

GENERAL NOTES (CONT.)

4.00 CONCRETE MASONRY:

- 4.01 CONCRETE MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI 530-08, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES AND ACI 530J-08, SPECIFICATIONS FOR MASONRY CONSTRUCTION.
- 4.02 PROVIDE LIGHT WEIGHT, HOLLOW, LOAD-BEARING CONCRETE MASONRY UNITS (CMU) CONFORMING TO ASTM C90, GRADE N, TYPE II, UNLESS NOTED OTHERWISE.
- 4.03 PROVIDE CONCRETE MASONRY WITH MINIMUM COMPRESSIVE STRENGTH, $f_m = 2,000$ PSI, CORRESPONDING TO UNIT STRENGTH OF 3,050 PSI ON NET CROSS-SECTIONAL AREA OF CMU DETERMINED IN ACCORDANCE WITH ASTM C140.
- 4.04 PROVIDE TYPE "S" MORTAR IN ACCORDANCE WITH ASTM C270, UNLESS NOTED OTHERWISE. MORTAR BED JOINTS SHALL NOT EXCEED 5/8 IN. THICKNESS.
- 4.05 PROVIDE GROUT FOR REINFORCED MASONRY IN ACCORDANCE WITH ASTM C416 WITH MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI UNLESS NOTED OTHERWISE.
- 4.06 PROVIDE HORIZONTAL JOINT REINFORCEMENT COMPLYING WITH ASTM A82, NO. 9 GAUGE OR HEAVIER, ZINC COATED, PLACED 1/2 INCHES ON CENTER UNLESS NOTED OTHERWISE.
- 4.07 PROVIDE MINIMUM 1" VERTICAL, GROUTED FULL HEIGHT, AT EACH SIDE OF OPENINGS AND WALL ENDS.
- 4.10 ALL VERTICAL REINFORCEMENT TO BE PLACED IN FULLY GROUTED CELLS. ALL BOND BEAMS TO BE FULLY GROUTED.
- 4.08 LAY MASONRY UNITS IN RUNNING BOND UNLESS NOTED OTHERWISE.

