STRUCTURAL NOTES

GENERAL

1. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
2. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
3. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
4. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
5. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. CONTRACTING OFFICER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE THE ARCHITECTURAL DRAWINGS.
6. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. CONTRACTING OFFICER SHALL BE NOTIFIED OF ANY DISCREPANCY.
7. CONTRACTOR SHALL VERIFY THAT MISCELLANEOUS FRAMING SHOWN ON THE STRUCTURAL DRAWINGS FOR MECHANICAL EQUIPMENT, OWNER-FURNISHED ITEMS, PARTITIONS, ETC. IS CONSISTENT WITH THE REQUIREMENTS OF SUCH ITEMS.
8. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, SAFETY, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
9. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR. CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTIBILITY ANALYSIS, AND ERECTION PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC.
10. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE CONTRACTING OFFICER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE CONTRACTING OFFICER. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE CONTRACTING OFFICER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
11. WHERE A SECTION OR DETAIL IS SHOWN OR DETAILED FOR ONE CONDITION, IT SHALL APPLY TO ALL SIMILAR AND LIKE CONDITIONS. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR.THE CONTRACTOR SHALL CONSIDER ALL OF THE CONTRACT DOCUMENTS IN DETERMINING SIMILAR AND LIKE CONDITIONS.
12. ENGINEER IS NOT RESPONSIBLE FOR THE DESIGN OF COLD-FORMED METAL FRAMING OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY OTHERS. FOR EACH SUCH SYSTEM CONTRACTOR SHALL PROVIDE FULLY ENGINEERED SHOP DRAWINGS SEALED BY THE NC REGISTERED PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION.

CODE/DESIGN CRITERIA

1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE NORTH CAROLINA BUILDING CODE, 2012 EDITION. (IBC 2009 edition)
2. GRAVITY LOADS
   - 2.1 UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
     - SLAB ON GRADE — 100 PSF
   - 2.2 UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
     - ROOF — 20 PSF
     - GROUND SNOW LOAD — 20 PSF
       - SNOW EXPOSURE FACTOR Ce = 1.0
       - THERMAL FACTOR Ct = 1.0
       - SNOW IMPORTANCE FACTOR Is = 1.0
     - PONDING AND DRIFT EFFECTS SHALL BE INCLUDED IN THE DESIGN.
   - 2.3 CONCENTRATED FLOOR LOADS – N.A.
   - 2.4 DEAD LOADS (IN ADDITION TO STRUCTURE SELF WEIGHT):
     - COLLATERAL LOADS — 6 p[illegible]
     - COLLATERAL LOADS COVER:
       - CEILING
       - LIGHTS
       - DUCTWORK
       - ELECTRICAL CONDUIT
     - SEE PLANS FOR OTHER ROO[illegible] [illegible]PORTED LOADS.
     - DO N[illegible]T ASSUME CO[illegible]ATERAL [illegible]ADS IN PLACE FOR [illegible]LIFT LOAD COM[illegible]ATIONS.
3. WIND LOAD[illegible]: A[illegible] 7-[illegible]
   - 90 [illegible]PH B[illegible] [illegible] SPEED
   - IMP[illegible]ANCE [illegible]OR Iw = 1.0
   - EXPO[illegible]E B
   - ENCLO[illegible] STRUCTURE
   - WIN[illegible]SE SHEARS:
     - Fx = XX.X KIPS
     - Fy = XX.X KIPS

CODE/DESIGN CRITERIA

4. EARTHQUAKE LOADS:

   SEISMIC RISK CATEGORY II
   SEISMIC IMPORTANCE FACTOR Ie = 1.00
   SPECTRAL RESPONSE COEFFICIENTS:
   - Ss = 27.6%
   - S1 = 9.2%
   - Sds = 22.1%
   - Sd1 = 10.4%

   SITE SOIL CLASSIFICATION: C
   SEISMIC DESIGN CATEGORY: B
   SEISMIC RESPONSE COEFFICIENT Cs: 0.074
   BASIC SEISMIC FORCE RESISTING SYSTEM:
   STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE *
   RESPONSE MODIFICATION COEFFICIENT R: 3.0
   SYSTEM OVERSTRENGTH FACTOR: 3.0
   DEFLECTION AMPLIFICATION FACTOR: 3.0
   ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

