

GENERAL NOTES

1.00 GENERAL:

- 1.01 ALL CONSTRUCTION SHALL CONFORM TO THE INTERNATIONAL BUILDING CODE, 2012 EDITION AND MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES ASCE/SEI 7-10. REFERENCE TO OTHER STANDARD SPECIFICATIONS OR CODES SHALL MEAN THE LATEST STANDARD OR CODE ADOPTED.
- 1.02 DRAWINGS SHOW TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. FOR DETAILS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS SIMILAR TO THOSE SHOWN.
- 1.03 VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS BEFORE STARTING WORK. NOTIFY STRUCTURAL ENGINEER OF ANY DISCREPANCY.
- 1.04 NOTIFY THE STRUCTURAL ENGINEER IN WRITING OF CONDITIONS ENCOUNTERED IN THE FIELD CONTRADICTORY TO THOSE SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS.
- 1.05 THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC.
- 1.06 COORDINATE STRUCTURAL CONTRACT DOCUMENTS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL. NOTIFY STRUCTURAL ENGINEER OF ANY CONFLICT OR OMISSION.
- 1.07 COORDINATE AND VERIFY FLOOR AND ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS. FOR ADDITIONAL OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE ARCHITECTURAL AND MECHANICAL DRAWINGS.
- 1.08 FOR DIMENSIONS NOT SHOWN SEE ARCHITECTURAL DRAWINGS.
- 1.09 REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. CONTRACTOR IS ALSO RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
- 1.10 DESIGN LIVE LOADS ARE AS FOLLOWS:
 

ROOF	20 PSF
PRIVATE ROOMS	40 PSF
SLEEPING AREAS	30 PSF
INHABITABLE ATTICS WITHOUT STORAGE	10 PSF
PRIVATE DECKS	40 PSF
STAIRS, BREEZEWAYS, AND CORRIDORS	100 PSF
PUBLIC ROOMS AND CORRIDORS SERVING THEM	100 PSF
CORRIDORS SERVING PRIVATE ROOMS	40 PSF
FLOOR DEAD LOAD	24 PSF
ROOF DEAD LOAD	10 PSF

WIND DESIGN CRITERIA:

WIND LOAD IS BASED ON AN ULTIMATE DESIGN WIND SPEED (3 SECOND WIND GUST),  $V_u$  OF 115 MILES PER HOUR. NOMINAL DESIGN WIND SPEED (3 SECOND WIND GUST),  $V_{50}$  IS 89 MILES PER HOUR.  
 RISK CATEGORY: II  
 SURFACE ROUGHNESS: B  
 WIND EXPOSURE CATEGORY: B  
 THE PROJECT IS NOT INCLUDED WITHIN THE WIND-BORNE DEBRIS REGION.  
 WIND PRESSURES FOR THE MUFRS MAIN WIND FORCE RESISTING SYSTEM MUFRS ARE DERIVED FROM ASCE/SEI 7-10 CHAPTER 28 (ENVELOPE PROCEDURE) METHOD 2 SECTION 28.6.  
 WIND PRESSURES FOR THE COMPONENTS AND CLADDING (C/C) ARE DERIVED FROM ASCE/SEI 7-10 CHAPTER 30 PART TWO: LOW-RISE BUILDINGS (SIMPLIFIED) SECTION 30.5.

- DESIGN WIND PRESSURES TO BE USED FOR THE DESIGN OF EXTERIOR COMPONENT AND CLADDING MATERIALS ARE SHOWN IN THE TABLE BELOW. REFERENCE ASCE/SEI 7-10 FIGURE 30.5-1 FOR APPLICATION OF DESIGN PRESSURES AND LOCATION OF ZONES FOR DESIGN PRESSURES.  
 INTERNAL DESIGN PRESSURES ARE INCLUDED IN TABULAR VALUES OF ASCE/SEI 7-10 FIGURE 30.5-1. NET DESIGN WIND PRESSURE AND ROOF OVERHANG NET WIND DESIGN PRESSURE. PRESSURES HAVE BEEN ADJUSTED FOR BUILDING HEIGHT AND EXPOSURE AS REQUIRED PER FIGURE 30.5-1.

