE TESTING AGENCY. ALL BACKFILL MATERIALS SHALL BE APPROVED BY TESTING AGENCY PRIOR TO PLACEMENT.
2. FOUNDATIONS HAVE BEEN DESIGNED FOR 2500 P.S.F. ALLOWABLE SOIL BEARING PRESSURE BASED ON IN-PLANE DENSIFICATION OF THE UPPER 3 FOOT OF SOIL (SEE GEOTECHNICAL REPORT). CAPACITY SHALL BE APPROVED BY THE TESTING AGENCY PRIOR TO CONCRETE PLACEMENT.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ALL SERVICE AND UTILITY LINES ON THE SITE.
4. REFER TO PROJECT GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION. IN CASE OF DISCREPANCY, THE GEOTECHNICAL REPORT SHALL GOVERN UNLESS APPROVED OTHERWISE IN WRITING BY THE ENGINEER.

CAST-IN-PLACE CONCRETE

1. SUBMIT CONCRETE MIX DESIGNS AND LABORATORY TEST REPORTS.
2. COMPLY WITH ASTM C 94; ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS"; ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"; AND CRSI'S "MANUAL OF STANDARD PRACTICE."
3. DEFORMED REINFORCING BARS: ASTM A 615, GRADE 60.
4. WELDED STEEL WIRE FABRIC: ASTM A 185, FLAT SHEETS, NOT ROLLS.
5. PORTLAND CEMENT: ASTM C 150, TYPE 1.
6. FLY ASH: ASTM C 618, TYPE F (LIMITED TO 15 PERCENT OF TOTAL CEMENT CONTENT)
7. PROPORTION NORMAL-WEIGHT CONCRETE MIXES TO PROVIDE THE FOLLOWING PROPERTIES:
   A. COMPRESSIVE STRENGTH: 4000 PSI AT 28 DAYS FOR BUILDING FOUNDATIONS.
      4000 PSI AT 28 DAYS FOR BUILDING SLABS.
   B. WATER-CEMENT RATIO: 0.53 MAXIMUM AT POINT OF PLACEMENT FOR SLABS ON GRADE.
   C. AIR CONTENT: 5.5 TO 7.0 PERCENT FOR CONCRETE EXPOSED TO FREEZING AND THAWING, 2 TO 4 PERCENT ELSEWHERE.
8. DO NOT ADD WATER TO CONCRETE DURING DELIVERY, AT PROJECT SITE, OR DURING PLACEMENT UNLESS APPROVED BY ENGINEER.
9. PROTECT CONCRETE FROM PHYSICAL DAMAGE OR REDUCED STRENGTH DUE TO WEATHER EXTREMES DURING MIXING, PLACING, AND CURING.
10. SLAB FINISHES: TROWELED FINISH FOR FLOOR SURFACES TO RECEIVE FLOOR COVERINGS, PAINT, OR OTHER THIN FILM-FINISH COATINGS. SPECIFIED OVERALL VALUES OF FLATNESS, F(F) 35; AND LEVELNESS, F(L) 25; WITH MINIMUM LOCAL VALUES OF FLATNESS, F(F) 24; AND LEVELNESS, F(L) 17. NONSLIP BROOM FINISH TO EXTERIOR CONCRETE PLATFORMS, STEPS, AND RAMPS.
11. FORM 1/8" WIDE CONTRACTION JOINTS WITH POWER SAWS WHEN CUTTING ACTION WILL NOT TEAR, ABRADE OR OTHERWISE DAMAGE SURFACE AND BEFORE CONCRETE DEVELOPS RANDOM CONTRACTION JOINTS. SEE DETAILS FOR ADDITIONAL INFO.
12. BEGIN CURING UNFORMED CONCRETE AFTER FINISHING. KEEP CONCRETE CONTINUOUSLY MOIST FOR AT LEAST 7 DAYS OR APPLY MEMBRANE-FORMING CURING COMPOUND TO CONCRETE. CONTRACTOR SHALL VERIFY COMPOUND IS COMPATIBLE WITH FLOOR COVERING/COATINGS.
13. OWNER SHALL ENGAGE A TESTING AGENCY TO PERFORM TESTS AND TO SUBMIT TEST REPORTS.
14. PROTECT CONCRETE FROM DAMAGE. REPAIR SURFACE DEFECTS IN CONCRETE.
15. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHEN SPECIFIED ON DRAWINGS UNLESS APPROVED OTHERWISE IN WRITING BY THE ENGINEER OF RECORD. HOLES SHALL BE PREPARED AND ANCHORS SHALL BE INSTALLED FOLLOWING THE MANUFACTURER'S INSTALLATION INSTRUCTIONS. UNLESS SPECIFIED OTHER- WISE, ANCHORS SHALL BE EMBEDDED A MINIMUM OF 8 TIMES THE DIAMETER. U.N.O., ACRYLIC BASED ADHESIVES SHALL BE USED WHEN BASE MATERIAL TEMPERATURES DROP BELOW 40°F AND TRUE EPOXY BASED ADHESIVES ARE PREFERRED FOR USE IN OVERSIZED HOLES. ADHESIVE ANCHORS: ACCEPTABLE PRODUCTS AND MANUFACTURER ARE:
   A. EPCON S7 ADHESIVE ANCHORING SYSTEM
   B. HILTI HIT-HY 150 MAX ADHESIVE ANCHORING SYSTEM
   C. SIMPSON STRONG-TIE SET-XP ADHESIVE ANCHORING SYSTEM