   SEISMIC BASE SHEARS:
   - Fx = XX.X KIPS
   - Fy = XX.X KIPS

* DO NOT DESIGN METAL BUILDING AS "ORDINARY MOMENT FRAME" AS THIS WILL INVOKE CERTAIN PROVISIONS OF AISC 341 AND RELATED INSPECTION REQUIREMENTS

FOUNDATION

1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY CATAWBA VALLEY ENGINEERING AND TESTING, P.C., CVET PROJECT * 17-508 DATED MARCH 28, 2017. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT FROM THOSE ASSUMED FOR DESIGN.
2. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.
3. INDIVIDUAL SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 3,000 PSF AND 3,000 PSF, RESPECTIVELY.
4. PROOFROLL BUILDING AREAS WITH TWO COMPLETE COVERAGES OF A LOADED TANDUM AXLE DUMP-TRUCK. REPLACE SOFT AREAS WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.
5. UNDERCUT ANY UNSUITABLE SOILS WITHIN THE AREA OF FOUNDATIONS PLUS A DISTANCE EQUAL TO THE DEPTH OF UNDERCUT BEYOND THE FOUNDATION PERIMETER. THE RESULTING EXCAVATION SHALL BE BACKFILLED WITH STRUCTURAL FILL AS REQUIRED B[illegible] THE SPECIFICATIONS.

REINFORCEMENT

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE [illegible] UNLESS N[illegible]TED OTHERWISE.
2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1[illegible] [illegible]D HAVE [illegible] S[illegible]E AND END LAPS OF 8".
3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DE[illegible]CT THE RE[illegible]RCI[illegible]G BAR [illegible]ES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORC[illegible]MENT WITHO[illegible] [illegible]DEQUATE S[illegible]TIONS, ELEVATIONS, AND DETAILS IS NOT A[illegible]EPTABLE.
4. PROVIDE DOWELS FROM FOUNDATIONS [illegible] SAME [illegible]ZE AND NUMBER A[illegible] THE VERTICAL WALL OR COLUMN REINFOR[illegible]G, UNLES[illegible] [illegible]ED O[illegible]HERWISE.
5. PLACE REINFORCEMENT AS F[illegible]WS, UNL[illegible]SS [illegible] OTHERWISE:
   - 5.1 CONCRETE REINFORCEMENT C[illegible]
     - BELOW GRADE: UNFORMED — 3" CLEAR
     - FORMED — 2" CLEAR
6. RE[illegible]FORCEMENT S[illegible]ALL BE SPLICED ONLY A[illegible] LOCATIONS SHOWN OR NOTED IN THE [illegible]TRUCTURAL [illegible]DOCU[illegible]ENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE [illegible]PLICED AT [illegible]OCATIO[illegible]S DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL [illegible]E A[illegible]ROVED IN WRITING BY THE CONTRACTING OFFICER. REINFORCING STEEL SPLICE[illegible] SHA[illegible] BE A[illegible] FOLLOWS, UNLESS NOTED OTHERWISE:
   - CO[illegible]RETE [illegible]NFOR[illegible]CEM[illegible]NT: CLASS B TENSION LAP
   - MA[illegible]NRY R[illegible]FOR[illegible]CEMENT: 48 BAR DIAMETERS
7. [illegible]DHES[illegible] FO[illegible] REINFORC[illegible]G DOWELS IN EXISTING CONCRETE SHALL BE EITHER THE [illegible]STEM CERAMIC 6 E[illegible]OXY ADHESIVE SUPPLIED BY ITW RAMSET/RED HEAD, POWER-FAST EP[illegible]Y INJECTION G[illegible]L SUPPLIED BY POWERS FASTENING, OR APPROVED EQUAL. MINIM[illegible] EMBEDMENT LENGTH SHALL BE 12 BAR DIAMETERS, UNLESS NOTED OTHERWISE. HIT-HY200 OR HIT-RE500 INJECTION ADHESIVE SUPPLIED BY HILTI FASTENING SYSTEMS, THE EPCON

