

**DESIGN AND LOADING**

THE STRUCTURAL DESIGN OF THIS BUILDING IS IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, 2015 EDITION.  
ALL BUILDING COMPONENTS ARE TO BE DESIGNED PER 2015 INTERNATIONAL BUILDING CODE. FABRICATION AND ERECTION OF ALL BUILDING ELEMENTS TO COMPLY WITH ALL OSHA AND OTHER APPLICABLE SAFETY REGULATIONS.

ROOF DEAD LOAD	20 PSF
LIVE LOAD	20 PSF
GROUND SNOW LOAD	10 PSF
ROOF SNOW	15 PSF
WIND VELOCITY 115 MPH PER ASCE 7-10. (EXPOSURE "B")	
IMPORTANCE FACTOR 1.0	GPI = ±0.18
MWFRS PRESSURE = 18 PSF	
COMPONENTS & CLADDING PRESSURE (ULTIMATE) = SEE TABLE	

**NOTES**

- FOR ROOF FRAMING DESIGN, A 5 PSF DEAD LOAD MAY BE UTILIZED TO OBTAIN THE NET UPLIFT PRESSURES.
- NO ALLOWABLE STRESS INCREASES ARE PERMITTED FOR ANY COMPONENT.
- WALL EDGE ZONES ARE 3'-10" FROM ALL CORNERS.

RISK CATEGORY= II	Sds= 24%g	Sd1= 9%g
SEISMIC DESIGN CATEGORY= C	BEARING WALLS—LIGHT FRAMED SHEAR WALLS	
SITE CLASS= D	DESIGN BASE SHEAR= 6 KIPS	
ANALYSIS: SIMPLIFIED LATERAL FORCE PROCEDURE		
Ss = 0.365		
S1 = 0.128		
Cs = 0.056		
R = 6.5		

**FOUNDATION NOTES**

FOUNDATION DESIGN IS BASED ON THE ASSUMED BEARING CAPACITY OF 2000 PSF. A GEOTECHNICAL ENGINEER SHALL CONFIRM ALL ASSUMED VALUES PRIOR TO CONSTRUCTION AND EXCAVATION.

ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR COMPACTED FILL HAVING AN ALLOWABLE BEARING CAPACITY OF 2,000 PSF. ALL FOUNDATIONS TO EXTEND 1'-6" MINIMUM BELOW ADJACENT FINAL GRADE.

ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY A SOILS TESTING LABORATORY TO VERIFY THE ALLOWABLE BEARING CAPACITY PRIOR TO PLACEMENT OF CONCRETE.

ALL COMPACTED FILL SHALL BE PLACED IN LAYERS NOT EXCEEDING 9" AND SHALL BE COMPACTED TO A MINIMUM DENSITY OF 95% UNDER FOOTINGS AND 90% UNDER SLABS AND PAVEMENTS AND OBTAINED IN ACCORDANCE WITH ASTM D-1557-78 OR AS SPECIFIED IN THE SOILS REPORT.

ALL SLAB ON GRADE AREAS SHALL BE PROOF ROLLED AND ALL SOFT SPOTS ENCOUNTERED SHALL BE REMOVED AND REPLACED TO FINISHED GRADE WITH APPROVED FILL MATERIAL IN ACCORDANCE WITH THE ABOVE PROCEDURE. COORD. W/ G.C., OWNER AND GEOTECH. ENGINEER FOR SOIL TREATMENT BELOW SLAB ON GRADE AREAS.

PLACE BACKFILL EQUALLY ON BOTH SIDES OF FOUNDATION WALLS OR GRADE BEAMS.

THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY IN THE EVENT THAT THE SOILS CONDITIONS ENCOUNTERED VARY FROM THOSE SHOWN ON THE BORING LOGS.

**CONCRETE AND REINFORCING**

**CONCRETE MATERIALS**

ALL CONCRETE SHALL BE IN ACCORDANCE WITH "AMERICAN CONCRETE INSTITUTE BUILDING CODE" (ACI 318) AND WITH "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301), LATEST EDITION.