STRUCTURAL STEEL

1. SUBMITTALS: SHOP DRAWINGS.
2. COMPLY WITH AISC 360-05 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325 OR A 490 BOLTS," AND AWS D1.1 "STRUCTURAL WELDING CODE--STEEL."
3. STRUCTURAL-STEEL WIDE FLANGE SHAPES: ASTM A992, GRADE 50. OTHER SHAPES, PLATES, AND BARS: ASTM A 36, CARBON STEEL.
4. COLD-FORMED STRUCTURAL-STEEL TUBING: ASTM A 500, GRADE B.
5. STEEL PIPE: ASTM A53, TYPE E, GRADE B.
6. ANCHOR RODS, BOLTS, NUTS: ASTM F1554, GRADE 55, UNHEADED RODS.
7. BOLTS, NUTS, AND WASHERS: ASTM A 325, TYPE 1, HIGH-STRENGTH HEAVY HEX STEEL STRUCTURAL BOLTS, HEAVY HEX CARBON-STEEL NUTS, AND HARDENED CARBON-STEEL WASHERS, UNCOATED.
8. PRIMER: LEAD- AND CHROMATE-FREE, NONASPHALTIC, RUST-INHIBITING PRIMER.
9. GROUT: ASTM C 1107, NONMETALLIC, SHRINKAGE RESISTANT, PREMIXED.
10. FABRICATE STRUCTURAL STEEL ACCORDING TO AISC SPECIFICATIONS AND TOLERANCE LIMITS OF AISC'S "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" FOR STRUCTURAL STEEL.
11. SHOP PRIMING: PREPARE SURFACES ACCORDING TO SSPC-SP 2 OR SSPC-SP 3. SHOP PRIME STEEL TO A DRY FILM THICKNESS OF AT LEAST 2.0 MILS. DO NOT PRIME SURFACES TO BE EMBEDDED IN CONCRETE OR MORTAR OR TO BE FIELD WELDED.
12. ERECT STRUCTURAL STEEL ACCORDING TO AISC SPECIFICATIONS AND WITHIN ERECTION TOLERANCES OF AISC'S "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
13. SET BASE AND BEARING PLATES ON WEDGES, SHIMS, OR SETTING NUTS. TIGHTEN ANCHOR BOLTS, CUT OFF WEDGES OR SHIMS FLUSH WITH EDGE OF PLATE, AND PACK GROUT SOLIDLY BETWEEN BEARING SURFACES AND PLATES.
14. BOLTED CONNECTIONS: SNUG TIGHTEN HIGH-STRENGTH BOLTS ACCORDING TO RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325 OR A 490 BOLTS."
15. UNLESS SPECIFICALLY NOTED, ALL BOLTED CONNECTIONS SHALL BE NON-SLIP CRITICAL USING 3/4" DIAMETER A325-N BOLTS WITH THREADS INCLUDED IN THE SHEAR PLANE. ALL CONNECTIONS SHALL BE CITED FROM TABLE 10-1, OR TABLE 10-2. AISC 360-05 CODE. UNLESS NOTED, CONNECTIONS SHALL DEVELOP AT LEAST ONE-HALF OF THE MAXIMUM TOTAL UNIFORM LOAD CAPACITY TABULATED IN THE MANUAL FOR THE GIVEN SHAPE AND SPAN OF THE BEAM. AS A MINIMUM, ALL SHEAR CONNECTIONS SHALL CONTAIN AT LEAST THE NUMBER OF ROWS OF 3/4" DIAMETER A325-N BOLTS (AT 3" PITCH) AS CAN BE FIT IN A CLIP ANGLE OF ONE-HALF THE BEAM T-DISTANCE IN LENGTH.
16. UNLESS SPECIFICALLY DETAILED, MOMENT CONNECTIONS SHALL BE DESIGNED BY A LICENSED ENGINEER FOR THE FORCES INDICATED ON THE DRAWINGS. IN CONFORMANCE WITH ALL APPLICABLE CODES.
17. ALL DECK EDGES SHALL HAVE CONTINUOUS SUPPORT. PROVIDE L4x4x1/4 CONTINUOUS SUPPORT ANGLE UNLESS NOTED OTHERWISE. WELD ANGLE TO EACH SUPPORT BEAM OR JOIST WITH 1/8"x2" FILLET WELDS, U.N.O. WHERE ANGLE ABUTS MASONRY WALL, ATTACH ANGLE TO WALL WITH 1/2" DIA. SLEEVE ANCHORS AT 24" O.C. U.N.O.
18. ALL STEEL BEAMS AND COLUMNS ADJACENT TO MASONRY SHALL HAVE ADJUSTABLE MASONRY TIES AT 24" O.C. UNLESS DETAILED OTHERWISE.