EFFECTIVE WIND AREA	ROOF ZONES						WALL ZONES				ROOF OVERHANG	
	①	②	③	④	⑤	⑥	⑦	⑧	⑨	⑩	⑪	⑫
10.0	+15.3	-24.4	+15.3	-42.4	+15.3	-62.7	+26.6	-28.9	+26.6	-35.7	-44.7	-83.6
20.0	+14.0	-25.7	+14.0	-34.1	+14.0	-50.7	+25.4	-28.7	+25.4	-33.3	-44.7	-75.4
50.0	+12.2	-22.9	+12.2	-34.6	+12.2	-53.3	+23.9	-26.1	+23.9	-30.1	-44.7	-64.6
100.0	+10.9	-22.2	+10.9	-31.1	+10.9	-44.3	+22.6	-24.9	+22.6	-27.7	-44.7	-56.4

COMPONENTS AND CLADDING WIND PRESSURE TABLE NOTES:

WIND LOAD PRESSURES ARE IN POUNDS PER SQUARE FEET. AREA IS IN SQUARE FEET. PRESSURES ARE BASED ON TYPICAL ROOF SLOPE OF 26.6 DEGREES. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACES, RESPECTIVELY. EDGE STRIP DIMENSION  $a$  AS SHOWN IN ASCE 7-10 FIGURE 30.5-1 IS 14'-0".

1.00 GENERAL (CONT.):

- DESIGN WIND PRESSURES TO BE USED FOR THE DESIGN OF THE MAIN WIND FORCE RESISTING SYSTEM ARE SHOWN IN THE TABLE BELOW. REFERENCE ASCE/SEI 7-10 FIGURE 28.6-1 FOR APPLICATION OF MUFRS LOADS AND LOCATION OF ZONES FOR MUFRS LOADING.  
 DESIGN WIND PRESSURES HAVE BEEN DETERMINED BY ASCE/SEI 7-10 CHAPTER 28 WIND LOADS ON BUILDINGS-MUFRS (ENVELOPE PROCEDURE). PRESSURES HAVE BEEN MODIFIED FOR BUILDING HEIGHT AND EXPOSURE AS REQUIRED PER TABLE 28.6-1.

	HORIZONTAL PRESSURES				VERTICAL LOADS				WINDWARD OVERHANG	
	END ZONE		INTERIOR ZONE		END ZONE		INTERIOR ZONE		END ZONE	INTERIOR ZONE
LOAD DIRECTION	A HALL	B ROOF	C HALL	D ROOF	HINDWARD ROOF E	LEEWARD ROOF F	WINDWARD ROOF G	LEEWARD ROOF H	END ZONE	INTERIOR ZONE
LOAD CASE 1	+28.5	+9.0	+21.3	+7.9	-5.6	-17.2	-6.3	-14.2	-19.6	-17.5
LOAD CASE 2	--	--	--	--	+1.0	-14.7	+2.0	-6.0	--	--

MUFRS WIND PRESSURE TABLE NOTES:

WIND LOAD PRESSURES ARE IN POUNDS PER SQUARE FEET. PRESSURES ARE BASED ON A TYPICAL ROOF SLOPE OF 26.6 DEGREES. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACES, RESPECTIVELY. EDGE STRIP DIMENSION  $2a$  AS SHOWN IN FIGURE 28.6-1 IS 28'-0". WIND LOADS USED IN THE DESIGN OF THE MUFRS SHALL NOT BE LESS THAN A MINIMUM LOAD CASE, ASSUMING THE PRESSURES FOR ZONES A AND C ARE EQUAL TO 16 PSF, ZONES B AND D ARE EQUAL TO 48 PSF WHILE ASSUMING ALL VERTICAL PRESSURES ARE EQUAL TO 0 PSF, PER ASCE 7-10 SECTION 28.6.4.

SEISMIC DESIGN CRITERIA:

- SEISMIC RISK CATEGORY: II
- SEISMIC IMPORTANCE FACTOR: 1.0
- MAPPED SPECTRAL RESPONSE ACCELERATION, 0.2 SECONDS:  $S_a = 0.254 g$
- MAPPED SPECTRAL RESPONSE ACCELERATION, 1.0 SECONDS:  $S_d = 0.118 g$
- DESIGN SPECTRAL RESPONSE ACCELERATION, 0.2 SECONDS:  $S_d = 0.271 g$
- DESIGN SPECTRAL RESPONSE ACCELERATION, 1.0 SECONDS:  $S_d = 0.183 g$
- SITE CLASS (ASSUMED): C
- SEISMIC DESIGN CATEGORY: D
- BASIC STRUCTURAL SYSTEM: BEARING WALL SYSTEM

SEISMIC RESISTING SYSTEM:

