

GENERAL NOTES

- ALL WORK SHALL BE IN STRICT ACCORDANCE WITH THE 2012 EDITION OF THE INTERNATIONAL BUILDING CODE AND ALL APPLICABLE LOCAL CODES
- LIVE LOADS
 - GROUND FLOOR - BUSINESS 50 PSF
 - ROOF - 20 PSF
 - ROOFTOP MECH AREAS - 50 PSF
- GROUND SNOW LOAD - NONE
- DESIGN WIND SPEED = 136 MPH, RISK CATEGORY IV, EXPOSURE CATEGORY C
- ONLY WRITTEN CHANGES APPROVED BY THE ARCHITECT/ENGINEER SHALL BE PERMITTED
- PHOTOGRAPHIC REPRODUCTIONS OF STRUCTURAL DRAWINGS ARE NOT PERMITTED ON SHOP DRAWINGS
- GENERAL CONTRACTOR SHALL COORDINATE STRUCTURAL DRAWINGS WITH ALL OTHER DISCIPLINES. WHERE THERE ARE CONFLICTS IN INFORMATION PRESENTED IN THE DRAWINGS OR IF THE DRAWINGS ARE UNCLEAR OR INSUFFICIENT IN ANY MANNER THAT MAY INHIBIT THE CONTRACTOR'S UNDERSTANDING OF THE PROJECT, SUCH CONFLICTS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER PRIOR TO BIDDING AND THE NECESSARY ADJUSTMENTS SHALL BE MADE PER THEIR INSTRUCTIONS.
- COORDINATE WITH ARCHITECTURAL DRAWINGS FOR DIMENSIONS, TOP OF CONCRETE ELEVATIONS, TOP OF STEEL ELEVATIONS, SLOPES, RECESSES, LEDGES AND STEPS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS.

CONCRETE

- ALL WORK TO BE IN STRICT ACCORDANCE WITH THE LATEST EDITIONS OF ACI 301 AND ACI 318
- MIX DESIGN CRITERIA - ALL FOUNDATION CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS. NO CHLORIDES ALLOWED. FLYASH SHALL BE LIMITED TO A MAXIMUM OF 20% OF CEMENTITIOUS MATERIALS BY WEIGHT.
- STEEL REINFORCING BARS - ASTM A615 GRADE 60
- WELDED PLAIN WIRE FABRIC SHEETS - ROLL TYPE WWF SHALL NOT BE USED. ASTM A1064 WITH MINIMUM YIELD STRENGTH = 60 KSI. WWF SHALL BE PLACED AT MID-DEPTH OF SLAB
- LAPS, SPLICES, TIES, AND EMBEDMENT LENGTHS FOR REINFORCING STEEL SHALL BE IN ACCORDANCE WITH THE ACI MANUAL OF STANDARD PRACTICE, DETAILS, AND DETAILING OF CONCRETE REINFORCEMENT, ACI 318, ACI 315, AND IN ACCORDANCE WITH CRSI STANDARDS.
- PLACEMENT, CLEARANCES, AND MINIMUM CONCRETE COVER FOR REINFORCING STEEL SHALL BE PROVIDED IN ACCORDANCE WITH ACI 318.
- PROVIDE CORNER BARS AT ALL GRADE BEAM CORNERS. CORNER BARS SHALL MATCH THE SIZE OF MAIN REINFORCEMENT AND SHALL HAVE 48" LEGS MINIMUM. PROVIDE ONE CORNER BAR AT EACH REBAR CORNER INTERSECTION.

FOUNDATION

- PREPARATION OF SUBGRADE SHALL BE IN STRICT ACCORDANCE WITH THE INSTRUCTIONS OUTLINED IN THE GEOTECHNICAL INVESTIGATION REPORT PREPARED BY GULF SOUTH ENGINEERING AND TESTING, INC. FILE NO. 16-041 DATED AUGUST 8, 2016. SIGNIFICANT DEVIATIONS FROM THE CONDITIONS DESCRIBED BY THE GEOTECHNICAL REPORT SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER IMMEDIATELY.
- CONTRACTOR SHALL COORDINATE WITH THE ARCHITECT AND GEOTECHNICAL ENGINEER AS NEEDED TO PERFORM COMPACTION TESTS, PROOFROLLING AND ALL OTHER MEANS OF QUALITY CONTROL RELATED TO THE SUBGRADE. REPORTS INDICATING SATISFACTORY SUBGRADE PREPARATION SHALL BE TRANSMITTED TO THE ENGINEER AND ARCHITECT PRIOR TO PLACEMENT OF THE FOUNDATION COMPONENTS.
- THE SLAB-ON-GRADE AREAS SHALL BE UNDERLAIN BY A 6" LAYER OF NO. 57 LIMESTONE.
- ALL INTERIOR SLAB-ON-GRADE AREAS SHALL HAVE A 15-MIL VAPOR RETARDANT BARRIER INSTALLED UNDER THE SLAB IN ACCORDANCE WITH THE SPECIFICATIONS.
- SAW-CUT CRACK CONTROL JOINTS ARE RECOMMENDED AT ALL INTERIOR SLAB AREAS. CONTRACTOR TO PROVIDE PROPOSED JOINT LAYOUT PRIOR TO POURING SLAB. CONCRETE JOINTS ARE RECOMMENDED AT 10' OC THROUGHOUT INTERIOR SLAB AREAS. SAW-CUT CRACK CONTROL JOINTS SHALL BE A MINIMUM OF 1/4 THE SLAB DEPTH AND INSTALLED WITHIN 12 HOURS OF CONCRETE PLACEMENT. COORDINATE WITH ARCHITECTURAL FLOOR FINISHES.