METAL-PLATE-CONNECTED HEAVY TIMBER TRUSSES

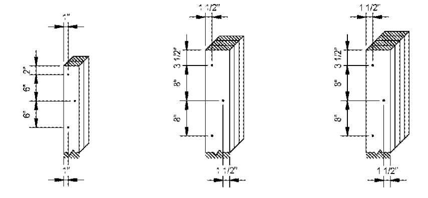
1. ENGINEER, FABRICATE, AND ERECT METAL-PLATE-CONNECTED WOOD TRUSSES TO WITHSTAND DESIGN LOADS WITHOUT EXCEEDING DEFLECTION LIMITS OF ANSI/TPI 1 "NATIONAL DESIGN STANDARD FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION."
2. IN ADDITION TO PRODUCT DATA, SUBMIT SHOP DRAWINGS AND STRUCTURAL ANALYSIS DATA, SIGNED AND SEALED BY A QUALIFIED PROFESSIONAL ENGINEER ENGAGED BY THE FABRICATOR WHO IS REGISTERED IN THE STATE WHERE PROJECT IS LOCATED.
3. ENGAGE A FABRICATOR WHO PARTICIPATES IN A RECOGNIZED QUALITY-ASSURANCE PROGRAM THAT INVOLVES INSPECTION BY AN INDEPENDENT INSPECTING AND TESTING AGENCY ACCEPTABLE TO AUTHORITIES HAVING JURISDICTION.
4. COMPLY WITH ANSI/TPI 1; TPI HIB, "COMMENTARY AND RECOMMENDATIONS FOR HANDLING INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES"; AND AFPA'S "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" AND ITS "SUPPLEMENT."
5. DIMENSION LUMBER: COMPLY WITH DOC PS 20, "AMERICAN SOFTWOOD LUMBER STANDARD," ANY SPECIES, GRADED VISUALLY OR MECHANICALLY. ALL MEMBERS SHALL BE NO. 2 KD SOUTHERN YELLOW PINE (OR BETTER). TOP AND BOTTOM CHORDS SHALL BE 6x12 MINIMUM.
6. FASTENERS: 3/4" DIAMETER THRU BOLTS (HD GALV)
7. CONNECTOR PLATES: 1/4" ASTM A 36 STEEL PLATE HOT DIP GALVANIZED, WHERE EXPOSED TO WEATHER OR TO HIGH RELATIVE HUMIDITIES. SIZE AND TYPE INDICATED.
8. METAL FRAMING ANCHORS: MANUFACTURED FROM HOT-DIP, ZINC-COATED STEEL SHEET COMPLYING WITH ASTM A 653, G60 (ASTM A 653M, Z180) COATING DESIGNATION; STRUCTURAL, COMMERCIAL, OR LOCK- FORMING QUALITY, AS STANDARD WITH MANUFACTURER FOR TYPE OF ANCHOR INDICATED.
9. FABRICATE WOOD TRUSSES WITHIN MANUFACTURING TOLERANCES OF ANSI/TPI 1 AND CONNECT TRUSS MEMBERS BY METAL CONNECTOR PLATES.
10. INSTALL AND BRACE TRUSSES ACCORDING TO RECOMMENDATIONS OF TPI. INSTALL PLUMB, SQUARE, AND TRUE TO LINE; AND SECURELY FASTEN TO SUPPORTING CONSTRUCTION.
11. ANCHOR ALL TRUSSES TO WOOD PLATE AS INDICATED IN SECTION 1/S403.
12. INSTALL AND FASTEN PERMANENT BRACING DURING TRUSS ERECTION. ANCHOR ENDS OF PERMANENT BRACING WHERE TERMINATING AT WALLS OR BEAMS.
13. INSTALL WOOD TRUSSES WITHIN INSTALLATION TOLERANCES OF ANSI/TPI 1.
14. DO NOT ALTER, CUT, OR REMOVE TRUSS MEMBERS.
15. REMOVE AND REPLACE WOOD TRUSSES THAT ARE DAMAGED OR DEFICIENT. IF TRUSSES TOPPLE OR DOMINO DURING ER[illegible]ON, DISCARD [illegible] DOMINOED TRUSSES AND REPLACE WITH NEW TRUSSES.
16. GENERAL CONTRACTOR SHALL COORDINATE ALL MECHANICAL EQUIPMENT W/ ROOF TRUSS MAN[illegible]SURE TRU[illegible] ARE PROPERLY FABRICATED TO ACCOMMODATE WEIGHTS AND REQUIRED ROOF OPENINGS.