ALL NORMAL WEIGHT CONCRETE (145 PCF) SHALL OBTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3,000 PSI EXCEPT AS FOLLOWS:

INTERIOR SLABS	3,500 PSI
EXTERIOR SLABS	4,000 PSI

REINFORCING BARS SHALL BE DEFORMED BARS OF NEW BILLET STEEL CONFORMING TO ASTM A615, GRADE 60. ALL REINFORCING AND ACCESSORIES SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI STANDARD 315-80 AND 315-80. REINFORCING BARS SHALL BE KEPT CLEAN AND FREE OF RUST.

ALL EXTERIOR CONCRETE, SUBJECTED TO FREEZING AND THAWING, SHALL ACHIEVE AN AIR CONTENT OF 4% - 7%. ALL INTERIOR FLOORS SHALL HAVE A MAXIMUM AIR CONTENT OF 3%. TEST CYLINDERS SHALL BE MADE AND TESTED AS OUTLINED IN ACI 301 OR AS PER ARCHITECTURAL SPECIFICATIONS.

PORTLAND CEMENT SHALL CONFORM TO ASTM C150 "SPECIFICATIONS FOR PORTLAND CEMENT" TYPE I OR II. FLY ASH OR SLAG SHALL BE USED IN ALL CONCRETE REQUIRED TO BE SULFATE RESISTANT FOR ALL MODERATE EXPOSURE.

AGGREGATES FOR CONCRETE OF NORMAL WEIGHT SHALL CONFORM TO ASTM C33 "SPECIFICATIONS FOR CONCRETE AGGREGATES". FINE AGGREGATE SHALL BE NATURAL, CLEAN, HARD SAND. WATER REDUCING/HIGH RANGE WATER REDUCING, AND MID-RANGE WATER REDUCING ADMIXTURES SHALL CONFORM TO ASTM C494. "SPECIFICATIONS FOR CHEMICAL ADMIXTURES FOR CONCRETE" AND SHALL CONTAIN NOT MORE THAN 0.05% CHLORIDE IONS.

MIXING WATER USED IN THE CONCRETE SHALL BE FRESH, CLEAN AND DRINKABLE.

CONCRETE MIXES SHALL BE PROPORTIONED TO ACHIEVE A MAXIMUM SLUMP OF 8" FOR CONCRETE CONTAINING HRWR, 6" FOR CONCRETE CONTAINING A MID-RANGE WATER REDUCING ADMIXTURE, OR 4" FOR OTHER CONCRETE. ALL MIXES SHALL HAVE A WATER SLUMP OF 2" - 3" (3" - 4" FOR CONCRETE RECEIVING A "DRY-SHAKE" HARDENER).

NO ALUMINUM OF ANY TYPE SHALL BE ALLOWED IN THE CONCRETE UNLESS COATED TO PREVENT ALUMINUM-CONCRETE REACTION. THIS INCLUDES PUMPING THROUGH ALUMINUM PIPING.

ALL CONCRETE SUBJECT TO FREEZING AND THAWING SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.50 (4000 PSI AT 28 DAYS OR MORE). ALL CONCRETE SUBJECT TO DEICERS AND/OR REQUIRED TO BE WATER TIGHT SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45 (4000 PSI AT 28 DAYS OR MORE). ALL REINFORCING CONCRETE SUBJECT TO BRACKISH WATER, SALT SPRAY OR DEICERS SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.40 (5000 PSI AT 28 DAYS OR MORE). ALL TROUBLE FINISHED INTERIOR SLABS, SUBJECT TO VEHICULAR TRAFFIC SHALL HAVE A MAXIMUM W/C RATIO OF 0.53.

ALL MIX DESIGNS SHALL BE PREPARED IN ACCORDANCE WITH ACI 318, SECTION 4.2.3 ON THE BASIS OF PREVIOUS FIELD EXPERIENCE OR TEST MIXES. ALL MIXES SHALL HAVE A MAXIMUM W/C OF 0.53, AND A MINIMUM OF 7% CUMULATIVE FINER THAN NO. 200 SIZE AGGREGATE IN ALL MIXES FOR SLABS ON GRADE.

SLAB ON GROUND OR FLY ASH SHALL BE SUBSTITUTED FOR PORTLAND CEMENT, NOT EXCEEDING 15% FOR SUBSTITUTION AND 15% FOR WEIGHT.

