1. GENERAL NOTES:

1. CODES AND SPECIFICATIONS:

   A. GENERAL BUILDING CODE: INTERNATIONAL BUILDING CODE, 2009 EDITION.

   B. MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTUERS INCLUDING SUPPLEMENTS NO. 1 AND 2, EXCLUDING CHAPTER 14 AND APPENDIX 11A (ASCE 7-05).

   C. CONCRETE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-08).

   D. STRUCTURAL STEEL: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-05). ALLOWABLE STRESS DESIGN (ASD).

   E. STEEL DECK: STEEL DECK INSTITUTE DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, ROOF DECKS AND CELLULAR METAL FLOOR DECK WITH ELECTRICAL DISTRIBUTION.

   F. MASONRY:
      1. BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-08).
      2. SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-08).

   G. COLD-FORMED METAL FRAMING: NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS (AISI S100-07).

2. THE GENERAL NOTES ARE NOT A SUBSTITUTE OR A REPLACEMENT FOR THE PROJECT SPECIFICATIONS. THESE NOTES ARE INTENDED AS A GUIDE TO THE DESIGN AND/OR CONSTRUCTION REQUIREMENTS ESTABLISHED FOR THIS PROJECT. NO CONTRACTOR SHOULD ATTEMPT TO DESIGN, BID, OR CONSTRUCT ANY PORTION OF THE WORK HEREIN WITHOUT CONSULTING THE PROJECT SPECIFICATIONS. THE MORE STRINGENT REQUIREMENT SHALL APPLY WHERE CONFLICTS OCCUR BETWEEN THESE NOTES AND THE SPECIFICATIONS, UNLESS A WRITTEN CLARIFICATION IS ISSUED BY THE STRUCTURAL ENGINEER.

3. STRUCTURAL DRAWINGS ARE INTENDED TO BE USED IN CONJUNCTION WITH THE DRAWINGS OF OTHER CONSULTANTS AND TRADES. THE CONTRACTOR SHALL COORDINATE THE VARIOUS REQUIREMENTS.

4. DO NOT SCALE THESE DRAWINGS.

5. ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS, UNLESS NOTED.

6. CONTRACTOR SHALL REVIEW AND VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. THE STRUCTURAL ENGINEER AND ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION.

7. FIREPROOFING OF STRUCTURAL ELEMENTS IS NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO THE DRAWINGS OF OTHER CONSULTANTS FOR SUCH INFORMATION.

2. SPECIAL INSPECTIONS: (BY TESTING AGENCY)

1. SPECIAL INSPECTOR (SI) SHALL BE RETAINED AND PAID BY THE OWNER.

2. THE SPECIAL INSPECTOR SHALL BE FULLY QUALIFIED, APPROVED BY THE BUILDING OFFICIAL, REGISTERED BY APPLICABLE REGISTRATION BOARD IF REQUIRED AND ACCEPTABLE TO THE ARCHITECT.

3. THE DUTIES OF THE SPECIAL INSPECTOR SHALL INCLUDE, BUT ARE NOT LIMITED TO, VERIFICATION OF CONSTRUCTION QUALITY CONTROL, TESTING, COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS, BUILDING CODE REQUIREMENTS, AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.

4. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE PROPER NOTIFICATION TO THE SPECIAL INSPECTOR AND PROCEED WITH THE CONSTRUCTION ONLY AFTER THE SPECIAL INSPECTOR'S REVIEW AND APPROVAL.

5. SPECIAL INSPECTOR'S SHALL KEEP RECORDS OF ALL INSPECTIONS AND TESTING. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE CODE OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CODE OFFICIAL AND THE DESIGN PROFESSIONAL OF RECORD. A FINAL REPORT OF INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTION AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY. INTERIM REPORTS SHALL BE SUBMITTED PERIODICALLY WITH MINIMUM FREQUENCY OF TWO WEEKS.

6. SPECIAL INSPECTIONS ARE REQUIRED FOR, BUT NOT LIMITED TO, THE ACTIVITIES AS INDICATED ON SHEET S1.02 PER THE 2009 INTERNATIONAL BUILDING CODE.