- LIGHT FRAMED WALLS WITH SHEAR PANELS OF WOOD STRUCTURAL PANELS:  $R=6$ ,  $C_d=4$
- RESPONSE MODIFICATION COEFFICIENT
- DEFLECTION AMPLIFICATION FACTOR
- LIGHT FRAMED WALLS WITH SHEAR PANELS OF GYPSUM SHEATHING:  $R=2$ ,  $C_d=2$
- RESPONSE MODIFICATION COEFFICIENT
- DEFLECTION AMPLIFICATION FACTOR

- DESIGN BASE SHEAR: BUILDING NO. 1: 280.0 KIPS

- SEE REFERENCED SOILS REPORT FOR DISCUSSION OF SITE-SPECIFIC POTENTIAL LIQUEFACTION FOUNDATION AND GROUND MODIFICATION RECOMMENDATIONS. THE OWNER SHALL REVIEW THIS SECTION AND ACKNOWLEDGE THAT HE UNDERSTANDS THE POTENTIAL SETTLEMENT RISKS ASSOCIATED WITH THIS SITE.
- ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

SNOW DESIGN CRITERIA:

GROUND SNOW LOAD:  $P_g = 10$  (TEN) PSF

- FIELD VERIFY ALL EXISTING ABOVE AND BELOW GROUND CONDITIONS PRIOR TO FABRICATION AND CONSTRUCTION.

- THE STRUCTURAL DESIGN OF THE BUILDING IS BASED ON THE FULL INTERACTION OF ITS COMPONENT PARTS, WITH NO PROVISION FOR CONDITIONS OCCURRING DURING CONSTRUCTION THEREFORE THE CONTRACTOR SHALL PROVIDE ADEQUATE BRACING DURING CONSTRUCTION. PROVIDE TEMPORARY BRACING OF STRUCTURAL FRAMING UNTIL ALL PERMANENT BRACING, WALL SHEATHING, AND FLOOR AND ROOF DECKS (DIAPHRAGMS) ARE COMPLETELY INSTALLED AND ALL TRUSS, TRUSS ASSEMBLY AND POST/BEAM CONNECTIONS ARE COMPLETED.

- GENERAL CONTRACTOR SHALL COORDINATE ALL DETAILS RELATED TO WATERPROOFING, FLASHING, EDGE STRIPS, SURFACE SEALERS, AND SCHEDULED MAINTENANCE INSTRUCTIONS TO THE OWNER RELATED TO THE FINISHES AND ASSEMBLIES PLACED ON WOOD FRAMING.

HANDRAILS AND GENERAL LOADS:

HANDRAILS, GUARDS, AND GRAB BARS SHALL BE DESIGNED AND CONSTRUCTED TO THE FOLLOWING STRUCTURAL LOADING CONDITIONS PER IBC SECTION 1607.18:

HANDRAIL ASSEMBLIES AND GUARDS SHALL BE DESIGNED TO RESIST A LOAD OF 50 POUNDS PER LINEAR FOOT APPLIED IN ANY DIRECTION AT THE TOP AND TO TRANSFER THIS LOAD TO THE SUPPORTING STRUCTURE.

HANDRAIL ASSEMBLIES AND GUARDS SHALL BE ABLE TO RESIST A SINGLE CONCENTRATED LOAD OF 200 POUNDS, APPLIED IN ANY DIRECTION AT ANY POINT ALONG THE TOP, AND HAVE ATTACHMENT DEVICES TO THE SUPPORTING STRUCTURE TO TRANSFER THIS LOADING TO APPROPRIATE STRUCTURAL ELEMENTS OF THE BUILDING. THIS LOAD NEED NOT BE ASSUMED TO ACT CONCURRENTLY WITH THE LOAD SPECIFIED ABOVE.

INTERMEDIATE RAILS (ALL THOSE EXCEPT THE HANDRAILS) BALUSTERS AND PANEL FILLERS SHALL BE DESIGNED TO WITHSTAND A HORIZONTALLY APPLIED NORMAL LOAD OF 50 POUNDS ON AN AREA NOT TO EXCEED 1 SQUARE FOOT INCLUDING OPENINGS AND THE SPACE BETWEEN RAILS. REACTIONS DUE TO THIS LOADING ARE NOT REQUIRED TO BE SUPERIMPOSED WITH THE LOADS SPECIFIED ABOVE.

SHOP DRAWINGS FOR HANDRAILS AND GUARDS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL INDICATING REQUIRED MATERIALS, SIZES, LOCATIONS, AND FOR ALL POSTS AND PICKETS INCLUDING FASTENERS AND ANCHORAGE AT BASE OF POSTS, INTERMEDIATE AND END POINTS OF HANDRAILS.

