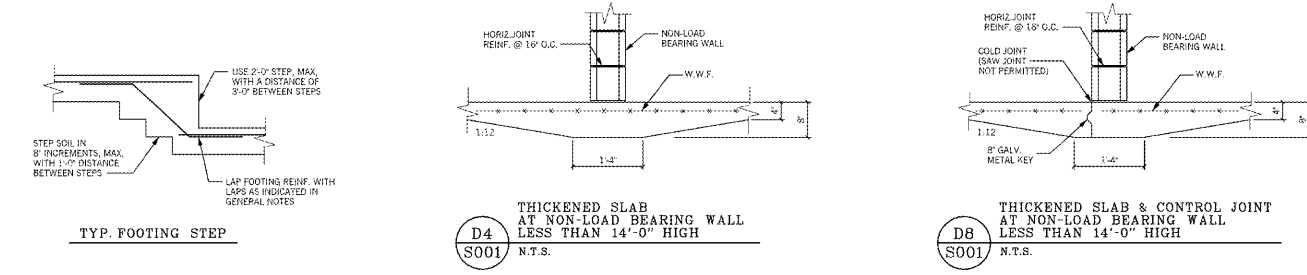


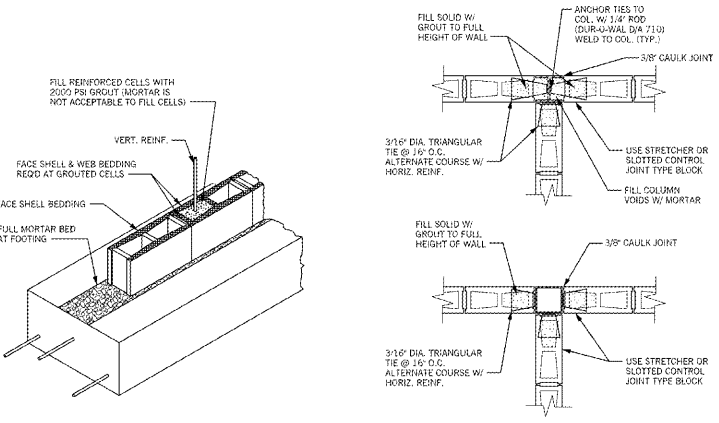
TYP. CONTROL JOINTS



TYP. FOOTING STEP

THICKENED SLAB AT NON-LOAD BEARING WALL LESS THAN 14'-0" HIGH (D4 S001 N.T.S.)

THICKENED SLAB & CONTROL JOINT AT NON-LOAD BEARING WALL LESS THAN 14'-0" HIGH (D8 S001 N.T.S.)



TYP. MORTAR REQUIREMENTS

STANDARD MASONRY ANCHORS AT WALLS ABUTTING COLUMNS

U-BLOCK LINTELS (NON-LOAD BEARING WALLS)	
MAX SPAN	8" LIGHT WEIGHT BLOCK
6'-0"	1-#4
6'-0"	1-#4
6'-0"	1-#4
6'-0"	1-#4

STEEL LINTELS (NON-LOAD BEARING WALLS)	
MAX SPAN	PER 4" THICKNESS
3'-0"	1-#4
3'-0"	1-#4
3'-0"	1-#4
3'-0"	1-#4

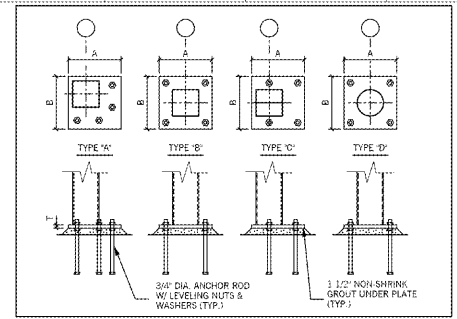
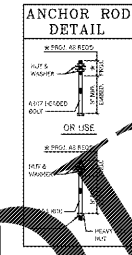


PLATE MARK	DIMENSIONS	PLATE TYPE	COLOR KEY
BP-1	14" x 14" x 3/4"	A	HSSB&S
BP-2	12" x 12" x 3/4"	A	HSSB&S
BP-3	12" x 12" x 3/4"	B	HSSB&S
BP-4	9" x 12" x 3/4"	C	HSSB&S
BP-5	11" x 14" x 3/4"	C	HSSB&S
BP-6	11" x 11" x 3/4"	A	HSSB&S