7. FAILURE TO NOTIFY THE SPECIAL INSPECTOR MAY RESULT IN THE CONTRACTOR HAVING TO REMOVE WORK FOR THE PURPOSE OF INSPECTION AT THE CONTRACTOR'S EXPENSE.

8. PREMATURE NOTIFICATION FOR INSPECTIONS WILL RESULT IN AN ADDITIONAL INSPECTION WITH THE EXPENSES AND FEES PAID BY THE CONTRACTOR.

3. CONSTRUCTION AND SAFETY:

1. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL SAFETY REGULATIONS, PROGRAMS, AND PRECAUTIONS TO ALL WORK, PERSONS, AND PROPERTY ON AND/OR ADJACENT TO THE PROJECT AND SHALL PROTECT AGAINST ANY DAMAGE, INJURY, OR LOSS.

2. MEANS AND METHODS OF CONSTRUCTION AND ERECTION OF STRUCTURAL MATERIALS ARE SOLELY THE CONTRACTOR'S RESPONSIBILITY.

3. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE STRUCTURE. SUCH LOADS SHALL NOT EXCEED THE DESIGN LOAD OF THE STRUCTURE AT ANY TIME.

4. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION, AND ANY TEMPORARY BRACING OR SUPPORT REQUIRED TO ACCOMMODATE THE CONTRACTOR'S MEANS AND METHODS ARE THE RESPONSIBILITY OF THE CONTRACTOR.

4. SUBMITTALS:

1. ALL SHOP DRAWINGS MUST BE REVIEWED FOR "APPROVAL" AND STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL.

2. SUBMIT EACH SET OF SHOP DRAWINGS ON THREE PRINTS ONLY. ONE PRINT WILL BE RETURNED TO THE CONTRACTOR. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHOULD BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED. ALL ADDITIONAL PRINTS THAT ARE SUBMITTED WILL BE RETURNED UNMARKED.

3. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS. ITEMS MARKED (*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED. ITEMS MARKED (#) SHALL BE SUBMITTED FOR ENGINEER 'S RECORD ONLY.

   A. REINFORCING STEEL
   B. STEEL DECK
   C. CONCRETE MIX DESIGNS
   D. COLD-FORMED METAL FRAMING (*#)
   E. SHOP FABRICATED COLD-FORMED METAL TRUSSES (*)

4. DESIGN CALCULATIONS: THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER'S REVIEW TWO SETS OF DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED, FOR THE FOLLOWING ITEMS:

   A. SHOP FABRICATED COLD-FORMED METAL TRUSSES
   B. COLD-FORMED METAL FRAMING

5. DESIGN LOADS:

1. DEAD LOADS:
   ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE GENERAL CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE.

2. LIVE LOADS:
   LIVE LOAD REDUCTIONS HAVE BEEN APPLIED TO THE STRUCTURAL MEMBERS IN ACCORDANCE WITH THE BUILDING CODE.

   ROOF------------------------------20 PSF
   MEZZANINE-------------------------150 PSF

3. SNOW LOAD:
   A. GROUND SNOW LOAD (Pg.)---------[illegible] PSF
   B. IMPORTANCE FACTOR (I)----------1.0
   C. EXPOSURE FACTOR (Ce)-----------1.0
   D. THERMAL FACTOR (Ct)------------1.0

4. WIND LOADS:
   A. BASIC WIND SPEED (3-SECOND GUST)----[illegible]
   B. WIND IMPORTANCE FACTOR (Iw)---------1.0
   C. WIND EXPOSURE-----------------------C
   D. INTERNAL PRESSURE COEFFICIENT-------+/- 0.18