2.00 FOUNDATIONS AND SLAB-ON-GROUND

- DESIGN OF FOUNDATIONS AND SLAB ON GRADE IS BASED ON A PRELIMINARY ASSUMED MAXIMUM BEARING CAPACITY OF 2500 PSF AND A MODULUS OF SUBGRADE REACTION OF 15 PCF. FOUNDATIONS WILL BE CONFIRMED AND MODIFIED AS REQUIRED BASED ON THE RECOMMENDATIONS OF THE FINAL GEOTECHNICAL SOILS INVESTIGATION CURRENTLY BEING PREPARED.

- THE OWNER'S GEOTECHNICAL ENGINEER SHALL VERIFY CONDITION AND ADEQUACY OF ALL SUB GRADES, FILLS AND BACK FILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, WALLS, FILLS, BACK FILLS, ETC.

- SIDES OF FOUNDATIONS SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING. FOUNDATIONS FORMED AGAINST THE EARTH REQUIRE THE FOLLOWING PRECAUTIONS: SLOPE SIDES OF EXCAVATIONS APPROVED BY GEOTECHNICAL ENGINEER AND CLEAN UP SLOUGHING BEFORE AND DURING CONCRETE PLACEMENT.

- WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL, UNLESS NOTED.

- DO NOT BACKFILL AGAINST FOUNDATION WALLS UNTIL THE WALL HAS REACHED 80% OF DESIGN STRENGTH AND TEMPORARY WALL BRACING HAS BEEN PLACED.

- UNLESS NOTED OTHERWISE, TYPICAL SLAB-ON-GROUND SHALL BE MINIMUM 4" THICK FRACED OVER VAPOR RETARDER OVER 6" COMPACTED LAYER OF FREE DRAINING, DEGRADABLE (CRUSHED STONE) AGGREGATE BASE ON COMPACTED SUBGRADE. REINFORCE SLAB WITH ONE LAYER OF #6x6-12" W/6" MIN. SPACING IN FLAT SHEETS (ROLLS NOT PERMITTED). PROVIDE POSITIVE SUPPORT TO CLEAR FROM BOTTOM OF SLAB. UNLESS OTHERWISE SHOWN ON THE DRAWINGS, PLACE CONTROL JOINTS AT COLUMN LINES AND AT INTERMEDIATE LINES SUCH THAT AREA OF EACH PANEL DOES NOT EXCEED 400'. CAREFULLY LOCATE CONSTRUCTION JOINTS AT CONTROL JOINTS, SUBJECT TO THE OWNER'S APPROVAL. SEE PLAN FOR GEOMETRIES OF CONTROL JOINTS.

- SOIL BELOW INTERIOR CONCRETE SLABS ON GRADE TO ANY FILL WITHIN 10'-0" OF BUILDING LIMIT SHALL BE COMPACTED TO 95% OF STANDARD PROCTOR. ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR CONTROLLED.

- SEE REFERENCED GEOTECHNICAL REPORTS AND CONSULT OWNER'S GEOTECHNICAL ENGINEER FOR REQUIRED SITE SPECIFIC GROUND MODIFICATION, SOIL REMEDIATION, GROUND IMPROVEMENT TECHNIQUES, PROTECTION OF SOILS FROM UNNECESSARY CONSTRUCTION TRAFFIC OR ACTIVITY, AND SPECIFIC FOUNDATION SUBGRADE SEPARATION REQUIREMENTS INCLUDING MATERIALS FOR SUB-FLOOR FILL, PLACEMENT OF WICK DRAINS AND REINFORCEMENT OF SOILS.

3.00 REINFORCED CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO ACI 301-05, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS. DESIGN IS BASED ON ACI 318-08, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.

- UNLESS NOTED OTHERWISE, ALL CONCRETE SHALL BE NORMAL WEIGHT AND HAVE THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTHS:

ISOLATED FOUNDATIONS AND WALLS	3500 PSI, NORMAL WEIGHT
SLAB-ON-GRADE	3500 PSI, NORMAL WEIGHT

- THE PROPOSED MATERIALS AND MIX DESIGNS SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE OWNER'S TESTING LABORATORY AND THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR OBTAINING THE REQUIRED DESIGN STRENGTH.

- USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.

- HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED ONLY WHERE INDICATED. THE LOCATION OF VERTICAL CONSTRUCTION JOINTS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER. CONSTRUCTION JOINTS SHALL BE THOROUGHLY ROUGHENED BY MECHANICAL MEANS AND CLEANED.