**CONCRETE MIXING**

ALL CONCRETE SHALL BE READY-MIXED CONCRETE, BATCHED, MIXED AND TRANSPORTED IN ACCORDANCE WITH ASTM C94, "SPECIFICATIONS FOR READY-MIXED CONCRETE". PLANT EQUIPMENT AND FACILITIES SHALL CONFORM TO THE "CHECK LIST FOR CERTIFICATION OF READY MIXED CONCRETE PRODUCTION FACILITIES" OF THE NATIONAL READY MIXED CONCRETE ASSOCIATION.

THE READY-MIXED CONCRETE PRODUCER SHALL SUBMIT DUPLICATE DELIVERY TICKETS, ONE FOR THE CONTRACTOR AND ONE FOR THE ENGINEER WITH EACH LOAD OF CONCRETE DELIVERED TO THE JOB SITE.

DELIVERY TICKET SHALL PROVIDE THE FOLLOWING INFORMATION:

**DATE**  
NAME OF READY-MIX CONCRETE PLANT  
CONTRACTOR  
JOB LOCATION  
TYPE AND BRAND OF CEMENT  
CEMENT CONTENT IN POUNDS PER CUBIC YARD  
WATER CONTENT  
TRUCK NUMBER  
TIME DISPATCHED, AND TIME UNLOADED  
AMOUNT OF CONCRETE IN LOAD IN CUBIC YARDS  
ADMIXTURES IN CONCRETE, IF ANY  
TYPE, QUANTITY, AND MAXIMUM SIZE OF AGGREGATE  
WATER ADDED AT JOB SITE, IF ANY

**CURING AND PROTECTION**

BEGINNING IMMEDIATELY AFTER PLACEMENT, CONCRETE SHALL BE PROTECTED FROM PREMATURE DRYING AND MECHANICAL INJURY AND SHALL BE MAINTAINED ABOVE 50 DEGREES F WITH MINIMAL MOISTURE LOSS FOR 7 DAYS. THIS TEMPERATURE REQUIREMENT MAY BE REDUCED TO 3 DAYS WHEN HIGH EARLY STRENGTH CONCRETE IS USED.

FOR CONCRETE SURFACES NOT IN CONTACT WITH FORMS, ONE OF THE FOLLOWING PROCEDURES SHALL BE APPLIED IMMEDIATELY AFTER COMPLETION OF PLACEMENT AND FINISHING. PONDING OR CONTINUOUS SPRINKLING; APPLICATION OF ABSORPTIVE MATS OR FABRIC; APPLICATION OF WATERPROOF SHEET MATERIALS; APPLICATION OF A CURING COMPOUND.

MOISTURE LOSS FROM SURFACES PLACED AGAINST WOODEN OR METAL FORMS EXPOSED TO HEATING BY THE SUN SHALL BE MINIMIZED BY WETTING.

CURING SHALL BE CONTINUED FOR AT LEAST 7 DAYS FOR ALL CONCRETE.

PROTECTIVE MEASURES SUCH AS WIND BREAKS, SHADING, FOG SPRAYING, PONDING, SPRINKLING, AND WET COVERING SHALL BE TAKEN WHEN THE TEMPERATURE OF THE CONCRETE AT THE TIME OF PLACING IS GREATER THAN 85 DEGREES F.

DURING THE CURING PERIOD, CONCRETE SHALL BE PROTECTED FROM DAMAGING MECHANICAL DISTURBANCES SUCH AS LOAD STRESSES, HEAVY SHOCK AND EXCESSIVE VIBRATION. ALL FINISHED CONCRETE SURFACES SHALL BE PROTECTED FROM DAMAGE BY CONSTRUCTION EQUIPMENT, MATERIALS OR METHODS OR BY APPLICATION OF CURING PROCEDURES.

ALL SLABS MAY BE CURED WITH A STRIPPABLE CURING COMPOUND SUCH AS KUREX DR VOX OR KUREX W VOX BY THE EUCLID CHEMICAL COMPANY OR BY AN APPROVED CONTINUOUS MOIST CURING METHOD.

**CONCRETE PLACING**

WELDING OF REINFORCING BARS SHALL NOT BE ALLOWED.

PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCEMENT AT POSITIONS SHOWN ON THE PLANS AND DETAILS. PLASTIC COATED ACCESSORIES SHALL BE USED IN ALL EXPOSED CONCRETE WORK.

FOUNDATION WALLS AND GRADE BEAMS SHALL HAVE A MINUM OF TWO #5 BARS TOP AND BOTTOM CONTINUOUS, UNLESS OTHERWISE NOTED.

PLACE TWO #5 BARS (EACH FACE) WITH 2'-0" PROJECTION AROUND ALL OPENINGS IN CONCRETE, UNLESS OTHERWISE NOTED.

REINFORCEMENT SHALL BE CONTINUOUS ACROSS JOINTS AND AROUND CORNERS OR SPLICES. SHALL BE PROVIDED IN ACCORDANCE WITH ACI 315-80 OR ACI 315-80 OR CORNER BARS SHALL BE PROVIDED AT ALL WALL CORNERS EQUAL TO THE HORIZONTAL WALL REINFORCEMENT.

MINIMUM LAP OF REINFORCEMENT BARS SHALL BE EQUIVALENT TO A CLASS "B" SPLICE, UNLESS OTHERWISE NOTED.

CONTRACTION (CONTROL) JOINTS FOR SLABS ON GRADE SHALL BE IN A SQUARE PATTERN (PANEL SHALL NOT EXCEED 1.5 TO 1 RATIO). MAXIMUM JOINT SPACING SHALL BE 16 TIMES SLAB THICKNESS UNLESS OTHERWISE NOTED.

THE GENERAL CONTRACTOR SHALL CHECK WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND ELECTRICAL DRAWINGS AND THE SUB-CONTRACTOR FOR OPENING SLEEVES, ANCHORS, HANGERS, INSERTS, SLAB DEPRESSIONS AND OTHER ITEMS RELATED TO THE CONCRETE WORK AND SHALL ASSUME RESPONSIBILITY FOR THEIR PROPER LOCATION, FITCH CONCRETE SLABS AS REQUIRED TO FLOOR DRAINS.

NO STRUCTURAL CONCRETE OR SLAB CONCRETE SHALL BE PLACED UNTIL THE CONCRETE DESIGN MIXES, THE CONCRETE PLACEMENT PROCEDURE, THE LOCATION OF CONSTRUCTION AND CONTROL JOINTS, AND THE SETTING OF REINFORCEMENT IS REVIEWED BY THE ENGINEER.

NO ALUMINUM OF ANY TYPE SHALL BE ALLOWED IN THE CONCRETE UNLESS COATED TO PREVENT ALUMINUM-CONCRETE REACTION. THIS INCLUDES PUMPING THROUGH ALUMINUM PIPE.

COLD WEATHER CONCRETING SHALL BE DONE IN ACCORDANCE WITH ACI 308 LATEST EDITION. HOT WEATHER CONCRETING SHALL BE DONE IN ACCORDANCE WITH ACI 305, LATEST EDITION.

**STRUCTURAL STEEL**

STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE". STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS (UNLESS OTHERWISE NOTED).

STRUCTURAL SHAPES AND PLATES	A-36 U.N.O.
STRUCTURAL PIPE	A-53 TYPE E GRADE B

ALL WELDING ELECTRODES SHALL BE E70-XX LOW HYDROGEN. ALL SHOP AND FIELD WELDING SHALL BE MADE IN ACCORDANCE WITH A.W.S. D11-81 "CODE FOR WELDING IN BUILDING CONSTRUCTION" AND SHALL BE MADE BY CERTIFIED WELDERS. FILLER METAL FOR ALL FULL PENETRATION WELDS SHALL HAVE A MINIMUM CWN TOUGHNESS OF 20 FT-LBS AT MINUS 20 DEGREES AND AT 0 DEGREES F. AND A MIN. CWN TOUGHNESS OF 40 FT-LBS AT 70 DEGREES F.