COMPONENT AND CLADDING WALL PRESSURES:
   A. SEE SHEET S1.04

5. SEISMIC LOADS:
   A. SEISMIC IMPORTANCE FACTOR (Ie)--------1.0
   B. MAPPED SPECTRAL RESPONSE ACCELERATIONS:
      Ss---------------------------------0.305
      S1---------------------------------0.100
   C. SITE CLASS-------------------------D
   D. SPECTRAL RESPONSE COEFFICIENTS:
      Sds--------------------------------0.316
      Sd1--------------------------------0.161
   E. SEISMIC DESIGN CATEGORY------------C
   F. BASIC SEISMIC-FORCE RESISTING SYSTEM:
      LIGHT-FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
         RESPONSE MODIFICATION FACTOR (R)----------6.5
         SEISMIC RESPONSE COEFFICIENT (Cs)---------0.049
         DESIGN BASE SHEAR-------------------------8 K
   G. ANALYSIS PROCEDURE:
         EQUIVALENT LATERAL FORCE METHOD

6. FOUNDATION NOTES:

1. A GEOTECHNICAL ENGINEER, EMPLOYED BY THE GENERAL CONTRACTOR, SHALL PROVIDE COMPACTED FILL REQUIREMENTS FOR THE BUILDING PAD AND REVIEW THE FOUNDATION BEARING SURFACE TO VERIFY THE ASSUMED BEARING PRESSURE NOTED. DO NOT PLACE CONCRETE PRIOR TO GEOTECHNICAL ENGINEER'S APPROVAL.

2. ASSUMED MAXIMUM BEARING PRESSURES (PSF)----------2000

3. ALL FOUNDATION BEARING SURFACES SHALL BE REVIEWED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO INSURE THEIR COMPLIANCE WITH PRESSURES NOTED. ALL BOTTOM ELEVATIONS ARE ESTIMATED AND MAY BE ADJUSTED IN THE FIELD BY THE GEOTECHNICAL ENGINEER.

4. ALL AREAS TO HAVE SLABS ON GRADE SHALL BE PROOF ROLLED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT UNDER OBSERVATION OF THE GEOTECHNICAL ENGINEER AND APPROVED PRIOR TO PREPARATION FOR CONCRETE PLACEMENT.

5. COMPACTED FILL WITHIN THE BUILDING AREA SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT.

6. BACKFILL FOR FOUNDATION AND RETAINING WALLS SHALL BE A FREE DRAINING GRANULAR MATERIAL, SUCH AS SIZE #57 STONE. BACKFILL SHALL BE COMPACTED SUFFICIENTLY TO PREVENT SUBSIDENCE OF SURFACE ADJACENT TO WALL. THE GRANULAR MATERIAL SHALL BE PLACED IN A 45 DEGREE WEDGE EXTENDING FROM THE BASE OF THE WALL UP TO SUBGRADE.

7. CONCRETE NOTES:

1. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS:
   3000 PSI --- NORMAL WT. --- TYPICAL

2. PEDESTAL, COLUMN AND WALL VERTICAL REINFORCING: DOWEL TO FOUNDATION WITH HOOKED BARS OF SAME SIZE AND SPACING AS VERTICAL REINFORCING.

3. EARTH SUPPORTED SLABS:
   4" THICK, REINFORCED WITH 6X6 W2.9/W2.9 WWF (SHEETS)AT TOP THIRD OF SLAB DEPTH, UNLESS NOTED.

4. CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.

8. CONCRETE REINFORCING STEEL NOTES:

1. REINFORCING BARS: ASTM A615 GRADE 60.

2. WELDED WIRE FABRIC (WWF): ASTM A185. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2 INCHES OR 6 INCHES.

3. REINFORCING STEEL SHOWN IN SECTIONS IS A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES AND GENERAL NOTES FOR ACTUAL REINFORCING REQUIRED.

4. REINFORCING BAR PLACING ACCESSORIES IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE. WHERE CONCRETE IS EXPOSED IN FINISHED BUILDING, PROVIDE ACCESSORIES WITH RUSTPROOF LEGS. WHERE CONCRETE IS SAND-BLASTED OR BUSH-HAMMERED, PROVIDE ACCESSORIES OF STAINLESS STEEL.

5. DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315. REINFORCEMENT SHALL NOT BE WELDED UNLESS NOTED OR APPROVED BY THE ENGINEER.