5.00 WOOD FRAMING:

- 5.01 ALL DIMENSIONAL LUMBER JOISTS, TRUSSES, AND PLYWOOD WEB JOISTS SHALL BE SUPPORTED WITH SIMPSON HANGERS, NOT LEDGERS OR BANDS. PROVIDE AND INSTALL GALVANIZED HANGERS, FRAMING ANCHORS, AND FASTENERS OF THE TYPE INDICATED. FASTENER TYPE AND QUANTITY SHALL BE AS NOTED BY THE MANUFACTURER'S TECHNICAL LITERATURE TO PROVIDE THE MAXIMUM CAPACITY UNLESS NOTED. NAILS SHALL BE FULLY DRIVEN IN ALL HOLES IN THE ANCHOR.
- 5.02 ROOF SHEATHING SHALL BE 15/32" APA SPAN-RATED STRUCTURAL PLYWOOD PANELS MANUFACTURED IN ACCORDANCE WITH VOLUNTARY PRODUCT STANDARD PS 1-01 STRUCTURAL PLYWOOD. ROOF DECKING PANELS SHALL BE APPLIED W/ LONG EDGE PERP. TO TRUSSES OR SUPPORT FRAMING MEMBERS. FASTEN SHEATHING PANELS TO SUPPORTING STRUCTURE WITH 8d x 2 1/2" LONG RING-SHANK NAILS ON 4" CENTERS AT PERIMETER OF ROOF DIAPHRAGM, AT 6" CENTERS AT ALL OTHER PANEL EDGES, AND ON 6" CENTERS IN THE FIELD AT INTERMEDIATE SUPPORTS.
- 5.03 FLOOR SHEATHING SHALL BE APA SPAN-RATED 3/4" TONGUE & GROOVE ORIENTED STRAND BOARD PANELS, GLUED AND NAILED. ADHESIVES SHALL CONFORM WITH PERFORMANCE SPECS APG-01 OR ASTM D3498. FASTER FLOOR SHEATHING PANELS TO SUPPORTING STRUCTURE WITH 0.13" DIA. X 2 3/8" LONG SMOOTH OR DEFORMED SHANK NAIL ON 6" AT ALL EDGES AND ON 12" IN THE FIELD AT INTERMEDIATE SUPPORTS.
- 5.04 UNLESS NOTED OTHERWISE, WALL SHEATHING SHALL BE 1/2" APA SPAN RATED ORIENTED STRAND BOARD STRUCTURAL SHEATHING MANUFACTURED IN ACCORDANCE WITH VOLUNTARY PRODUCT STANDARD PS-2 OR APA FRP-108 PERFORMANCE STANDARDS AND NAILED PER SHEAR WALL SCHEDULE.
- 5.05 ALLOWABLE DESIGN STRESSES FOR ENGINEERED LUMBER ARE AS FOLLOWS:
ALL USE MICROLAM LVL BEAMS PLACED OUTSIDE THE BUILDING ENVELOPE (OR UNIT) SHALL BE TREATED WITH TRUSS-JOIST WATERSHED RESIN-IMPREGNATED STABILITY OVERLAY. TREAT ALL SAILED ENDS OF EACH PLY WITH OVERLAY TREATMENT.
2.0E MICROLAM LVL ENGINEERED LUMBER: FOR BEAM ORIENTATION (UNTREATED, INTERIOR USE):
 $F_c = 2,500$ PSI F_c Perp = 150 PSI $F_b = 2,600$ PSI $F_v = 285$ PSI $E = 2,000,000$ PSI
2.0E PARALLAM PSL ENGINEERED LUMBER: FOR BEAM ORIENTATION (UNTREATED, INTERIOR USE):
 $F_c = 2,300$ PSI F_c Perp = 150 PSI $F_b = 2,300$ PSI $F_v = 230$ PSI $E = 2,000,000$ PSI
1.8E PARALLAM PSL ENGINEERED LUMBER: FOR COLUMN ORIENTATION (UNTREATED, INTERIOR USE):
 $F_c = 2,500$ PSI F_c Perp = 425 PSI $F_b = 2,400$ PSI $F_v = 190$ PSI $E = 1,800,000$ PSI
ALL ENGINEERED LUMBER PLACED OUTSIDE THE BUILDING ENVELOPE (OR UNIT) SHALL BE WOLMANIZED PRESERVATIVE TREATED (SERVICE LEVEL 2) PARALLAM PSL MATERIAL.
- 5.06 SEE LOAD BEARING WALL STUD SCHEDULE ON SHEET 613 OR VERTICAL WOOD STUD SPECIES, GRADE, AND SPACING.
- 5.07 USE PRESERVATIVE TREATED WOOD FOR ALL EXPOSED LUMBER AND SILL PLATES IN CONTACT WITH CONCRETE.
- 5.08 SEE STRUCTURAL LOAD BEARING WALL NOTES FOR MIN. SILL PLATE AND WALL SHEATHING FASTENING.
- 5.09 DECKS TO BE ATTACHED TO BUILDING WITH P.T. 2x10 BAND THRU- BOLTED TO TRUSSES WITH TWO (2) 1/2" ROUND CARRIAGE BOLTS AT EACH TRUSS, AND NAILED AT RATE OF FOUR (4) 1/2" NAILS PER FOOT TO TRUSS ENDS AND BANDS, UNLESS NOTED OTHERWISE.
- 5.10 THE NUMBER AND SIZE OF NAILS CONNECTING WOOD FRAMING MEMBERS SHALL NOT BE LESS THAN THAT STATED IN TABLE 2302.4.3 OF THE FLORIDA BUILDING CODE, 2012 EDITION. SEE SHEET 602.
- 5.11 ALL NAILS CONNECTING WOOD FRAMING MEMBERS SHALL BE COMMON NAILS, UNLESS OTHERWISE NOTED.
- 5.12 PREFABRICATED METAL CONNECTORS SHALL BE AS MANUFACTURED BY SIMPSON STRONG-TIE. BASE MATERIAL AND APPLIED GALVANIZED FINISH TO HANGERS, FASTENERS, AND NAILS SHALL BE AS FOLLOWS:
VERIFY ACTUAL RETENTION LEVEL FOR ALL FRAMING MATERIAL WITH THE WOOD TREATER.
AT ALL INTERIOR (ENCLOSED) CONDITIONS PROTECTED FROM WEATHER EXPOSURE, PROVIDE ZMAX GALVANIZED CONNECTORS WITH FASTENERS GALVANIZED PER ASTM A513.
AT ALL INTERIOR (ENCLOSED) CONDITIONS WHERE CONNECTORS ARE PLACED AGAINST WOOD WITH ACTUAL RETENTION LEVELS GREATER THAN 0.40 PCF FOR ACE, 0.41 PCF FOR CBA-A, OR 0.42 PCF CA-B (CONCRETE CONTACT), OR ANY TREATMENTS CONTAINING AMMONIA, PROVIDE TYPE 304 OR 316 STAINLESS STEEL CONNECTORS AND FASTENERS.
- 5.13 AS AN ALTERNATE TO THE SIMPSON HOLD DOWN HARDWARE SHOWN ON THE UPLIFT ANCHORAGE SCHEDULE, THE CONTRACTOR MAY PROVIDE HOLD DOWN SYSTEMS AS DESIGNED BY THE SYSTEMS CONTACT PHONE NO. 800-241-1616 OR QUICK TIE SYSTEMS, CONTACT PHONE NO. 804-281-0925, PROVIDE ENGINEERED SHOP DRAWINGS SHOWING ALL HOLD DOWN LOCATIONS AND CONNECTIONS FOR REVIEW.