**WOOD**

ALL WOOD TO WOOD CONNECTIONS SHALL EMPLOY METAL ANCHORS. ANCHORS SPECIFIED ARE BY SIMPSON STRONG TIE CO. INSTALL ALL THE MANUFACTURER SPECIFIED NAILS IN THE MEMBERS CONNECTED. ALTERNATE MANUFACTURER CONNECTORS MAY BE SUBMITTED FOR REVIEW.

ALL BOLTED OR NAILED STRAP CONNECTIONS SHALL HAVE AN EQUAL NUMBER OF BOLTS OR NAILS EACH SIDE AT THE SPLICE JOINT. THE FIRST BOLT OR NAIL FROM EACH SIDE OF THE SPLICED OR TREATED MEMBER SHALL BE EQUAL DISTANCE FROM THE SPLICE. STRAPS USING 16d NAILS ON 2X MATERIAL SHALL BE INSTALLED ON THE 1 1/2" EDGE OF THE MEMBER.

ALL WOOD TO WOOD CONNECTIONS FOR PRESERVATIVE TREATED WOOD SHALL BE AS FOLLOWS: G185 HOT-DIP GALVANIZED CONNECTORS AND FASTENERS SHALL BE USED FOR ALL WOOD TREATED WITH DOT SODIUM BORATE (SBX), ALKALINE COPPER QUAT ACQ-C AND ACQ-D (CARBONATE), COPPER AZOLE (CBA-A AND CA-B), OR SBX (DOT) WITH N650 USED FOR ALL OTHER TYPES OF PRESERVATIVE TREATED WOOD. THE CONNECTORS AND FASTENERS SHALL BE OF THE SAME MATERIAL FOR ALL WOOD TO WOOD CONNECTIONS FOR PRESERVATIVE TREATED WOOD.

MOISTURE CONTENT SHALL NOT BE GREATER THAN 19%.

ALL STRUCTURAL DIMENSIONAL LUMBER SHALL BE SOUTHERN PINE STAMPED IN ACCORDANCE WITH STANDARD GRADING RULES WITH MINIMUM GRADES AS SHOWN BELOW.

MEMBER	GRADE
2x6 STUDS	#2
ALL OTHER MEMBERS	

ROOF SHEATHING SHALL BE AS NOTED ON PLAN.  
ALL SHEATHING SHALL HAVE SPAN RATINGS AND THICKNESS AS INDICATED. ALL WALL SHEATHING SHALL BE FASTENED TO ALL FRAMING W/ 10d NAILS 96" O.C. U.N.O. IN SHEAR WALL SCHEDULE.

LAY UP WITH MINIMUM 1/8" CLEAR BETWEEN PANELS TO ALLOW FOR EXPANSION. SECURE WITH GALVANIZED NAILS TO EACH SUPPORT AS NOTED IN PLAN.

SEE SHEARWALL NOTES FOR SHEATHING NAILING PATTERNS. ALL PANEL JOINTS IN SHEARWALLS TO BE BLOCKED W/ 2x BLOCKING. NO OVERDRIVEN NAILS WILL BE ACCEPTABLE IN SHEARWALL OR ROOF.

SILL PLATES AND ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESURE TREATED. (P.P.T.) ALL ANCHOR BOLTS THRU SILL PLATES SHALL BE HOT DIP GALVANIZED.

**PREFABRICATED WOOD TRUSSES**

ALL PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED TO MEET THE LOADINGS SPECIFIED. FABRICATION AND ERECTION SHALL BE PER TRUSS PLATE INSTITUTE RECOMMENDATIONS AS CONTAINED IN THE APPROPRIATE PUBLICATIONS.

SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT.

COORDINATE TRUSS WEB CONFIGURATION WITH MECHANICAL DUCTWORK AS INDICATED ON MECHANICAL SHEETS. PROVIDE CLEAR SPACE BETWEEN WEBS AS REQUIRED TO INSTALL DUCTWORK.

PROVIDE ALL TEMPORARY AND PERMANENT TRUSS BRACING AND BRIDGING AS REQUIRED AND SHOWN ON THE TRUSS MANUFACTURERS SHOP DRAWINGS.

PROVIDE 2x4 DIAGONAL BRACING AT ROOF TRUSS CONNECTIONS WHERE INDICATED ON SECTIONS, DETAILS, OR TRUSS ELEVATION SCHEMATICS.