6. ALL SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.

7. ALL REINFORCING MARKED "CONTINUOUS" SHALL BE SPLICED WITH CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.

8. PROVIDE CORNER BARS WITH CLASS "B" LAP SPLICE AT ALL FOUNDATION INTERSECTIONS AND CORNERS.

9. CONCRETE COVERAGE OF REINFORCEMENT:

   FOOTINGS------------------3" BOTTOM & SIDES, 2" TOP

10. FIELD BENDING OF CONCRETE REINFORCING STEEL IS NOT PERMITTED WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.

9. STRUCTURAL STEEL NOTES:

1. STRUCTURAL STEEL: ASTM A992 FOR WIDE FLANGE SHAPES; ASTM A36 ELSEWHERE.

2. HOLLOW STRUCTURAL SECTIONS: ASTM A500, GRADE B.

3. WELDED CONNECTIONS: E70XX ELECTRODES, MINIMUM SIZE FILLET WELD 3/16".

4. HEADED ANCHOR RODS: ASTM F1554 GRADE 36 AND HEAVY HEX NUT, UNLESS OTHERWISE INDICATED.

5. HEADED STUDS: ASTM A108, GRADE 1015 OR 1020, COLD-FINISHED CARBON STEEL WITH DIMENSIONS COMPLYING WITH AISC.

6. BOLTED CONNECTIONS: BEARING TYPE A325-N IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". BOLTS THROUGH 4" WIDE BEAM FLANGES SHALL BE 5/8"DIAMETER. OTHER BOLTS SHALL BE 3/4" DIAMETER. USE SNUG TIGHT BEARING CONNECTIONS FOR ALL BOLTED CONNECTIONS.

7. FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."

8. THE STEEL FRAME IS "NON-SELF-SUPPORTING." ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR UNTIL REQUIRED CONNECTIONS OR ELEMENTS ARE IN PLACE.

10. STEEL DECK NOTES:

1. DECK PROPERTIES AND ATTACHMENTS SHALL BE IN ACCORDANCE WITH THE STEEL DECK INSTITUTE.

2. DECK SHALL BE CONTINUOUS OVER THREE OR MORE SPANS.

3. ROOF DECK: WIDE RIB TYPE "WR" STEEL ROOF DECK, 22 GAGE, 1-1/2" DEEP, GALVANIZED.

4. ROOF DECK ATTACHMENT: #12 TEK SCREWS IN A [illegible] PATTERN WITH (3)#10 TEK SCREW SIDELAP FASTENERS PER SPAN.

5. FORM DECK: [illegible]1/2" THICK CONCRETE SLAB ON NON-COMPOSITE STEEL FORM DECK, 24 GAGE, 9/16" DEEP WITH #3@[illegible] EA WAY, 1" CLEAR FROM TOP.

6. FORM DECK ATTACHMENT: #12 TEK SCREWS IN A 36/3 PATTERN WITH (1)#10 TEK SCREW SIDELAP FASTENERS PER SPAN.

7. LIGHTGAGE METAL FRAMING, SUSPENDED CEILINGS, LIGHT FIXTURES AND DUCTS OR OTHER UTILITIES SHALL NOT BE SUPPORTED BY THE METAL ROOF DECK.

11. MASONRY NOTES:

1. MASONRY CONSTRUCTION SHALL CONFORM TO ACI 530.1 SPECIFICATION.

2. COMPRESSIVE STRENGTH OF MASONRY (f'm) SHALL BE 1500 PSI AT 28 DAYS. MASONRY UNIT STRENGTH OF 1900 PSI IS REQUIRED TO ACHIEVE REQUIRED f'm.

3. MASONRY GROUT FILL SHALL CONFORM TO ASTM C 476. GROUT EITHER FINE (SAND) OR COURSE (SAND + #76 STONE) AGGREGATE SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI. MASONRY CONCRETE FILL SHALL CONFORM TO THE REQUIREMENTS NOTED UNDER "CONCRETE" IN THE GENERAL NOTES.