MARK	DIMENSIONS	REINFORCEMENT
F-6	6'-0" x 6'-0" x 2'-4"	SEE K9/S3
F-8	8'-0" x 8'-0" x 2'-4"	SEE F18/S301
F-12	12'-0" x 10'-0" x 2'-4"	SEE F16/S301

MARK	BEAM SIZE	PLATE SIZE
L-1	WB40	12" x 12"
L-2	WB40	12" x 12"
L-3	WB15	12" x 12"
L-4	WB18	12" x 12"
L-5	WB18	30" x 30"
L-6	HSS60x14	NO PLATE

GENERAL NOTES

THESE NOTES COMPLEMENT THE DRAWINGS AND SPECS AND SHOULD NOT BE CONSIDERED INCLUSIVE OF ALL ITEMS. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE OR SIMILAR CONDITIONS, EVEN THOUGH NOT SPECIFICALLY SHOWN ON THE DRAWINGS. CONTRACTOR SHALL PROVIDE ADEQUATE BRACING OR SHORING FOR ALL WORK DURING CONSTRUCTION. ALL WALLS RELYING ON TOP SLAB FOR SUPPORT SHALL BE BRACED UNTIL SLAB HAS CURED FOR A MINIMUM OF THREE DAYS. DRAWINGS ARE TO BE CAREFULLY COORDINATED AND LINTELS ACCORDING TO THE STANDARD Lintel SCHEDULE ARE TO BE PROVIDED AT ALL MASONRY OPENINGS SHOWN ON ARCHITECTURAL, MECHANICAL, ELECTRICAL AND STRUCTURAL PLANS. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE EXECUTING WORK. SHOP DRAWINGS SHALL BE INDEPENDENTLY DEVELOPED REPRODUCTION OF STRUCTURAL DESIGN DRAWINGS FOR USE AS SHOP DRAWINGS IS PROHIBITED. STEEL FABRICATION DRAWINGS SHALL BE SEALED BY AN ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

APPLICABLE CODES FOR DESIGN: INTERVENTIONAL BUILDING CODE (IBC) 2012, WITH 2012 CORRECTIONS; AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) 13TH EDITION; AMERICAN CONCRETE INSTITUTE (ACI) 308R-11; BUILDING CODE REQUIREMENTS FOR CONCRETE (ACI 318M-11); SEISMIC DESIGN CATEGORY: C; SEISMIC DESIGN CATEGORY: C; WIND FACTOR: 0.9; DESIGN LOADS: FLOOR LIVE LOAD: 80 PSF (REDUCED AS ALLOWED); ROOF LIVE LOAD: 20 PSF; WIND LOAD: AS PER ASCE 7-10; WIND SPEED: 110 MPH (SECOND GUST FACTORED LOAD); EXPOSURE: B; WIND PRESSURE COEFFICIENTS: +0.18, -0.18; WIND FORCE EFFECTS FOR VELOCITY PRESSURE FOR CLADDING DESIGN: SEE DESIGN NOTES; SEISMIC SITE CLASS: D; SEISMIC DESIGN CATEGORY: C; SEISMIC BASE SHEAR: V = 0.144 W; SEISMIC FORCES DETERMINED BY EQUIVALENT LATERAL FORCE PROCEDURE; SEISMIC RESISTING SYSTEM: 1. STRUCTURAL STEEL SYSTEMS (NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE (R = 3, C2 = 3))

FOUNDATIONS: IF AFTER EXAMINATION, THE CONDITION OF SOIL INDICATES A SAFE BEARING CAPACITY OF LESS THAN 2000 PSF, THE ARCHITECT/ENGINEER SHALL BE NOTIFIED PRIOR TO PLACEMENT OF ANY FOUNDATION. FOUNDATIONS AND RETAINING WALLS HAVE BEEN DESIGNED FOR ACTIVE PRESSURE OF 40 PSF, AT REST PRESSURE OF 80 PSF, PASSIVE PRESSURE OF 300 PSF, AND A COEFFICIENT OF FRICTION OF 0.50. THESE VALUES ARE BASED ON SOIL REPORTS AND TYPICAL VALUES FOR SOILS IN THIS AREA. ALL BACKFILL SHALL BE COMPACTED TO THE REQUIREMENTS OF THE EARTHWORK SPECIFICATION.