INSTALL STRONG BACKS, BRACING AND BRIDGING PRIOR TO CONSTRUCTION AND AS TRUSSES ARE ERECTED.

INSTALL 2x4 CONTINUOUS BOTTOM CHORD BRACING AT EFFECT OCCUPANCY AT ALL AREAS WHERE A RIGID CEILING IS NOT ATTACHED DIRECTLY TO THE TRUSS BOTTOM CHORD.

ALL HURRICANE STRAPS SHALL BE INSTALLED PRIOR TO SHEATHING.

REFER ARCHITECTURAL DRAWINGS FOR TRUSS PROFILES.

ALL TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED BY THE DELEGATED TRUSS ENGINEER.

**SPECIAL INSPECTION**

CONTRACTOR TO PROVIDE SPECIAL INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF THE IBC 2012 FOR THE FOLLOWING ITEMS:  
ALL CONCRETE PER SECTION 1704.4 & TABLE 1704.4

CONCRETE BLOCK MASONRY PER TABLE 1704.5  
SPRAY APPLIED FIRE-PROOFING PER SECTION 1705.12  
CHEMICAL ANCHORS

SPECIAL INSPECTION SERVICES SHALL BE PROVIDED BY A CERTIFIED TESTING LABORATORY APPROVED BY THE OWNER.

**MISCELLANEOUS**

ALL DIMENSIONS ON STRUCTURAL DRAWINGS TO BE CHECKED AGAINST ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS BY THE GENERAL CONTRACTOR AND ANY DISCREPANCIES ARE TO BE REPORTED TO THE ARCHITECT IMMEDIATELY.

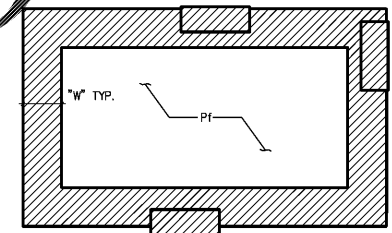
THE CONTRACTOR SHALL ASSUME RESPONSIBILITY, UNRELIEVED BY REVIEW OF SHOP DRAWINGS OR PERIODIC OBSERVATION OF CONSTRUCTION, FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS, FOR FABRICATION PROCESSES AND CONSTRUCTION TECHNIQUES, AND FOR SAFE CONDITIONS ON THE JOB SITE.

DO NOT SCALE DRAWINGS. ALL DIMENSIONS AND FIT SHALL BE DETERMINED AND VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCEMENT OF WORK.

DETAILS NOT FULLY OR SPECIFICALLY SHOWN SHALL BE OF SAME NATURE AS OTHER SIMILAR CONDITIONS.

**COMPONENTS & CLADDING PRESSURE (ULTIMATE) = SEE TABLE**

	Walls:	Design Wind Pressure (psf)					Effective Wind Area (sqft)							
		10	20	30	100	200	500	10	20	30	100	200	500	
Interior	Area 4	+	18.2	17.4	16.6	16.0	16.0	16.0	-19.7	-18.9	-18.1	-17.0	-16.2	-16.0
	Edge	Area 5	+	18.2	17.4	16.6	16.0	16.0	16.0	-24.3	-22.6	-21.0	-18.9	-17.3
Roof:	Interior	Area 1	+	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
	Edge	Area 2	+	-19.9	-19.4	-18.7	-18.2	-18.2	-18.2	-33.4	-29.8	-25.1	-21.6	-21.6
Corner	Area 3	+	18.2	17.4	16.6	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
	Edge	Area 3	+	-33.4	-29.8	-25.1	-21.6	-21.6	-21.6	-33.4	-29.8	-25.1	-21.6	-21.6
Overhang:	Interior	Area 1	+	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
	Edge	Area 2	+	-28.6	-28.1	-27.5	-27.0	-27.0	-27.0	-28.6	-28.1	-27.5	-27.0	-27.0
Corner	Area 3	+	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
	Edge	Area 3	+	-47.2	-37.0	-23.6	-16.0	-16.0	-16.0	-47.2	-37.0	-23.6	-16.0	-16.0
Parapet:		Design Wind Pressure (psf)					Effective Wind Area (sqft)							
		10	20	30	100	200	500	10	20	30	100	200	500	
Edge	Area 2	+	56.4	51.0	43.9	38.5	37.5	36.1	-39.5	-37.0	-34.8	-32.8	-30.8	-28.2
	Corner	Area 3	+	56.4	51.0	43.9	38.5	37.5	36.1	-42.1	-42.1	-38.2	-35.2	-32.2