4. GROUTING
   A. ALL BOND BEAMS SHALL BE FILLED WITH GROUT AND REINFORCED AS INDICATED ON THE DRAWINGS (DETAILS OR SCHEDULES). MORTAR FILL IS NOT PERMITTED.
   B. ALL MASONRY WALL CELLS OR CAVITIES INDICATED AS REINFORCED SHALL BE GROUTED FOR THE FULL HEIGHT OF THE WALL, UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS. UN-REINFORCED WALLS INDICATED AS GROUTED SHALL BE GROUTED FULL HEIGHT, UNLESS SPECIFICALLY NOTED OTHERWISE. MORTAR FILL IS NOT PERMITTED.
   C. ALL MASONRY CELLS OR CAVITIES BELOW GRADE SHALL BE GROUTED SOLID UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS. MORTAR FILL IS NOT PERMITTED.
   D. VERTICAL GROUTING SHALL BE LOW LIFT OR HIGH LIFT AS FOLLOWS:
      1. LOW LIFT GROUTING SHALL BE USED FOR ALL CAVITY WALLS AND MAY BE USED FOR ALL WALLS AT THE OPTION OF THE CONTRACTOR. LIFTS SHALL NOT EXCEED 4'0" IN HEIGHT.
      2. HIGH LIFT GROUTING IS PERMISSIBLE ONLY FOR FILLING OF CELLULAR MASONRY UNITS AND SHALL NOT EXCEED ONE STORY IN HEIGHT. CLEAN OUT HOLES SHALL BE PROVIDED AT THE BASE OF EACH GROUTED CELL.

5. MORTAR SHALL CONFORM TO ASTM C 270. MORTAR SHALL BE TYPE "M" FOR BELOW GRADE APPLICATIONS AND TYPE "S" FOR ABOVE GRADE APPLICATIONS AND SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH 1900 PSI.

6. ALL MASONRY SHALL BE RUNNING BOND, UNLESS NOTED.

7. ALL BLOCK CELLS AND CAVITIES BELOW GRADE SHALL BE FILLED WITH CONCRETE OR GROUT.

8. ALL INTERIOR PARTITION WALLS SHALL HAVE A 8" DEEP BOND BEAM WITH 1#5 CONTINUOUS TOP AND BOTTOM AT THE TOP OF THE WALL, UNLESS NOTED OTHERWISE IN DRAWINGS.

9. REINFORCING:
   A. ALL BARS MARKED "CONTINUOUS" SHALL BE LAPPED A MINIMUM OF 48 BAR DIAMETERS AT ALL SPLICES. UNLESS NOTED OTHERWISE, ALL WALL/FOOTING DOWELS SHALL BE LAPPED 48 BAR DIAMETERS OR 34", WHICHEVER IS GREATEST.
   B. FOUNDATION DOWELS MAY SLOPE A MAXIMUM OF 1:6 TO ALIGN WITH WALL CAVITIES OR VERTICAL CMU CORES. GREATER SLOPES WILL REQUIRE REPLACEMENT OF THE FOUNDATION DOWELS.
   C. SPLICED REINFORCING SHALL BE LAPPED UNDER "REINFORCING" ABOVE OR AS SHOWN ON DRAWINGS, WHICHEVER IS GREATEST. ALL SPLICES SHALL BE WIRED TOGETHER.
   D. VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 3/4" FROM MASONRY AND SHALL BE HELD IN POSITION TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 4"-0". ACCESSORIES FOR SUCH SUPPORT SHALL BE USED, PROVIDE "AA WIRE PRODUCTS COMPANY" (OR APPROVED EQUAL) REBAR POSITIONER AA225 OR AA239 FOR VERTICAL BARS AND AA28 FOR HORIZONTAL BARS OR APPROVED EQUAL PRODUCTS FROM OTHER SUPPLIERS.
   E. HORIZONTAL JOINT REINFORCING SHALL BE LAPPED NO LESS THAN 6" AT ALL SPLICES, INCLUDING CORNERS AND TEES WHERE NO CONTROL JOINT IS USED.
   F. ALL HORIZONTAL JOINT REINFORCING SHALL STOP AT CONTROL JOINTS.
   G. HORIZONTAL REINFORCING IN BOND BEAMS SHALL BE CONTINUOUS THROUGH CONTROL JOINTS.

10. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND DETAILS OF MASONRY CONTROL JOINTS.

11. WHEN REINFORCING IS SPECIFIED, PROVIDE AT EACH SIDE OF CONTROL JOINTS, OPENINGS AND WALL ENDS.

12. ALL MASONRY WALLS SHOWN ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES IN THE FINAL CONSTRUCTED CONFIGURATION ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ADEQUATELY BRACE THE WALLS FOR VERTICAL AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF CONSTRUCTION.

13. PROVIDE HORIZONTAL LADDER-TYPE JOINT REINFORCEMENT AT 16" O.C., U.N.O.

12. COLD-FORMED METAL FRAMING NOTES:

1. ALL MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AMERICAN IRON AND STEEL INSTITUTE.

2. ALL FRAMING MEMBERS SHALL BE FORMED FROM CORROSION-RESISTANT STEEL CORRESPONDING TO THE REQUIREMENTS OF ASTM A446, WITH A MINIMUM YIELD STRENGTH OF 33 KSI OR 50 KSI AS INDICATED.

3. ALL MEMBERS SHOWN ARE STANDARD DESIGNATIONS OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA).

4. DESIGN OF MEMBERS INDICATED IN STRUCTURAL DRAWINGS IS BASED ON MINIMUM PROPERTIES OF PRODUCTS PRODUCED BY THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA).

5. FABRICATOR IS RESPONSIBLE FOR THE DESIGN AND DETAILING OF ALL LIGHTGAGE STEEL FRAMING AND SHALL RETAIN A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. THE ENGINEER SHALL SEAL AND SIGN BOTH CALCULATIONS AND SHOP DRAWINGS. [illegible]LY, CONNECTIONS SHOWN ON THE DRAWINGS ARE SCHEMATIC AND ARE INTENDED ONLY TO SHOW THE RELATIONSHIP OF THE MEMBERS AND INFORMATION FOR PRICING/BIDDING.

6. SUBMIT CALCULATIONS AND SHOP DRAWINGS FOR DETAILS, FABRICATION, AND ERECTION OF LIGHTGAGE STEEL FRAMING. DRAWINGS SHALL INCLUDE LAYOUT, SPACING, TYPE, MATERIAL/MEMBER PROPERTIES AND ALL DETAILS OF CONNECTIONS FOR ALL LIGHTGAGE STEEL FRAMING INDICATED ON THE STRUCTURAL DRAWINGS.

7. SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMISSION. DRAWINGS SHALL BEAR THE CONTRACTOR'S APPROVAL STAMP ACCEPTING RESPONSIBILITY FOR DIMENSIONS, QUANTITIES AND COORDINATION WITH THE OTHER TRADES.

8. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR AS REQUIRED FOR AN ANGULAR FIT TIGHT AGAINST ABUTTING MEMBERS.

9. AXIALLY LOADED STUDS SHALL BE INSTALLED IN A MANNER WHICH WILL ASSURE THAT THEIR ENDS ARE POSITIONED TIGHT AGAINST THE INSIDE OF RUNNER WEBS PRIOR TO FASTENING. PROVIDE WEAK-AXIS HORIZONTAL BRACING AT 36 INCHES MAXIMUM VERTICAL SPACING, BOTH STUD FLANGES. HORIZONTAL BRACING SHALL BE 1-1/2" X 20 GA STRAPS AND CS TYPE RUNNER SOLID BRIDGING AT EACH END OF WALL, ADJACENT TO WALL OPENINGS, AND 8'-0" O/C MAXIMUM.

10. FLOOR JOISTS SHALL HAVE SOLID BLOCKING, A CUT-TO-LENGTH CS TYPE CLOSURE OR JOIST SECTION PLACED BETWEEN OUTER JOISTS, OVER ALL INTERIOR SUPPORTS, ADJACENT TO OPENINGS AND AT 8'-0 O/C MAXIMUM. BETWEEN SOLID BLOCKING PROVIDE STRAP BRACING OF 1-1/2 X 20 GA STRAPS. JOIST BRIDGING SHALL BE PROVIDED AT MID-SPAN OR AT 8-0' O/C MAXIMUM FOR SPANS EXCEEDING 15-0.

