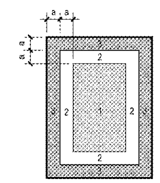


FLAT ROOF 7-27 DEG.

a = 6' - 0" MIN.

(160 MPH) ALLOWABLE (ASD) WIND PRESSURES FOR COMPONENTS & CLADDING (ASCE 7-10)						
EFFECTIVE AREA (SQ FT)	ZONE 1		ZONE 2		ZONE 3	
	PRESSURE	SUCTION	PRESSURE	SUCTION	PRESSURE	SUCTION
10	46.1 PSF	-44.3 PSF	69 PSF	-67.1 PSF	92 PSF	-109.4 PSF
20	46.1 PSF	-44.3 PSF	69 PSF	-67.1 PSF	92 PSF	-109.4 PSF
50	46.1 PSF	-44.3 PSF	69 PSF	-67.1 PSF	69 PSF	-67.1 PSF
100	46.1 PSF	-44.3 PSF	69 PSF	-67.1 PSF	69 PSF	-67.1 PSF
A ≥380	46.1 PSF	-44.3 PSF	46.1 PSF	-44.3 PSF	46.1 PSF	-44.3 PSF

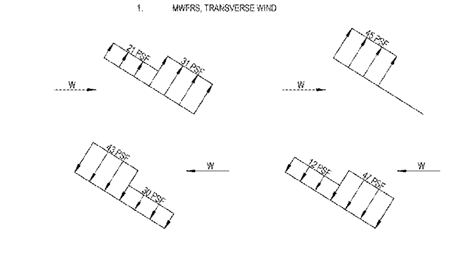
NOTES:
1. 'a' DENOTES EDGE STRIP = 6'-0"
2. POSITIVE & NEGATIVE SIGNS SIGNIFY PRESSURES ACTING TOWARDS AND AWAY FROM THE BUILDING SURFACES, RESPECTIVELY .
3. WIND PRESSURES INDICATED ABOVE ARE ALLOWABLE STRESS DESIGN WIND PRESSURES. DIVIDE THE PRESSURES BY A FACTOR OF 0.6 TO OBTAIN STRENGTH DESIGN WIND PRESSURES

COMPONENTS & CLADDING PRESSURES

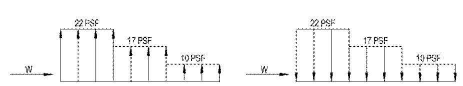
NON-BLDG STRUCTURE WIND LOAD

A. MONOSLOPE OPEN ROOF

1. MWFRS, TRANSVERSE WIND



2. MWFRS, LONGITUDINAL WIND



WIND PRESSURES & VELOCITIES GIVEN HEREIN ARE NOMINAL (V nom)

3. ROOF WL RESSURE (MWFRS & C&C) ARE THE COMBINED EFFECT OF WIND IN BOTH SURFACES (TOP & BOTTOM)

B. 2D TRUSSES, 53 PSF ON SOLID AREA, EACH TRUSS SIMULTANEOUSLY

C. SIGN, 53 PSF. CASES A,B, AND C CONSIDERED

D. COLUMNS, SQUARE, 53 PSF ALL COLUMNS SIMULTANEOUSLY IN THE WIND DIRECTION BEING CONSIDERED

GENERAL

A. ALL WORK SHALL BE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2014 AS ADOPTED AND SUPPLEMENTED BY LOCAL REGULATION.

B. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ARCHITECT/ENGINEER OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH WORK.

C. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETE DESIGN OF THE STRUCTURE. THEY DO NOT INDICATE THE MEANS AND METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKERS, OR THER PERSONS DURING CONSTRUCTION.

D. OBSERVATION VISITS TO THE SITE BY EOR OR REPRESENTATIVES OF THE EOR MAY BE MADE DURING CONSTRUCTION. ANY SUPPORT SERVICES PERFORMED HEREIN SHALL BE DISTINGUISHED FROM INSPECTION AND.OR TESTING SERVICES PERFORMED BY OTHERS, AND ARE NOT TO BE CONSTRUED AS SUPERVISION AND/OR MANAGEMENT OF CONSTRUCTION.

E. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTRUAL MEMBERS AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT/ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STAILITY OF THE STRUCTURE OR CAUSE DISTRESS WITHIN THE STRUCTURE.

F. CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED IN THE GENERAL NOTES. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.

G. DRAWINGS ARE NOT TO BE SCALED.

H. FOR ACTUAL ELEVATION DATUMN ON FIRST FLOOR (REF. ELEV. 0'-0" HEREIN). SEE SITE PLAN.

I. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO THE DETAILS PRESENTED, SIMILAR DETAILS SHALL BE USED SUBJECT TO THE REVIEW OF ENGINEER OF RECORD.

J. SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS, AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR. SUBSEQUENT DOCUMENTATION/REQUESTS TO BUILDING ENGINEER OF RECORD FROM GENERAL CONTRACTOR SHALL INCLUDE EVALUATION OF DEVIATIONS BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.

K. THE MOST STRINGENT REQUIREMENTS APPLY IN CASE OF CONFLICT BETWEEN SPECIFICATIONS, STANDARDS, CODES AND DRAWINGS.

L. DESIGN DATA

ROOF LIVE LOADS	20 PSF
DEAD LOADS (D)	
ROOF:	16 PSF, INCLUDING STRUCTURE
SIGN:	20 PSF, INCLUDING STRUCTURE
COLUMNS & TRUSSES	STEEL SELF WEIGHT (490 PCF)
UPLIFT LOAD COMBINATIONS (D)	
ROOF:	10 PSF, INCLUDING STRUCTURE
SIGN:	10 PSF, INCLUDING STRUCTURE
COLUMNS & TRUSSES:	STEEL SELF WEIGHT (490 PCF)
WIND DESIGN:	(ASCE 7-10)
ULTIMATE DESIGN WIND SPEED (V ult):	158 MPH
NOMINAL DESIGN WIND SPEED (V nom):	123 MPH
RISK CATEGORY:	II
WIND EXPOSURE:	C
ENCLOSURE CLASSIFIATION:	OPEN
INTERNAL PRESSURE COEFF: +/-	+/- 0.0

SUBMITTALS

A. SHOP DRAWING REVIEW IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT. CORRECTIONS OR COMMENTS MADE ON THIS REVIEW DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR ERRORS AND OMMISIONS, AND FROM COMPLIANCE WITH THE PLANS AND SPECIFICATIONS. CORRECTIONS OR COMMENTS DO NOT AUTHORIZE AN INCREASE IN THE CONSTRUCTION BUDGET.

B. APPROVAL OF SHOP DRAWINGS DOES NOT INDICATE ACCEPTANCE OF DEVIATIONS FROM CONTRACT DOCUMENTS OR PREVIOUS SHOP DRAWING REVIEW, UNLESS SPECIFICALLY NOTED THEREIN BY ENGINEER OF RECORD.

C. ANY CHANGES TO THE DESIGN CONCEPT SHOWN IN CONTRACT DOCUMENTSHALL BE SUBMITTED IN WRITING AND APPROVED BY THE ARCHITECT AND ENGINEER PRIOR TO SUBMITTING SHOP DRAWINGS. ALL SUCH CHANGES SHALL BE "BUBBLED" ON THE SHOP DRAWINGS AND REFERENCED TO THE PROPER R.F.I.

D. SUBMITTALS SHALL CONFORM TO THE REQUIREMENTS OF THE CONTRACT DRAWINGS (REFERENCE ITEM C ABOVE FOR EXCEPTION). NON-CONFORMING OR NON-REVIEWED SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.