6.00 PRE-ENGINEERED WOOD TRUSSES:

- 6.01 TRUSSES TO BE PLACED ON 2'-0" MAXIMUM CENTERS UNLESS SHOWN OTHERWISE. SMALLER SPACINGS MAY BE USED IF REQUIRED BY TRUSS DESIGNER. SEE PLANS FOR TRUSS LOCATIONS AND SPANS.
- 6.02 TRUSS SUPERIMPOSED DESIGN LOADS TO BE AS FOLLOWS:
ROOF: FLOOR: CORRIDORS AND BREEZEWAYS:
TOP CHORD LIVE LOAD 20 PSF 40 PSF SEE GENERAL NOTE 110
TOP CHORD DEAD LOAD 15 PSF 15 PSF PARTITIONS SEE DETAILS
BOTTOM CHORD DEAD LOAD 5 PSF 5 PSF 6 PSF
NET UPLIFT, ROOF SEE GENERAL NOTE 110
- 6.03 TRUSS MANUFACTURER TO PROVIDE END WALL TRUSSES AT EACH SIDE OF VAULTED CEILINGS, AT GABLE ENDS, AT BUILDING STEP DOWNS, AT ATTIC DRAFT STOPPING LOCATIONS, AND AT CHANGE OF ROOF LINES.
- 6.04 CONFIGURATION AND SIZE OF WEB MEMBERS TO BE DETERMINED BY TRUSS MANUFACTURER.
- 6.05 ENGINEERED SHOP DRAWINGS OF TRUSSES SHALL BE SUBMITTED FOR REVIEW AND APPROVAL BY ARCHITECT AND ENGINEER OF RECORD PRIOR TO FABRICATION. SHOP DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF ALABAMA.
- 6.06 TOP CHORD DEAD LOAD SHOWN ABOVE INCLUDES 3 PSF TRUSS SELF-WEIGHT.
- 6.07 MAXIMUM LIVE LOAD DEFLECTION SHALL BE SPAN/360 FOR FLOORS, SPAN/360 FOR ROOF.
- 6.08 TRUSS BRIDGING, TEMPORARY AND PERMANENT BRACING SHALL BE IN ACCORDANCE WITH BUILDING AND COMPONENT SAFETY INFORMATION (BCSI 1-03) THE GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, AND BRACING OF PLATE CONNECTED WOOD TRUSSES INTRODUCED BY THE WOOD TRUSS COUNCIL OF AMERICA AND TRUSS PLATE INSTITUTE.
- 6.09 TRUSSES SHALL BE DESIGNED FOR APPLICABLE WIND LOADS IN ACCORDANCE WITH FBC CHAPTER 16 AND ASCE/SEI 1-10 WIND DESIGN CRITERIA. APPLICABLE CODE PRESSURE FACTORS SHALL BE USED IN DETERMINING WIND LOADS AT TRUSS LOCATIONS.
- 6.10 THE FOLLOWING INFORMATION SHALL BE SHOWN ON THE TRUSS SHOP DRAWINGS. ALL TRUSS SHOP DRAWINGS, INCLUDING ERECTION STABILITY BRACING AND PERMANENT BRACING, SHALL BE AVAILABLE ON JOB SITE AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER:
A) SPECIES OF THE LUMBER USED TO FABRICATE ALL TRUSS TYPES.
B) NOMINAL DIMENSIONS OF ALL TRUSS MEMBERS.
C) UNIFORM LIVE AND DEAD LOAD MAGNITUDE, INCLUDING ALL CONCENTRATED LOAD MAGNITUDES (FROM COLUMNS, BEARING PARTITIONS, ETC.) AND THEIR LOCATION.
D) MAGNITUDE OF FORCES IN ALL MEMBERS FOR EACH CRITICAL LOAD CASE.
E) BRIDGING AND BRACING DETAILS AND LOCATIONS.
F) INTERMEDIATE AND END BEARING DETAILS AND OTHER DETAILS OF STRUCTURAL CONNECTIONS NOT ADDRESSED ON STRUCTURAL OR ARCHITECTURAL PLANS.
G) ERECTION PLANS IDENTIFYING INDIVIDUAL TRUSSES SHOWN AND DETAILED ON SHOP DRAWINGS.
H) ALL HARDWARE (BOLTS, HANGERS, STRAPS, ETC.) REQUIRED FOR CONNECTIONS BETWEEN TRUSSES AND OTHER ELEMENTS.