11. WHERE STUD FRAME TO STRUCTURAL FLOOR MEMBERS SUBJECT TO DEFLECTION FROM OCCUPANT LIVE LOADING, A DEFLECTION CLIP ASSEMBLY SHALL BE PROVIDED AT THE TOP RUNNER TO ACCOMMODATE VERTICAL STRUCTURAL MOVEMENT.

12. FASTENING OF COMPONENTS SHALL BE BY SELF-DRILLING SCREWS OR BY WELDING AS DEFINED BELOW U.N.O. ON THE DRAWINGS.

13. SCREWED CONNECTIONS:
    A. SCREWS SHALL BE TYPE S-12 OR TYPE S-4 FOR ALL FRAMING MEMBERS PER MANUFACTURER'S RECOMMENDATIONS.
    B. A MINIMUM OF THREE (3) EXPOSED THREADS SHALL PENETRATE THROUGH ALL JOINED MATERIALS.
    C. CORROSION-RESISTANT CADMIUM-PLATED SCREWS SHALL BE USED FOR SCREWS ATTACHING METAL LATH, MASONRY TIES, AND OTHER EXTERIOR MATERIALS.

14. WELDED CONNECTIONS:
    A. USE GAS METAL ARC WELDING (GMAW) FOR 20 GA OR LIGHTER MEMBERS. AWS E-703-3, E-705-5, E-705-6 WIRE ELECTRODES OF 035" DIAMETER SHALL BE USED WITH CARBON DIOXIDE, ARGON-OXYGEN, OR ARGON-CARBON DIOXIDE GAS SHIELDING. WELDING EQUIPMENT SHALL PROVIDE 60 TO 100 AMPERES AT 25 VOLTS USING 220 VOLT 3-PHASE ELECTRIC SERVICE.
    B. SHIELDED METAL ARC WELDING (SMAW) SHALL BE USED FOR 18 GA AND HEAVIER MEMBERS AWS E-6012, E-6013, OR E-014 ELECTRODES OF 3/32" OR 1/8" DIAMETER SHALL BE USED. WELDING EQUIPMENT HEAT SETTING SHALL BE VARIED DEPENDENT ON MATERIAL THICKNESS.
    C. ALL WELDS SHALL BE TOUCHED UP WITH A ZINC RICH PAINT, OR PAINT SIMILAR TO THAT USED BY THE FRAMING MEMBER SUPPLIER ON PAINTED MEMBERS FROM THE SUPPLIER.

15. CUTTING OF STEEL FRAMING MEMBERS MAY BE DONE WITH A SAW OR CUTTING SHEARS. TORCH CUTTING OF LOAD BEARING MEMBERS IS NOT PERMITTED.

16. COMPLETE, UNIFORM, AND LEVEL BEARING SUPPORT SHALL BE PROVIDED FOR THE BOTTOM RUNNER. AT SPLICES WHERE SUPPORT IS NOT COMMON TO BOTH RUNNERS, EITHER BUTT WELD RUNNERS OR USE A STUD SECTION INSERTED IN THE RUNNER AS A SPLICING MEMBER ATTACHED PER MANUFACTURER'S RECOMMENDATIONS. RUNNER INTERSECTIONS SHALL BUTT EVENLY.

17. SPACING OF STUDS SHALL HAVE A TOLERANCE OF 1/8" FROM THAT SHOWN ON THE DRAWINGS, PROVIDING THAT THE CUMULATIVE ERROR DOES NOT EXCEED THE REQUIREMENTS OF OTHER MATERIALS OR CONSTRUCTION.

18. ALIGNMENT OF STUDS (PLUMBNESS) AND WALLS (STRAIGHTNESS) SHALL BE WITHIN 1/960TH OF THEIR RESPECTIVE HEIGHTS AND LENGTHS.

19. STUDS SHALL BE PLUMBED ALIGNED AND SECURELY ATTACHED TO BOTH TOP AND BOTTOM RUNNERS. SPLICES IN STUDS ARE NOT PERMITTED.