CAST-IN-PLACE CONCRETE

1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS.
2. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:
   - 2.1 NORMAL WEIGHT STRUCTURAL CONCRETE:
     - FOOTINGS: 3,000 PSI
     - SLABS-ON-GRADE: 3,500 PSI W/ MACRO-FIBER
3. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.
4. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.
5. CONSTRUCTION JOINT LOCATIONS MAY OCCUR ONLY WHERE INDICATED "CJ" ON DRAWINGS. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
6. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.01 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE CONTRACTING OFFICER.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE "LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
2. STRUCTURAL STEEL SHALL BE OF THE FOLLOWING GRADE UNLESS NOTED OTHERWISE ON DRAWINGS:
   - 2.1 W-SHAPES SHALL CONFORM TO ASTM A992, GRADE 50. (ASTM A572, GRADE 50 MAY BE SUBSTITUTED FOR ASTM A992.)
   - 2.2 SQUARE/RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500, GRADE B.
   - 2.3 ROUND HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A501 OR ASTM A500, GRADE B.
   - 2.4 PLATE MATERIAL SHALL CONFORM ASTM A36
   - 2.5 OTHER STEEL SHAPES (CHANNELS AND ANGLES) MAY CONFORM TO ASTM A36.
3. BOLTS, RODS, ANCHORS AND HEADED STUDS:
   - 3.1 ALL CONNECTIONS SHALL BE SNUG TIGHT WITH A MINIMUM ¾" DIAME[illegible] A325 HIGH-STRE[illegible] [illegible]TS.
   - 3.2 ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 36, UNLESS NOTED OTHERWISE.
   - 3.3 EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT 3 ANCHORS SUPPLIED BY HI[illegible] [illegible]ENING S[illegible]MS, TRUE[illegible] WEDGE ANCHORS SUPPLIED BY ITW RAMSET/RED HEAD, POWER-STUD [illegible]HORS SUPPLIED BY POWE[illegible] FASTENING [illegible] APPROVED EQUAL. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMEND[illegible]S. MINIMUM EMBEDMENT S[illegible] BE EQU[illegible] TO 6 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.
   - 3.4 ADHESIVE ANCHORS SHALL CONSIST OF AN [illegible]-THREAD ST[illegible] ANCHOR WITH H[illegible]-RE500 MAX INJECTION ADHESIVE (HIT HY70 INJECTION ADHESIVE FOR MASONRY CONSTRUCTION WIT[illegible]DS) SUPPLIE[illegible] HILTI FASTE[illegible]S SYSTEMS, EPCON SYSTEM CERAMIC 6 EPOXY ADHESIVE SUPPLIED BY [illegible]/RED HEA[illegible] POWER-FAST E[illegible] INJEC[illegible] GEL SUPPLIED BY POWERS FASTENING, OR APPROVED EQUAL. INSTALL IN [illegible]CORDANCE [illegible] THE MAN[illegible]TURER'S PRI[illegible] [illegible]STALLATION INSTRUCTIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 6 TIMES THE AN[illegible] DIAMETER, UNLE[illegible] [illegible]TED OTHERWISE.
4. CONNECTIONS SHALL B[illegible] [illegible]TAILED BASED ON [illegible] DESIGN IN[illegible]ION PROVIDED IN THE CONTRACT DOCUMENTS. DEVIATION FROM THE CONNECTION DETAILS DE[illegible]D IN THE CONTRA[illegible] DOCUMEN[illegible] SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.
   - [illegible] STANDARD SHEAR CON[illegible]ONS SHAL[illegible] BE DETAILED AS DOUBLE-ANGLE, SINGLE-PLATE, SINGLE-ANGLE, OR TEE CONNECTIONS IN ACCORDANCE WITH CONN[illegible] TA[illegible]ES IN THE "MANUAL OF STEEL CONSTRUCTION: LRFD", SECOND EDITION, VOLUME II, PART 9.
   - 4.2 [illegible]R WELDED CONN[illegible]CTIONS, USE PREQUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF [illegible] AMERICAN WE[illegible]NG SOCIETY. "NON-PREQUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION.
   - 4.3 FA[illegible] [illegible]SIGN REACTIONS SHALL BE AS SHOWN ON THE STRUCTURAL DRAWINGS OR, IF NOT SHOWN, THE FACTORED DESIGN REA[illegible] SHALL BE HALF OF THE MAXIMUM FACTORED UNIFORM LOAD TABULATED IN THE "MANUAL OF STEEL CONSTRUCTION: LRFD", SE[illegible]D EDITION, VOLUME I, PART 4.
   - 4.4 STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. SHOP DRAWINGS AND CALCULATIONS OF SUCH CONNECTIONS SHALL BE SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS. FOR CONNECTION DETAILS DEPICTING ARRANGEMENT CONCEPT OF THE CONNECTION WITHOUT COMPLETE DETAILS, THE CONNECTION DESIGN ENGINEER SHALL FOLLOW THAT ARRANGEMENT CONCEPT IN THE DESIGN.
5. STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED, UNLESS OTHERWISE DIRECTED BY THE ARCHITECT.