7.00 STRUCTURAL TESTS AND SPECIAL INSPECTIONS

THE OWNER SHALL HIRE AN INDEPENDENT APPROVED AGENCY TO PROVIDE INSPECTIONS DURING THE CONSTRUCTION OF THE TYPES OF WORK PER FBC 2010 CHAPTER 11 AND AS LISTED HEREIN. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED FOR ALL OTHER BUILDINGS IN SECTION 2010.

THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK OUTLINED BELOW FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS, AND KEEP RECORD OF INSPECTIONS, FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN CHARGE. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION AND NOT UNCOVERED TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE WORK PRIOR TO COMPLETION OF THAT PHASE OF THE WORK. A FINAL REPORT OF INSPECTIONS DOCUMENTS REQUIRED BY THESE INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF THE WORK.

A. STEEL CONSTRUCTION: SPECIAL INSPECTIONS FOR STEEL ELEMENTS OF THE BUILDING SHALL INCLUDE THE FOLLOWING:

1. PERIODIC MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS.
2. PERIODIC INSPECTION OF HIGH-STRENGTH BOLTED BEARING TYPE CONNECTIONS.
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL MATERIAL INCLUDING VERIFICATION OF CONFORMANCE TO ASTM STANDARDS SPECIFIED IN THE CONSTRUCTION DOCUMENTS, AND VERIFICATION OF MANUFACTURER'S CERTIFIED TEST REPORTS.
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS INCLUDING VERIFICATION OF CONFORMANCE TO AISC STANDARDS SPECIFIED IN THE CONSTRUCTION DOCUMENTS, AND VERIFICATION OF MANUFACTURER'S CERTIFICATE OF COMPLIANCE.
5. CONTINUOUS INSPECTION OF COMPLETE AND PARTIAL PENETRATION GROOVE WELDS, MULTI-PASS FILLET WELDS, AND SINGLE-PASS FILLET WELDS GREATER THAN 5/16".
4. PERIODIC INSPECTION OF SINGLE PASS WELDS LESS THAN 5/16", AND FLOOR AND DECK WELDS.
5. PERIODIC INSPECTION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A106, AND OTHER REINFORCING STEEL.
6. PERIODIC INSPECTION OF ANCHOR RODS AND EMBEDDED ITEMS.

B. CONCRETE CONSTRUCTION: SPECIAL INSPECTIONS AND VERIFICATIONS OF CONCRETE CONSTRUCTION SHALL INCLUDE THE FOLLOWING:

1. PERIODIC INSPECTION OF REINFORCING STEEL PLACEMENT.
2. CONTINUOUS INSPECTION OF ANCHOR RODS AND EMBED ASSEMBLIES TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE.
3. PERIODIC INSPECTION TO VERIFY USE OF REQUIRED CONCRETE DESIGN MIX.
4. CONTINUOUS INSPECTION OF SAMPLING OF FRESH CONCRETE AND PERFORMING SLUMP, AIR CONTENT AND DETERMINING THE TEMPERATURE OF FRESH CONCRETE AT THE TIME OF MAKING SPECIMENS FOR STRENGTH TESTS.
5. CONTINUOUS INSPECTION OF CONCRETE FOR PROPER APPLICATION TECHNIQUES.
6. PERIODIC INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.
7. PERIODIC VERIFICATION OF IN-SITU CONCRETE STRENGTH.