STRUCTURAL STEEL

- STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AISC CODE OF STANDARD PRACTICE LATEST EDITION AND THE AISC MANUAL OF STEEL CONSTRUCTION 14TH EDITION.
- ANCHOR BOLTS SHALL BE A MINIMUM OF 3/4-INCH DIAMETER ASTM F1554 GRADE 36 WITH A 12 INCH EMBEDMENT UNLESS OTHERWISE STATED ON THE DRAWINGS. ALL ANCHOR BOLTS SHALL BE HEADED BOLTS OR ROD THREADED AND NUTTED.
- MATERIALS
 - WIDE FLANGE SHAPES: ASTM A992
 - HSS SQUARE OR RECTANGULAR SHAPES (TUBE STEEL): ASTM A500 GRADE B, FY = 46 KSI
 - OTHER MISCELLANEOUS ANGLES, PLATES AND SHAPES: ASTM A36
- HIGH STRENGTH BOLTING FOR CONNECTIONS SHALL BE A MINIMUM OF 3/4 INCH DIAMETER ASTM A325 BOLTS INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS OF THE LATEST EDITION OF AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ALL CONNECTIONS SHALL BE SNUG TIGHTENED.
- STRUCTURAL STEEL SHALL BE BRACED DURING CONSTRUCTION UNTIL ALL STRUCTURAL BUILDING COMPONENTS ARE IN PLACE AND PERMANENTLY ATTACHED.
- ALL STEEL EXPOSED TO THE GENERAL WEATHER AND AS REQUIRED BY DETAILS WITHIN THE DRAWINGS SHALL BE HOT-DIPPED GALVANIZED.

STEEL JOISTS

- STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE SJI STANDARD SPECIFICATIONS, LOAD TABLES & WEIGHT TABLES FOR STEEL JOISTS AND JOIST GIRDERS, LATEST EDITION. JOISTS SHALL BE DESIGNED AND INSTALLED TO COMPLY WITH ALL OSHA REQUIREMENTS.
- STEEL ROOF JOISTS ARE SUBJECT TO A NET UPLIFT WIND LOAD AS FOLLOWS:
 - 34 PSF UPLIFT FOR A 5 FOOT STRIP AROUND THE PERIMETER
 - 21 PSF UPLIFT FOR REMAINDER OF AREA OF ROOF
- BRIDGING SHALL BE INSTALLED AS REQUIRED BY THE STANDARD SPECIFICATIONS FOR BOTH STABILITY DURING CONSTRUCTION AND RESISTANCE TO UPLIFT LOADS

COLD-FORMED STEEL

- ALL COLD-FORMED STEEL SHAPES SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE SPECIFICATIONS CONTAINED WITHIN THE LATEST AISI NORTH AMERICAN SPECIFICATIONS.
- COLD FORMED STUDS AND JOIST MEMBERS SHALL HAVE A MINIMUM OF STRENGTH OF FY=50 KSI. GENERAL EXTERIOR COLD-FORMED STUD WALLS SHALL BE A MINIMUM OF 600S162-68 STUDS AT 12 INCHES ON CENTER. WHERE NOT SPECIFIED, HEADERS SHALL BE A MINIMUM OF TWO 600S162-68 WITH 600T25-68 TOP AND BOTTOM ATTACHED TO 600S162-68S WITH #10 SCREWS AT 8 INCHES ON CENTER EACH SIDE.
- A MINIMUM OF TWO #10 SCREWS SHALL BE USED TO ATTACH MEMBERS TO ADJOINING FRAMING UNLESS OTHERWISE DETAILED ON DRAWINGS.
- ATTACH INTERIOR NON-LOAD BEARING COLD-FORMED STEEL WALLS TO UNDERSIDE OF ROOF DECK OR IF WALLS DO NOT EXTEND TO ROOF DECK, BRACE AT FOUR FEET ON CENTER WITH 362S162-43 STUD SECTIONS EXTENDING DIAGONALLY AT APPROXIMATELY 45 DEGREES TO BOTTOM CHORD OF BAR JOISTS. ALTERNATE BRACES IN EACH DIRECTION. ATTACH WITH 18 GAUGE SHEET METAL ANGLES WITH MINIMUM TWO #10 SCREWS AT EACH CONNECTION.
- THE ROOF DECK SHALL BE 1-1/2" DEEP 20 GAUGE DECK IN ACCORDANCE WITH THE STEEL DECK INSTITUTE DESIGN MANUAL. ALL DECK SHALL BE GALVANIZED.
- THE ROOF DECK SHALL BE INSTALLED AS FOLLOWS: FASTENING TO SUPPORT STRUCTURE- USE BUILDER OR ELCO TEXTRON #12 OR #14 SCREWS IN 36/4 PATTERN.