PREENGINEERED METAL BUILDING DESIGN

A. DESIGN OF PREENGINEERED METAL BUILDING SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA. IN ADDITION TO REQUIREMENTS BELOW, DESIGN SHALL INCLUDE THE EFFECTS OF LOADING FROM MISCELLANEOUS FRAMING, HUNG BRICK LINTELS, SUSPENDED EQUUIPMENT, ETC.

B. MEMBERS TO WITHSTAND BUILDING SYSTEM DEAD LOADS, SNOW LOADS, LIVE LOAD, COLLATERAL LOAD, WIND LOAD, AND SEISMIC LOADS BASED ON CRITERIA LISTED IN THE "CODE/ DESIGN CRITERIA" SECTION OF THESE NOTES. ALL LOADS SHALL BE PROPORTIONED, COMBINED, AND APPLIED IN ACCORDANCE WITH THE 2012 EDITION OF THE NORTH CAROLINA BUILDING CODE EXCLUSIVELY.

C. DEFLECTIONS SHALL BE LIMITED AS FOLLOWS:

- PRIMARY FRAMING
  - L/360 FOR ROOF SNOW LOAD OR LIVE LOAD.
  - LESS THAN OR EQUAL TO H/240 LATERAL DEFLECTION OR DRIFT FOR 10-YEAR RECURRENCE INTERVAL WIND LOAD. LATERAL DEFLECTION UNDER THE EQUIVALENT 50-YEAR RECURRENCE INTERVAL WIND LOAD (H/169) SHALL NOT IMPAIR THE SERVICABILITY OF THE METAL BUILDING SYSTEM.
- SECONDARY FRAMING
  - ROOF FRAMING FOR GRAVITY LOAD
    - L/360 FOR ROOF SNOW LOAD OR LIVE LOAD.
  - WALL FRAMING FOR WIND LOAD
    - L/240 FOR 50-YEAR RECURRENCE INTERVAL WIND LOAD.
  - ROOF FRAMING FOR WIND LOAD
    - L/240 FOR 50-YEAR RECURRENCE INTERVAL WIND LOAD.
- SHEETING
  - L/180 FOR ROOF SNOW LOAD OR WIND LOAD (BUT NOT LESS THAN 20 PSF).

WHERE L IS THE SPAN OF THE ELEMENT BETWEEN SUPPORT POINTS, AND H IS THE EAVE HEIGHT OF THE BUILDING.

D. CONNECTIONS OF COLUMN BASES TO THE FOUNDATION SHALL BE CONSIDERED AS THEORETICAL PINS (NO MOMENT TRANSFER TO FOUNDATION)

E. SUBMIT FOUNDATION REACTIONS FOR EACH COLUMN FOR ALL REQUIRED LOADS AND COMBINATIONS AND LOADING DIRECTIONS. SUBMIT ANCHOR BOLT LAYOUT, SIZE AND SPACING FOR EACH COLUMN. CONTRACTING OFFICER WILL REVIEW SUBMITTAL AND FOUNDATION FOR ADEQUACY BASED ON THE LOADS SUBMITTED AND MAKE REVISIONS IF NECESSARY. DO NOT BEGIN FOUNDATION FABRICATION OR CONSTRUCTION PRIOR TO THIS REVIEW.

F. NO PROVISIONS ARE REQUIRED OR PROVIDED FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

WILKES COMMUNITY COLLEGE

SCO# [illegible]-01A
N[illegible]CS# 223[illegible]

[illegible]ULINARY LAB

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REVISIONS:

No.	Description	Date

PROJECT: 9202-169770
DATE: January 2, 2018
DRAWN BY: M.W.T.
CHECKED BY: W.D.B. III

STRUCTURAL NOTES

S-001

BID SET

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