ROUGH CARPENTRY

1. DRESSED LUMBER, S4S. 19 PERCENT MAXIMUM MOISTURE CONTENT FOR 2-INCH THICKNESS O[illegible] MARKE[illegible] GRADE STAMP OF INSPECTION AGENCY.
2. PRESERVATIVE-TREATED MATERIALS: AWPA C2 LUMBER AND AWPA C9 PLYWOOD, LABELED BY AN [illegible]CTION AGENCY APPROVED BY A[illegible] BOARD OF REVIEW. AFTER TREATMENT, KILN-DRY LUMBER AND PLYWOOD TO 19 AND 15 PERCENT M[illegible]RE CONTENT, RESPECTIVELY. TREAT INDICATED ITEMS AND THE FOLLOWING:
   A. WOOD MEMBERS IN CONNECTION WITH ROOFING, FLASHING, V[illegible] BARRIERS, AND WATERPR[illegible]
   B. CONCEALED MEMBERS IN CONTACT WITH MASONRY OR CONC[illegible]
3. CONCEALED BOARDS: 19 PERCENT MAXIMUM MOISTURE CONTE[illegible]
4. BEAMS & POSTS: SOUTHERN PINE NO. 2
5. RAFTERS: SPRUCE-PINE-FIR NO. 2
6. LVL: ALL LVL LUMBER [illegible] BE 1.75" WIDE EACH SI[illegible]ER AND F[illegible]0 PSI, E=1.9MPSI
7. MISCELLANEOUS [illegible] NO. 3 OR STANDA[illegible] GRADE [illegible] SPECIE[illegible] NAILERS, BLOCKING, AND SIMILAR MEMBERS.
8. WOOD-BASED STRUCT[illegible] USE PANELS: [illegible]OC PS 2. P[illegible]E PLYWOOD [illegible]PLYING WITH DOC PS 1, WHERE PLYWOOD IS INDICATED.
   A. FACTORY MARK[illegible]LS EVIDENC[illegible]G CO[illegible]IANCE WITH GRADE [illegible]NTS.
   B. PANELS WITH SPA[illegible]TINGS REQ[illegible] BY SUPPORT SP[illegible]ING IND[illegible]D.
   C. WALL SHEATH[illegible]A-RATED [illegible] PLYWOOD OR OS[illegible] SHEAT[illegible]NG, EXPOSURE 1. BLOCK ALL PANEL EDGES.
   D. FLOOR SH[illegible]THING: [illegible]A-RATED [illegible] PLYWOOD SH[illegible]THING, EXTERIOR. INSTALL SHEATHING WITH FACE GRAIN ACROSS SUPPO[illegible]S.
   E. ROOF S[illegible]ATHING: 5/8", AP[illegible]ED CDX [illegible]EATHING, EXTERIOR. INSTALL SHEATHING WITH FACE GRAIN ACROSS SUPPORTS. [illegible]ROVID[illegible] CLIPS AT MID S[illegible] PANELS. BLOCK ALL PANELS EDGES. SEE DETAIL D/S0.01 FOR ATTACHMENT.
9. FIT [illegible]GH CAR[illegible]RY TO OTHER CO[illegible]ON; SCRIBE AND COPE FOR ACCURATE FIT. CORRELATE LOCATION OF FURRING, BLOCKING, AND [illegible]LAR SUPPOR[illegible] ALLOW ATTACH[illegible] OF OTHER CONSTRUCTION.
10. [illegible]VIDE A MINIMU[illegible] BEARING A[illegible]H END OF WOOD HEADERS U.N.O.
11. [illegible]RELY ATTACH R[illegible]RY WORK TO SUBSTRATE BY ANCHORING AND FASTENING AS INDICATED, COMPLYING WITH THE FO[illegible]ING:
   [illegible]TABLE 2304.9.1--FASTENING SCHEDULE" OF THE INTERNATIONAL BUILDING CODE.
12. ALL E[illegible]OR WALL STUDS TO BE 2x6 @ 16" O.C. U.N.O.