**SHEAR WALL DESIGN NOTES:**

- SHEAR WALL SHEATHING: 1/2" BLOCKED APA RATED STRUCTURAL I GRADE, EXPOSURE I, PLYWOOD OR 7/16" OSB.
- NAIL TYPE: 10d GALVANIZED.
- NAIL SPACING: SEE SCHEDULE
- END POST REQUIREMENTS AT SHEAR WALLS: (2) 2x6 @ SW1 (5) 2x6 @ SW2
- HOLDOWN ANCHORS ARE SPECIFIED IN THE SCHEDULE FOR EACH SHEAR WALL. HOLDOWN ANCHORS SHALL BE POST INSTALLED W/ SIMPSON SET ADHESIVE.
- FOR BOTTOM SILL PLATE TO FOUNDATION CONNECTORS SEE SCHEDULE FOR TYPE AND SPACING.
- SEE FOUNDATION PLAN FOR SHEAR WALL (SW'S) LOCATION.
- ALL HOLDOWN TYPES (EXCEPT INTERMED. A.B.'S) REFER TO SIMPSON STRONG-TIE" MODEL No.'S (OR APPROVED EQUAL).
- WHERE SHEAR WALL ENDS @ A STEEL COLUMN, ATTACH WALL TO COLUMN W/ POWER ACTUATED NAILS @ 8" O.C.
- INTERMEDIATE SILL ANCHORS SHALL BE 5/8" DIA. THREADED RODS W/ 5" EMBED POST INSTALLED W/ SIMPSON SET EPOXY. SEE SCHEDULE FOR SILL ANCHOR SPACING.
- HOLDOWN ANCHORS TO BE CAST-IN-PLACE IF CMU STEMWALL OPTION IS USED. (PAB5 W/ 5" EMBED FOR 5/8" DIA. PAB7 W/ 7" EMBED FOR 7/8" DIA., AND PAB8 W/ 9" EMBED FOR 1" DIA. ANCHORS.)
- SEE 4/S4, 8/S4 FOR FORCE TRANSFER AROUND OPENINGS REQUIREMENTS.

**SHEAR WALL SCHEDULE**

SHEAR WALL MARK	SHEATHING TYPE	END POST HOLDOWN MODEL NO.	HOLDOWN ANCHORS (NOTE 7)	EMBED. DEPTH	INTERMEDIATE SILL ANCHORS AND SPACING	NAILING PATTERN		REMARKS
						EDGES	INTERMEDIATE PANELS	
"SW1"	SEE DESIGN NOTE 1	H0J2	5/8"ø	8"	5/8"ø A.B. @ 24"OC	6"OC	12"OC	
"SW2"	SEE DESIGN NOTE 1	H014	1"ø	12"	5/8"ø A.B. @ 16"OC	3"OC	12"OC	NOTE 12

**Original Plans**

**GENERAL STRUCTURAL NOTES**

NRD Project # 16399

**BRITT PETERS AND ASSOCIATES, INC.**  
consulting engineer  
999 Waterside Dr.  
Suite 2002  
Norfolk, VA 23513  
(757) 965-5710  
www.britt-peters.com  
A Job # 2013-0373

ARCHITECT: **Hardees**  
3001 CARROLLTON HILL RD., SUITE 100  
MORRISVILLE, NORTH CAROLINA 27560  
PH: 919.544.7251 Fax: 919.544.9392  
A Division of Liberty Architecture

**REVISIONS**

NO.	DATE	DESCRIPTION
1	5/29/2017	COORDINATION

**GENERAL STRUCTURAL NOTES**

CHECKED BY: [Signature]  
CREATED BY: [Signature]  
APPROVED BY: [Signature]

CHEK SITE ID: \_\_\_\_\_

DATE: 2-28-17

SHEET NUMBER: **S6**