1. SHOP DRAWINGS SHALL BE "APPROVED", SIGNED AND DATED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO ENGINEER AND ARCHITECT OF RECORD.
2. SHOP DRAWINGS SHALL NOT CONTAIN REPRODUCTIONS OF THE CONTRACT DRAWINGS.
3. SUBMITTAL REQUIREMENTS:

SHOP DRAWINGS/DATA TO BE SUBMITTED FOR APPROVAL

CONCRETE MIX DESIGNS
REINFORCING STEEL
STRUCTURAL STEEL

SHOP DRAWINGS/DATA TO BE SUBMITTED FOR APPROVAL. SUBMITTALS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENCED IN THE STATE OF THE PROJECT

PRE-ENGINEERED METAL PANEL DECKING
PRE-ENGINEERED LAM. PANEL DECKING

E. PROVIDE COPIES OF MANUFACTURERS LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.

FOUNDATIONS/SITE WORK

A. FOUNDATION DESIGN IS BASED UPON THE FOLLOWING SOILS REPORT:

NO REPORT PROVIDED:
PRIOR TO CONSTRUCTION, CONTRACTOR SHALL RETAIN THE SERVICES OF /QUALIFIED GEOTECHNICAL ENGINEER. THE GENERAL CONTRACTOR HEREIN IRESPONSIBLE FOR PERFORMING ALL EARTHWORK OPERATIONS IN STRICT ACCORDANCE WITH GEOTECHNICAL ENGINEERING REQUIREMENTS.

IF THE FOUNDATION RECOMMENDATIONS AN/OR DESIGN VALUES DEFFER FROM THAT ASSUMED, MODIFICATIONS TO THE DRAWINGS MAY BE REQUIRED.

DO NOT PLACE CONCRETE PRIOR TO GEOTECHNICAL APPROVAL.

B. NET ALLOWABLE SOIL PRESSURE ASSUMED TO BE 2,500 PSF AT 6'-0" DEPTH (REF A,. ABOVE). SHALLOW FOUNDATION SYSTEM IS ASSUMED TO BE ADEQUATE. ESTIMATED SETTLEMENT ASSUMED TO BE NO GREATER THAN 1" OVERALL AND 1/2"DIFFERENTIAL SETTLEMENT. IT IS ASSUMED THAT ALL PROBLEMATIC SOILS (ORGANIC, EXPANSIVE, ETC.) WILL BE REMOVED AND REPLACED WITH SUITABLE STRUCTURAL FILL.

C. ANY FILL REQUIRED TO BACKFILL EXCAVATED AREA OR ACHIEVE FINISHED GRADE IN STRUCTURAL AREAS SHALL BE AS INDICATED BY GEOTECHNICAL ENGINEER. THEILL SHALL BE PLACED IN LEVEL LIFTS NOT EXCEED 12 INCHES LOOSE THICKNESS AND COMPACTED TO A MINIMUM OF 95% OF THE SOIL'S MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM SPECIFICATION D-1557.

D. IN-PLACE DENSITY TESTS SHALL BE PERFORMED BY AN EXPERIENCED ENGINEERING TECHNICIAN. TESTS SHALL BE PERFORMED FOR EACH 2,000 SQUARE FEET, IN EVERY COLUMN FOOTING LOCATION AND EACH 50 FEET ALONG WALL FOOTINGS. COPIES OF THE TEST REPORTS SHALL BE FURNISHED TO THE STRUCTURAL ENGINEER.

E. REMOVE FREE WATER FROM EXCAVATIONS BEFORE PLACING CONCRETE. WATER TABLE ASSUMED BELOW 6' PRECLUDING BUOYANCY CONDITONS ON FOOTINGS.

F. CAUTION SHOULD BE USED WHEN OPERATING VIBRATORY COMPACTING EQUIPMENT NEAR THE EXISTING STRUCTURE TO AVOID THE RISK OF DAMAGE TO THE STRUCTURE.