20. TEMPORARY BRACING WHERE REQUIRED, SHALL BE PROVIDED UNTIL ERECTION IS COMPLETED.

21. WHERE MANUFACTURER'S RECOMMENDATIONS FOR ERECTION, ATTACHMENT, ASSEMBLY, BRACING ALIGNMENT OR OTHER REQUIREMENTS ARE MORE STRINGENT THAN INDICATED IN THESE DRAWINGS OR THE PROJECT SPECIFICATIONS, THE MANUFACTURER'S RECOMMENDATIONS SHALL APPLY.

13. PREFABRICATED COLD-FORMED METAL TRUSS NOTES:

1. STRUCTURAL PROPERTIES OF TRUSS MEMBERS SHALL BE COMPUTED IN ACCORDANCE WITH AISI "NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".

2. THE TRUSS FABRICATOR IS RESPONSIBLE FOR THE DESIGN AND DETAILING OF ALL PREFABRICATED COLD-FORMED METAL TRUSS FRAMING AND SHALL RETAIN A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. THE ENGINEER SHALL SEAL AND SIGN BOTH CALCULATIONS AND SHOP DRAWINGS.

3. THE TRUSS MANUFACTURER SHALL DESIGN FOR THE FOLLOWING SUPERIMPOSED LOADS (IN ADDITION TO ANY LOADS NOTED ON THE STRUCTURAL DRAWINGS OR OTHER CONSULTANTS/TRADES DRAWINGS):

   TOP CHORD DEAD LOAD--------------------15 PSF
   BOTTOM CHORD DEAD LOAD-----------------10 PSF
   TOP CHORD LIVE LOAD--------------------20 PSF

4. DESIGN ROOF TRUSSES TO RESIST THE FOLLOWING WIND SUPERIMPOSED NET UPLIFT WIND PRESSURES:
   A. SEE SHEET S1.04.

5. SUBMIT CALCULATIONS AND SHOP DRAWINGS FOR DETAILS FABRICATION, AND ERECTION OF COLD-FORMED METAL TRUSS FRAMING. DRAWINGS SHALL INCLUDE LAYOUT, SPACING, TYPE, MATERIAL/MEMBER PROPERTIES, TEMPORARY BRACING, PERMANENT BRACING, AND ALL DETAILS OF CONNECTIONS FOR ALL COLD-FORMED METAL TRUSS FRAMING INDICATED ON THE STRUCTURAL DRAWINGS.

6. SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR PRIOR TO SUBMISSION. DRAWINGS SHALL BEAR THE CONTRACTOR'S APPROVAL STAMP ACCEPTING RESPONSIBILITY FOR DIMENSIONS, QUANTITIES, AND COORDINATION WITH THE OTHER TRADES.

7. ALL TEMPORARY AND PERMANENT BRACING MEMBERS AND CONNECTIONS REQUIRED FOR PREFABRICATED COLD-FORMED METAL TRUSSES SHALL BE DESIGNED AND DETAILED ON THE TRUSS MANUFACTURER'S ERECTION PLANS. BRACING MEMBERS SHALL BE FURNISHED AND INSTALLED BY THE GENERAL CONTRACTOR.

8. TEMPORARY BRACING SHALL NOT IMPOSE ANY FORCE ON THE SUPPORTING STRUCTURE. PERMANENT BRACING FORCES SHALL BE TRANSFERRED TO THE ROOF DIAPHRAGM BY THE BRACING DESIGN PROVIDED BY THE TRUSS MANUFACTURER.

TUCKER-JONES
3300 CAHABA ROAD, SUITE 210
BIRMINGHAM, ALABAMA 35223
PHONE: 205.879.5660
FAX: 205.879.5606
PROJECT #17171

GMC
2701 1st Avenue S
Birmingham, AL 35233
T 205.879.4462
GMCNETWORK.COM

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Oneonta Community Development Center
100 1st Ave W.
Oneonta, AL
GMC # ABHM170022

ALABAMA REGISTERED No. 21338 PROFESSIONAL ENGINEER GREGORY L. TUCKER
11/30/2017

GENERAL NOTES

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