METAL-PLATE-CONNECTED WOOD TRUSSES

1. ENGINEER, FABRICATE, AND ERECT METAL-PLATE-CONNECTED WOOD TRUSSES TO WITHSTAND DESIGN LOADS WITHOUT EXCEEDING DEFLECTION LIMITS OF ANSI/TPI 1, "NATIONAL DESIGN STANDARD FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION."
2. IN ADDITION TO PRODUCT DATA, SUBMIT SHOP DRAWINGS AND STRUCTURAL ANALYSIS DATA, SIGNED AND SEALED BY A QUALIFIED PROFESSIONAL ENGINEER ENGAGED BY THE FABRICATOR WHO IS REGISTERED IN THE STATE WHERE PROJECT IS LOCATED.
3. ENGAGE A FABRICATOR WHO PARTICIPATES IN A RECOGNIZED QUALITY-ASSURANCE PROGRAM THAT INVOLVES INSPECTION BY AN INDEPENDENT INSPECTING AND TESTING AGENCY ACCEPTABLE TO AUTHORITIES HAVING JURISDICTION.
4. COMPLY WITH ANSI/TPI 1: TPI HIB, "COMMENTARY AND RECOMMENDATIONS FOR HANDLING INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES"; AND AFPA'S "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" AND ITS "SUPPLEMENT."
5. DIMENSION LUMBER: COMPLY WITH DOC PS 20, "AMERICAN SOFTWOOD LUMBER STANDARD." ANY SPECIES, GRADED VISUALLY OR MECHANICALLY. ALL MEMBERS SHALL BE NO. 2 KD SOUTHERN YELLOW PINE (OR BETTER). TOP AND BOTTOM CHORDS SHALL BE 2x6 MINIMUM.
6. CONNECTOR PLATES: STRUCTURAL-QUALITY STEEL SHEET, ZINC COATED, COMPLYING WITH ASTM A 653, GRADE 33, G60 (ASTM A 653M, GRADE 230, Z180) COATING DESIGNATION; AT LEAST 0.0359 INCH (0.91 MM) THICK.
7. FASTENERS: HOT-DIP GALVANIZED PER ASTM A 153 OR STAINLESS STEEL TYPE 304 OR 316, WHERE EXPOSED TO WEATHER OR TO HIGH RELATIVE HUMIDITIES. SIZE AND TYPE INDICATED.