- CHAMFER OR ROUND ALL EXPOSED CORNERS MINIMUM 3/4".

- DETAIL CONCRETE REINFORCEMENT AND ACCESSORIES IN ACCORDANCE WITH ACI 318-99 DETAILING MANUAL. SUBMIT SHOP DRAWINGS FOR APPROVAL, SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.

- DETAIL ALL CONCRETE WALLS AND BEAMS ON THE SHOP DRAWINGS IN ELEVATION UNLESS SPECIFICALLY APPROVED OTHERWISE.

- REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60.

- WELDED WIRE FABRIC (MESH) SHALL CONFORM TO ASTM A185 AND SHALL BE PROVIDED IN FLAT SHEETS ONLY (ROLLS NOT PERMITTED). LAP ALL END AND CROSS SIDE LAPS ONE CROSS WIRE PLUS 2".

- TIE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. "STICKING" DOUELS INTO WET CONCRETE IS NOT PERMITTED.

- PROVIDE CONTINUOUS REINFORCEMENT WHERE POSSIBLE. SPLICE ONLY AS SHOWN OR APPROVED. USE TENSION SPLICE (CLASS "B") UNLESS NOTED OTHERWISE.

- REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:
 

CONCRETE CAST AGAINST EARTH (NOT FORMED)	3"
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THROUGH #8 BARS	2"
#9 BARS AND SMALLER	1-1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
SLABS AND WALLS	3/4"
BEAMS AND PIERS: TIES AND STIRRUPS	1/2"

- DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB THICKNESS WITHIN THE SLAB UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.

- DO NOT WELD OR TACK WELD REINFORCING STEEL.

- ALL REINFORCING STEEL PLACEMENT SHALL BE REVIEWED BY THE REGISTERED DESIGN PROFESSIONAL OR BY A REPRESENTATIVE RESPONSIBLE TO HIM. (RE: ACI 318-08, SECTION 13.1)

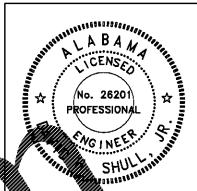
- PROVIDE FOR AN ALLOWANCE OF 1% OF REINFORCING BARS TO BE FABRICATED, AND PLACED DURING PROGRESS OF WORK AS MAY BE DIRECTED BY THE STRUCTURAL ENGINEER, IN ADDITION TO ALL THE STEEL INDICATED ON THE DRAWINGS. CREDIT ANY UNUSED QUANTITY AT THE END OF THE PROJECT TO THE OWNER.

- THE RESULTS OF ALL CONCRETE COMPRESSIVE TEST RESULTS SHALL BE PROMPTLY DISTRIBUTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. THE TEST RESULTS SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR.

REINFORCING BAR SPLICE LENGTH SCHEDULE

BAR SIZE, DIAMETER	#3	#4	#5	#6	#7	#8
TENSION SPLICE, CLASS A, 3500 PSI	16"	21"	26"	31"	45"	51"
TENSION SPLICE, CLASS B, 3500 PSI	20"	27"	33"	40"	58"	66"
COMPRESSION SPLICE	12"	15"	19"	23"	27"	30"

DAVID H. SHULL, JR., P.E.  
 ALABAMA REGISTRATION #: PE 24201  
 IN ASSOCIATION WITH:  
 SHULL & ASSOCIATES, INC.  
 ALABAMA CERTIFICATE OF AUTHORIZATION CA 3177 E  
 127 CHURCH STREET, NE, SUITE 100, MARIETTA, GA 30060  
 (404) 459-0585



PUCCIANO & ENGLISH, INC.  
 ARCHITECTS, AIA  
 3084 MERCER UNIVERSITY DRIVE, SUITE 110  
 ATLANTA, GA 30341  
 PH 770-457-0823 FAX 770-457-0092  
 denglish@pucciano-english.com



JOB No: 1701  
 DRAWN BY:  
 CHECK BY:  
 JOB PROGRESS:  
 ITEM DATE  
 PRICING SET 07-14-2017  
 PERMIT SET 02-26-2018

REVISIONS:  
 DATE NO

This drawing, as an instrument of service, is and shall remain the property of the Architect and shall not be reproduced, published or used in any way without the permission of the Architect.

Legacy at Jones Farm Phase 2  
 HUNTSVILLE, ALABAMA  
 AN APARTMENT COMMUNITY FOR  
 RBJ BAILEY, LLC

GENERAL NOTES

SHEET NO.

S0.1

Order Plans