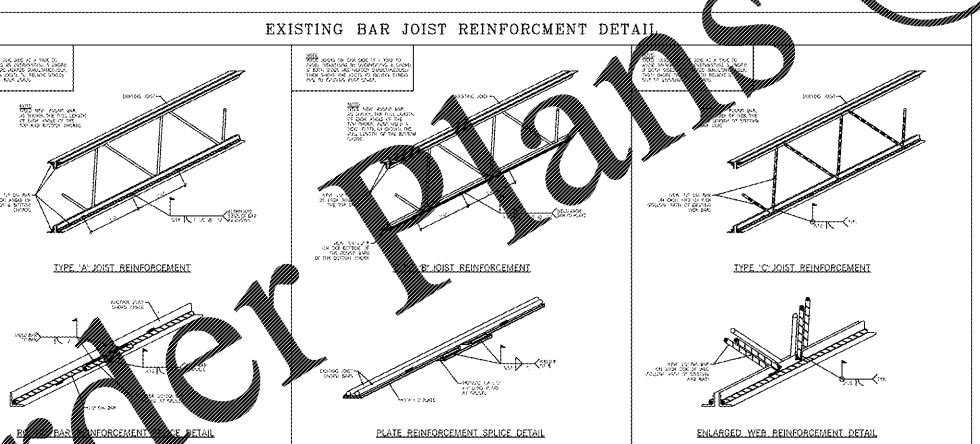
CONCRETE & REINFORCING: ALL CONCRETE TO HAVE A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI WITH A MAXIMUM AGGREGATE SIZE OF 3" UNLESS NOTED OTHERWISE ON PLANS. CONSTRUCTION OF CONTROL JOINTS SHALL BE PROVIDED IN FLOOR SLABS ON GRADE SUCH THAT THE MAXIMUM SPAN BETWEEN JOINTS IS 12'-0" OR AS NOTED ON THE DRAWINGS. PROVIDE #4 BARS 4'-0" LONG AT THE END OF SLAB AT ALL RE-ENTRANT CORNERS AND INTERSECTIONS AND AT ALL DISCONTINUOUS CONTROL JOINTS IN SLAB ON GRADE. ALL REINFORCING BARS SHALL BE ASTM A615, GRADE 60. ALL BAR SPLICES SHALL BE AT A MINIMUM THE FOLLOWING LENGTHS UNLESS NOTED OTHERWISE ON THE DRAWINGS: 36" FOR #3 BARS, 36" FOR #4 BARS, AND 24" FOR #5 BARS. CORNER BARS ARE TO BE PROVIDED AT ALL CORNERS AND INTERSECTIONS OF REINFORCEMENT. EXTEND WALL FOOTING REINFORCING BARS CONTINUOUSLY THROUGH COLUMN FOOTINGS.

CONCRETE MASONRY WALLS: THE DESIGN COMPRESSIVE STRENGTH,  $f_m$ , IS 1500 PSI. GROUT FOR MASONRY FILL SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI OR 25% DRY WEIGHT COMPACTED WITH PROPORTIONS BY VOLUME OF 1 PART PORTLAND CEMENT TO 3 PARTS FINE AGGREGATE. ALL MASONRY WALLS SHALL HAVE HORIZONTAL REINFORCEMENT AT 16" O.C. VERTICALLY. ALL MORTAR FOR CONCRETE MASONRY UNITS SHALL BE TYPE S. REINFORCING BARS AS SHOWN ON DRAWINGS SHALL BE TYPE 60. UNLESS NOTED OTHERWISE, ALL BARS SHALL BE AT A MINIMUM THE FOLLOWING LENGTHS: 36" FOR #3 BARS, 36" FOR #4 BARS, AND 24" FOR #5 BARS. IN PLACE OF LAP SPLICES, ANY OF THE FOLLOWING METHODS MAY BE USED: 1. LAP SPLICES WITH MECHANICAL COUPLERS (EPLERS-42); 2. BAR LOCK COUPLERS (BAR-LOCK COUPLERS); 3. DOUBLE BARREL WELDED SPLICES (WELDED SPLICES); 4. WELDED THROUGH REBAR SPLICES (WELDED THROUGH REBAR SPLICES). ALL METHODS MUST BE APPROVED BY THE ENGINEER OF RECORD.