C. SOILS: SPECIAL INSPECTIONS FOR EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND LOAD BEARING REQUIREMENTS SHALL INCLUDE THE FOLLOWING. THE SOILS REPORT SHALL BE USED TO DETERMINE COMPLIANCE.

1. PRIOR TO PLACEMENT OF PREPARED FILL, DETERMINE THAT THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE APPROVED SOILS REPORT.
2. DURING PLACEMENT AND COMPACTION OF THE FILL MATERIAL DETERMINE THAT THE MATERIAL BEING USED AND THE MAXIMUM LIFT THICKNESS COMPLY WITH THE APPROVED SOILS REPORT, AS SPECIFIED IN SECTION 1803.4.
3. DETERMINE, AT THE APPROVED FREQUENCY, THAT THE IN-PLACE DRY DENSITY OF THE COMPACTED FILL COMPLIES WITH THE APPROVED SOILS REPORT.

D. WOOD AND PRE-ENGINEERED PLATE-CONNECTED WOOD TRUSS FRAMING

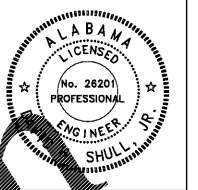
1. PERIODIC INSPECTION OF FRAMING MEMBER SIZES, SPACINGS, SPECIES, AND LOCATIONS.
2. PERIODIC INSPECTION OF CONNECTIONS OF MEMBERS.
3. PERIODIC INSPECTION OF WALL SHEATHING ATTACHMENT TO STUDS TO ENSURE COMPLIANCE WITH CODE REQUIREMENTS AND SHEARWALL NAILING SCHEDULE.
4. PERIODIC INSPECTION OF FLOOR SHEATHING ATTACHMENT TO FLOOR FRAMING MEMBERS TO ENSURE COMPLIANCE WITH CODE REQUIREMENTS AND NAILING SCHEDULE.
5. PERIODIC INSPECTION OF PLATE-CONNECTED WOOD TRUSS FRAMING TO ENSURE MEMBER SIZES, SPACINGS AND LOCATIONS, BRACING, AND METAL CONNECTIONS OF TRUSSES TO TRUSS COMPONENTS AND TRUSSES TO SUPPORTING FRAMING.

FASTENING SCHEDULE	
CONNECTION	FASTENER
DOUBLE STUDS	2-0.131" DIA. X 3 1/4" LONG # 9
WALL CORNERS AND WALL INTERSECTIONS	2-0.131" DIA. X 3 1/4" LONG # 9
MULTI-PLY BEAMS 2" TO 2" W/ 1/2" SPACER	SEE DETAIL 1/6/14
CONTINUOUS HEADER TO STUD (TOE-NAIL)	4-0.131" DIA. X 3 1/4" LONG NAILS # 9
DOUBLE TOP PLATES TO EACH OTHER (FACE NAIL)	0.131" DIA. X 3 1/4" LONG NAILS # 8, STAGGERED
TOP PLATE TO TOP PLATE - END JOINT (LAP SPLICE)	SEE DETAIL 6/6/14
TOP PLATE OVERLAP AT CORNERS AND INTERSECTIONS (FACE NAIL)	4-0.131" DIA. X 3 1/4" LONG NAILS # 9
TOP OR SOLE (BOTTOM) PLATE TO STUD (END NAIL)	3-0.131" DIA. X 3 1/4" LONG NAILS
STUD TO TOP OR SOLE (BOTTOM) PLATE (TOE-NAIL)	4-0.131" DIA. X 3 1/4" LONG NAILS

FASTENING SCHEDULE NOTES

1. FOR CONNECTIONS NOT NOTED OR FOR ARCHITECTURAL FINISH NAILING REQUIREMENTS SEE INTERNATIONAL BUILDING CODE, EDITION 2012, TABLE 2302.4.3.
2. FASTENING SCHEDULE IS BASED IN PART ON THE ICC-ES REPORT NO. ESR-1533, POWER-DRIVEN STAPLES AND NAILS, REISSUED ON JULY, 2015.

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HUNTSVILLE, ALABAMA
AN APARTMENT COMMUNITY FOR
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GENERAL NOTES

SHEET NO.

S0.2

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