G. REFER TO ARCHITECTURE DRAWINGS FOR ANY NECESSARY WATERPROOFING REQUIREMENTS.

REINFORCED CONCRETE

A. CONCRETE WORK SHALL CONFORM TO ACI-08 SPECIFICATIONS, CODES AND STANDARDS.

B. CAST-IN-PLACE CONCRETE, UNLESS OTHERWISE NOTED:

PORTLAND CEMENT PER ASTM C150
PROPORTIONED FOR MAX. ALLOWABLE UNIT SHRINKAGE OF 0.03%
25% MAXIMUM FLYASH BY WEIGHT

FOOTINGS & PEDESTAL
4000 PSI 28-DAY COMPRESSIVE STRENGTH, NORMAL WEIGHT,
MAXIMUM AGGRETGATE SIZE OF 1", 0.55 MAX. W.C. 3"-4" SLUMP

C. CONCRETE COVER OVER REINFORCEMENT:

FOOTINGS & PEDESTAL
3"

D. REINFORCING, UNLESS OTHERWISE NOTED:

1. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.
2. WHERE CONTINUOUS REINFORCING IS SPECIFIED, SUCH REINFORCING MAY BE SPLICED WHERE APPROVED IN WRITTING BY THE ENGINEER OF RECORD.
3. AT CHANGES IN DIRECTION OF CONCRETE WALLS, BEAMS, AND FOOTINGS, PROVIDE CORNER BARS OF SAME SIZE, QUANTITY AND SPACING AS HORIZONTAL STEEL.
4. LAP SPLICES SHALL BE 48 BAR DIAMETERS, WIRED TOGETHER.

E. CONTRACTOR SHALL EMPLOY A QUALIFIED TESTING LABORATORY TO PREPARE ONE SET OF FOUR TEST CYLINDERS PER DAY OR ONE SET FOR EACH 50 CUBIC YARDS POURED MAXIMUM. LABORATORY REPORT SHALL BE FURNISHED TO THE STRUCTURAL ENGINEER SHOWING STRENGTH OF CONCRETE AT 7 (1 CYLINDER) AND 28 (2 CYLINDERS) DAYS. FINAL CYLINDER SHALL BE HELD IN RESERVE.

F. THERE SHALL BE NO HORIZONTAL JOINTS IN ANY CONCRETE WORK UNLESS SHOWN ON THE STRUCTURAL DRAWINGS. THE ENGINEER OF RECORD SHALL APPROVE ALL DEVIATIONS OR ADDITIONAL JOINTS IN WRITING.

STRUCTURAL STEEL

A. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO AISC SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS-ALLOWABLE STRESS DESIGN, 13TH ED.

B. MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED:

STRUCTURAL STEEL PLATES	ASTM 572, GRADE 50
THREADED ROD ANCHOR BOLTS	ASTM F1554 GRADE 55 (S1)
TUBE SECTIONS	ASTM A500, GRADE B
STRUCTURAL ANGLE & CHANNELS	ASTM A36

C. WELDING, UNLESS OTHERWISE NOTED:

1. ALL SHOP AND FIELD WELDING SHALL CONFORM TO AWS STRUCTURAL WELDING CODE, ANSI/AWS D1.1
2. ALL WELDING IN THE SHOP AND IN THE FIELD SHALL BE PERFORMED BY CERTIFIED WELDERS ONLY.
3. WELDING ELECTRODES SHALL BE E70XX LOW HYDROGEN.

D. CONNECTIONS, UNLESS OTHER WISE NOTED (U.O.N.):

1. TIGHTEN BOLTS BY THE "SNUG-TIGHT" METHOD.
2. FIELD CONNECTIONS SHALL BE MADE WITH 3/4" DIA. MIN. HIGH-STRENGTH BEARING TYPE BOLTS (A325N) WITH THREADS ASSUMED TO BE INCLUDED IN SHEAR PLANES.

E. SHOP DRAWINGS (SEE SUBMITTALS) SHALL INCLUDE COMPLETE DETAILING OF STRUCTURAL STEEL MEMBERS AND CONNECTIONS AS REQUIRED TO FABRICATE AND ERECT ALL STRUCTURAL STEEL FRAMING.

F. GROUT UNDER BEARING PLATES SHALL BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 6000 PSI WHEN BEARING ON 3000 PSI CONCRETE, AND 8000 PSI WHEN BEARING ON 4000 PSI CONCRETE.

G. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED OR OTHERWISE PROTECTED.