MASONRY

- ALL WORK TO BE IN STRICT ACCORDANCE WITH 2012 EDITION OF IBC AND LATEST EDITION OF ACI 530.1
- ALL CONCRETE BLOCK SHALL BE NOMINAL 8"X8"X16" HOLLOW NORMAL WEIGHT UNITS MANUFACTURED IN ACCORDANCE WITH ASTM C90 GRADING, OR AS SPECIFIED ON DRAWINGS.
- MORTAR SHALL BE TYPE M IN ACCORDANCE WITH ASTM C270. F'M SHALL BE A MINIMUM OF 2,800 PSI.
- ALL CELLS CONTAINING REBAR SHALL BE FILLED WITH MINIMUM 3,000 PSI CONCRETE. MAXIMUM HEIGHT OF PLACEMENT SHALL BE 24 INCHES.
- ALL UNREINFORCED SHALL BE CROUTED SOLID. GROUT FOR FILLING CELL CAVITIES SHALL BE IN ACCORDANCE WITH ASTM C476 AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI. MAXIMUM HEIGHT OF PLACEMENT SHALL BE 24 INCHES.
- ALL CONCRETE BLOCK SHALL BE SET IN A RUNNING BOND PATTERN, UNLESS OTHERWISE SPECIFIED ON THE DRAWINGS.
- REINFORCING REBAR SHALL BE ASTM A615 GRADE 60
- HORIZONTAL JOINT REINFORCEMENT SHALL BE LADDER TYPE PLAIN WELDED WIRE IN ACCORDANCE WITH ASTM A185. JOINT REINFORCEMENT SHALL BE PLACED AT MAXIMUM OF 16" ON CENTER.

WOOD FRAMING

- ROOF DECK AT THE MAIN TRANSVERSE JOIST SHALL BE 1/2" NOMINAL THICKNESS TONGUE AND GROOVE PLANKS. 3 INCH T&G DECKING SHALL BE TOENAILED TO JOIST WITH ONE 6D NAIL AND EDGE NAILED WITH ONE 6D NAIL AT EACH END.
 - MAIN ENTRANCE CANOPY ROOF DECKING SHALL BE LAPPED IN A SIMPLE SPAN LAYOUT WITH EACH COURSE CONNECTED TO THE ADJACENT WITH 8 INCH LONG SPANNA @ 30 INCHES THROUGH PREDRILLED EDGE HOLES AND WITH ONE SPIKE AT A DISTANCE NOT EXCEEDING 10 INCHES FROM THE END OF THE SPIKE. ALL T&G ROOF DECKING SHALL BE PRESSURE TREATED.
 - ANY TIMBER FRAMING IN THE CANOPY AREA SHALL BE A MINIMUM OF NO. 1 DOUGLAS FIR-LARCH QUALITY. MINIMUM ALLOWABLE BENDING STRESS EQUAL TO 1,000 PSI. MINIMUM MODULUS OF ELASTICITY EQUAL TO 1,700,000 PSI. TIMBER FRAMING SHALL BE PRESSURE AND FIRE TREATED AS REQUIRED BY ARCHITECTURAL PLANS.
- ALL CANOPY TRUSS MEMBERS SHALL BE FULL SAWN TO DIMENSIONS SHOWN ON PLANS AND CONNECTED WITH PARTIALLY EXPOSED STEEL PLATE BRACKETS AND THROUGH BOLTS. STEEL PLATE BRACKETS SHALL BE HOT DIP GALVANIZED.

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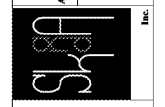
Open Plans

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CONSTRUCTION REISSUE SET

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