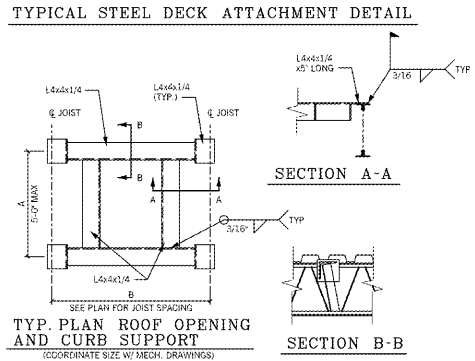
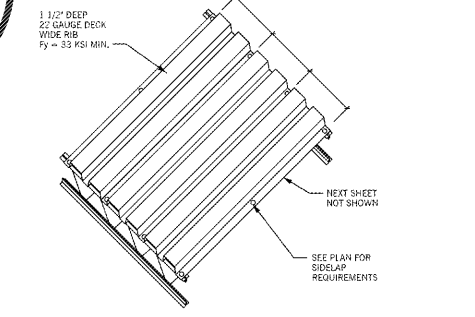
STEEL: ALL WELDED STRUCTURAL STEEL TO HAVE A MINIMUM YIELD STRENGTH OF 50 KSI (ASTM A992), UNLESS NOTED. STRUCTURAL STEEL ARE TO HAVE A MINIMUM WELD STRENGTH OF 45 KSI (ASTM A572). ANGLES AND CHANNELS SHALL BE ASTM A36. ALL WELDS ARE TO BE MADE WITH E70XX ELECTRODES. ALL CONNECTIONS, EXCEPT THOSE INDICATED ON THE DRAWINGS AS "WELDED CONNECTIONS," ARE TO BE MADE USING 3/4" DIA. AISC BOX JOINT CONNECTIONS DESIGNATED AS "CRITICAL" SHALL BE MADE USING LOAD-INDICATING WASHERS OR TENSION-CONTROL BOLTS. DESIGN ALL CONNECTIONS FOR ASD (UNFACTORED) REACTIONS SHOWN ON PLANS.

METAL DECK: ROOF METAL DECKING SHALL BE 1/2" x 22 GA. WIDE RIB WITH  $F_y = 53.0$  KSI. AT THE ROOF DECK ATTACHMENT, IN LIEU OF THE 5/8" PUDDLE WELDS, THE METAL DECK FASTENING SYSTEM CAN BE USED WITH A MIN. 1.0 L15 PIN. ONE ADDITIONAL SIDE LAP PER SPAN MUST BE INSTALLED WITH THE FASTENING SYSTEM IS USED. SEE PLANS FOR THE NUMBER OF SIDELAPS REQUIRED WHEN THE DECKING IS ATTACHED USING THE 5/8" PUDDLE WELD. FLOOR COMPOSITE METAL DECKING SHALL BE 1/2" x 22 GA. MEETING THE FOLLOWING CRITERIA:  $f_y = 53.0$  KSI,  $f_u = 68.0$  KSI. AT THE COMPOSITE FLOOR DECK ATTACHMENT, IN LIEU OF THE 5/8" PUDDLE WELDS, THE METAL DECK FASTENING SYSTEM CAN BE USED WITH A MIN. 1.0 L15 PIN AS AN EQUIVALENT (NO ADDITIONAL SIDELAPS REQUIRED) ATTACHMENT TO THE 5/8" PUDDLE WELD.

SPECIAL INSPECTIONS: SEE SPECIFICATIONS SECTION 04100 FOR REQUIREMENTS.



Order Plans @



NOTE: WHERE CURB SUPPORT IS LOCATED BETWEEN TOP CHORD PANEL POINTS ON JOIST, ADD L2x2x1/4 WELDED TO TOP CHORD & NEAREST BOTTOM CHORD PANEL POINT.

**Southern A&E** REGISTERED ARCHITECTS & ENGINEERS  
 7951 Iron Circle, Austell, Ga. 30168  
 (770) 819-7777  
 DATE: February 28, 2018  
 JOB NUMBER: 01-647-061  
 FACILITY CODE NUMBER: 664-1050  
 RENOVATIONS TO:  
**BELWOOD ELEMENTARY SCHOOL**  
 GORDON COUNTY BOARD OF EDUCATION  
 CALHOUN, GEORGIA  
 DRAWING NUMBER: S001