8. METAL FRAMING ANCHORS: MANUFACTURED FROM HOT-DIP, ZINC-COATED STEEL SHEET COMPLYING WITH ASTM A 653, G60 (ASTM A 653M, Z180) COATING DESIGNATION; STRUCTURAL, COMMERCIAL, OR LOCK- FORMING QUALITY, AS STANDARD WITH MANUFACTURER FOR TYPE OF ANCHOR INDICATED.
9. FABRICATE WOOD TRUSSES WITHIN MANUFACTURING TOLERANCES OF ANSI/TPI 1 AND CONNECT TRUSS MEMBERS BY METAL CONNECTOR PLATES.
10. INSTALL AND BRACE TRUSSES ACCORDING TO RECOMMENDATIONS OF TPI. SPACE TRUSSES AT 24" O.C. MAX; INSTALL PLUMB, SQUARE, AND TRUE TO LINE; AND SECURELY FASTEN TO SUPPORTING CONSTRUCTION.
11. ANCHOR ALL TRUSSES TO WOOD PLATE WITH SIMPSON H2.5A HURRICANE TIES UNLESS NOTED OTHERWISE. ANCHOR ALL GIRDER TRUSSES TO WOOD PLATE AND STUDS WITH SIMPSON VGT/HDU4 TIEDOWNS UNLESS NOTED OTHERWISE.
12. INSTALL AND FASTEN PERMANENT BRACING DURING TRUSS ERECTION. ANCHOR ENDS OF PERMANENT BRACING WHERE TERMINATING AT WALLS OR BEAMS.
13. INSTALL WOOD TRUSSES WITHIN INSTALLATION TOLERANCES OF ANSI/TPI 1.
14. DO NOT ALTER, CUT, OR REMOVE TRUSS MEMBERS.
15. REMOVE AND REPLACE WOOD TRUSSES THAT ARE DAMAGED OR DEFICIENT. IF TRUSSES TOPPLE OR DOMINO DURING ERECTION, DISCARD ALL DOMINOED TRUSSES AND REPLACE WITH NEW TRUSSES.
16. GENERAL CONTRACTOR SHALL COORDINATE ALL MECHANICAL EQUIPMENT W/ ROOF TRUSS MANUFACTURER TO ASSURE TRUSSES ARE PROPERLY FABRICATED TO ACCOMMODATE WEIGHTS AND REQUIRED ROOF OPENINGS.



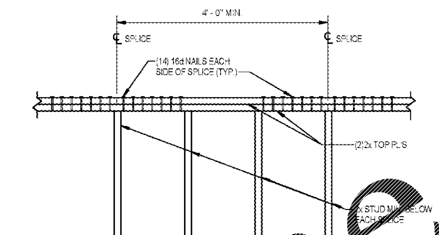
(2) 2X LAMINATIONS WITH (1) ROW OF STAGGERED 10d COMMON WIRE NAILS

(3) 2X LAMINATIONS WITH (1) ROW OF STAGGERED 30d COMMON WIRE NAILS

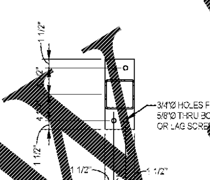
(4) 2X LAMINATIONS WITH (2) ROWS OF STAGGERED 30d COMMON WIRE NAILS (EACH SIDE OF POST)