H. ALL STEEL EXPOSED TO SOIL SHALL BE ENCASED IN CONCRETE OR OTHERWISE PROTECTED.

STRUCTURAL INSPECTION

A. THE CONTRACTOR OR OWNER SHALL EMPLOY A QUALIFIED INDEPENDENT INSPECTION AGENCY. THIS AGENCY (REFERRED TO AS INSPECTOR HEREIN) SHALL BE RESPONSIBLE FOR VERIFYING THAT PROJECT STRUCTURAL WORK IS ACCOMPLISHED IN CONFORMANCE WITH THE CONTACT DOCUMENTS.

B. DURATION AND FREQUENCY OF JOB VISITS SHALL BE SUFFICIENT FOR THE INSPECTOR TO STATE AT THE COMPLETION OF THE PROJECT THAT THE STRUCTURAL WORK IS ACCOMPLISHED, AND ITS RELATED ELEMENTS HAVE BEEN ERECTED IN ACCORDANCE WITH THE CONTACT DOCUMENTS. SPECIFIC SYSTEMS TO BE INSPECTED INCLUDE:

FOUNDATION REINFORCING & CONCRETE
STRUCTURAL STEEL
PRE-ENGINEERED METAL PANEL DECKING
PRE-ENGINEERED LAM. PANEL DECKING

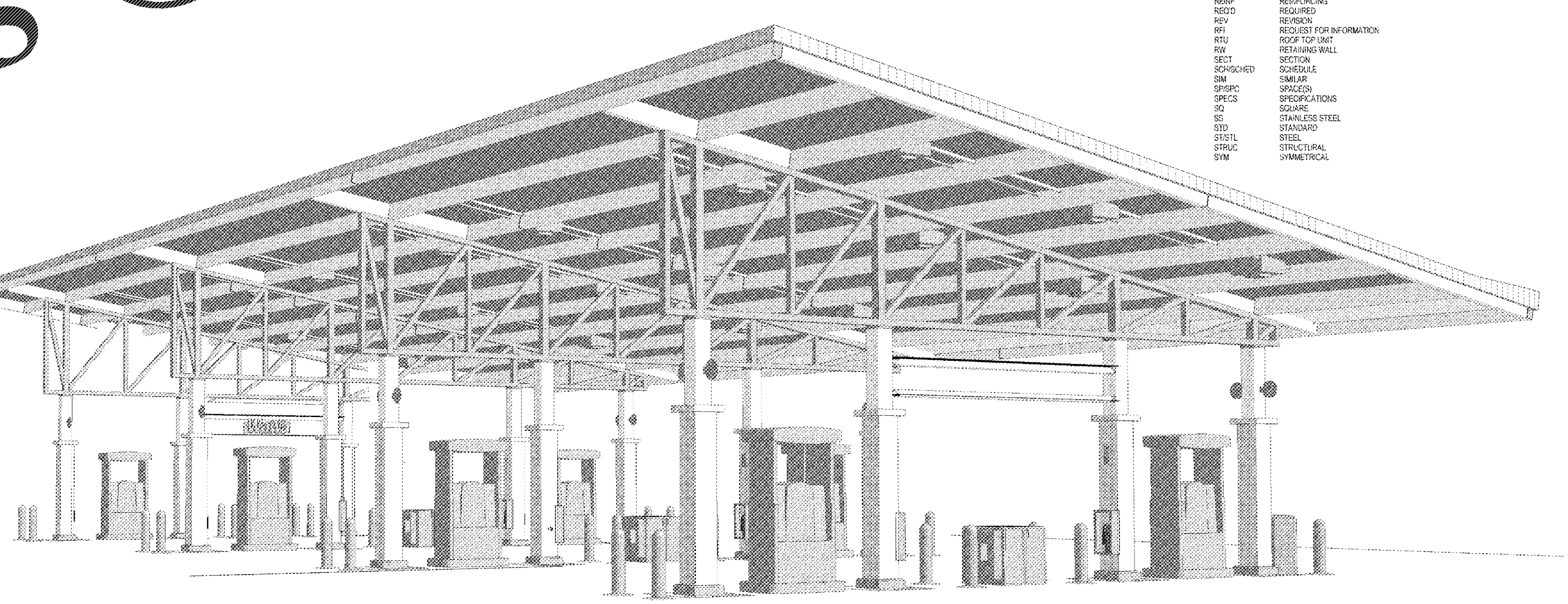
C. THE FOREGOING LIST IS NOT INTENDED TO BE ALL INCLUSIVE. THE INSPECTOR SHALL USE HIS PROFESSIONAL JUDGEMENT AND HIS KNOWLEDGE OF THE JOB SITE CONDITIONS AND THE OFFICIAL CONTRACT DOCUMENTS. THE INSPECTOR WILL NOT REPLACE THE QUALITY CONTROL PERSONNEL OF THE CONTRACTOR.