BUILT-UP STUD COLUMN CONNECTION

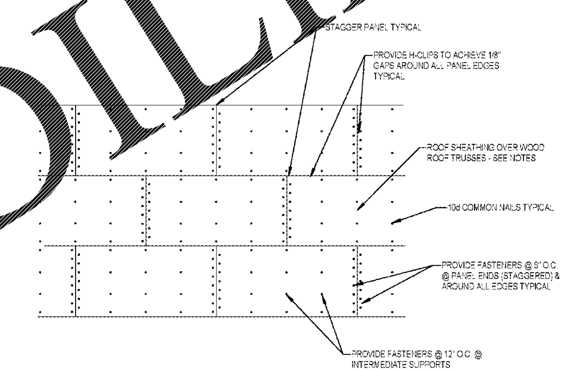
A / S0.01 BUILT UP COLUMN
NTS



B / S0.01 TOP PLATE SPLICE1
NTS



C / S0.01 TYP. COLUMN TO BEAM CAP PLATE
NTS



TYPICAL ROOF SHEATHING FASTENER DETAILS

D / S0.01 DETAIL
NTS

DESIGN LOAD CRITERIA

This analysis is made utilizing the International Building Code, 2015 edition.

ROOF TRUSS DESIGN LOADS		
Type	Dead Load	Live Load
Top Chord	10 psf	20 psf
Bottom Chord	15 psf	0 psf
Overframe Load	5 psf	0 psf
Uplift Load (Net)	16 psf	
SNOW LOADS		
Ground Snow Load, Pg	10 psf	
Exposure Factor, Ce	0.9	
Thermal Factor, Ct	1.1	
Importance Factor, Is	1.0	
Rain on Snow Surcharge	5 psf	
Flat Roof Snow Load	7 psf + Rain on Snow	
Min. Snow Load for Low Slope Roof, Pm	12 psf	
WIND LOADS		
Basic Wind Speed (3 - Second Gust), Vult	126 mph	
Exposure Category	C	
Internal Pressure Coefficient	±0.18	
Risk Category	II	
Height & Exposure Adjustment, λ	1.38	
Wind Directionality Factor, Kd	0.85	
Topographic Factor, Kzt	1.0	

STRENGTH LEVEL WIND PRESSURES			
COMPONENTS AND CLADDING			
ROOF (AREA = 100 SF)			
ZONE 1		ZONE 2	
+ 32.1 psf	- 32.1 psf	+ 32.1 psf	- 38.6 psf
ZONE 3			
+ 32.1 psf	- 38.6 psf		
WALL (AREA = 40 SF)			
ZONE 5		ZONE 5	
+ 35.3 psf	- 38.6 psf	+ 35.3 psf	- 45.2 psf

SEISMIC LOADS	
Importance Factor I	1.0
Risk Category	II
Site Class	D
Ss	0.675 g
S1	0.217 g
Sds	0.567 g
Sd1	0.284 g
Design Category	D
Basic System	Bearing Wall
Resisting System	Light Framed (wood) Walls Sheathed With Wood Structural Panels Rated For Shear Resistance
Seismic Response Coefficient (Cs)	0.09
Response Modification Factor (R)	6.5 K
Base Shear	V=Cs*W
Analysis Procedure	Equivalent Lateral Force

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SEAL

1890 EXTENSION SC STATE UNIVERSITY
CAMP HARRY G. DANIELS 1890 EXTENSION FACILITY CONSTRUCTION
Agency Review ID: H24-9649-CA
116 Camp Daniels Road, Elloree, SC

SHEET ISSUE:

NO.	DATE	DESCRIPTION	BY
1	2016.01.15	OSE schematic Design Submitt	WTF
2	2016.04.25	Design Development Review	WTF
3	2016.12.28	OSE Construction Documents	WTF
4	2017.02.10	OSE CD Review Response	WTF
5	2017.04.17	OSE CD REVIEW	WTF
6	2017.06.08	OSE BD DRAWINGS	WTF

OSE BD DRAWINGS 2017.06.08

PRINCIPAL IN CHARGE: WTF
PROJECT ENGINEER: MWM
DRAWN BY: MEW

SHEET TITLE:
GENERAL NOTES AND DESIGN CRITERIA

SHEET NO. S0.01
PROJ. NO. 13140.00

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