THE INSPECTOR DOES NOT RELIEVE THE CONTRACTOR OF CONTRACTUAL OR STATUTORY OBLIGATIONS. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE OFFICAL CONTRACT DOCUMENTS. THE INSPECTOR WILL NOT REPLACE THE QUALITY CONTROL PERSONNEL OF THE CONTRACTOR.

E. ALL INSPECTION REPORTS SHALL BE FORWARD BY THE INSPECTOR TO THE ENGINEER AND ARCHITECT OF RECORD.

LIST OF STRUCTURAL ABBREVIATIONS

AB	ANCHOR BOLTS
ADDL	ADDITIONAL
ALT	ALTERNATE
ANCH	ANCHOR
APPROX	APPROXIMATE
ARCH	ARCHITECT/ARCHITECTURAL
BB	BOND BEAM
BLDG	BUILDING
BM	BEAM
BO	BOTTOM OF
BOT/BTM	BOTTOM
BP	BASE PLATE/BEARING PLATE
BRG	BEARING
BTW	BETWEEN
CANT	CANTILEVER
CB	CONCRETE BEAM
CC	CONCRETE COLUMN
CIP	CAST IN PLACE
CJ	CONSTRUCTION JOINT
CL	CENTERLINE
CLR	CLEAR/CLEARANCE
COL	COLUMN
CONC	CONCRETE
CONNX	CONNECTION
CONSTR	CONSTRUCTION
CONT	CONTINUOUS
CORR	CORRUGATED
CMU	CONCRETE MASONRY UNIT
DET	DETAIL
DIA/DIAM	DIAMETER
DIM	DIMENSION
DIST	DISTANCE
DN	DOWN
DR	DRAIN
DWG	DRAWING
DWL	DOWEL
EA	EACH
EE	EACH END
EF	EACH FACE
EJ	EXPANSION JOINT
EL/ELEV	ELEVATION
ELECT	ELECTRICAL
EMB	EMBEDMENT
ENGR	ENGINEER
EOR	ENGINEER OF RECORD
EQ	EQUAL
EQUIP	EQUIPMENT
EQUIV	EQUIVALENT
ES	EACH SIDE
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
EW	EACH WAY
FAB	FABRICATE
FBC	FLORIDA BUILDING CODE
FD	FLOOR DRAIN
FDN	FOUNDATION
FF	FINISHED FLOOR
FIN	FINISH(ED)
FL/FLR	FLOOR
FTG	FOOTING
GA	GAGE/GAUGE
GALV/GV	GALVANIZED
GC	GENERAL CONTRACTOR
HAS	HEADED ANCHOR STUD
HB	HIGH BEAM (MASONRY)
HORIZ	HORIZONTAL
HSB	HIGH STRENGTH BOLTS
HSS	HOLLOW STEEL SECTION
HT	HEIGHT
INT	INTERIOR
JST	JOIST
JT	JOINT
K	KIPS
KO	KNOCK OUT
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
L	ANGLE
LB	LOW BEAM (MASONRY)
LBS	POUNDS
LD	DEVELOPMENT LENGTH
LDH	LONG DIMENSION HORIZONTAL
LDV	LONG DIMENSION VERTICAL
LGTH	LENGTH
LLBB	LONG LEG BACK TO BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
MATL	MATERIAL
MAX	MAXIMUM
MB	MASONRY BEAM
MC	MOMENT CONNECTION
MC	MASONRY COLUMN
MCJ	MASONRY CONTROL JOINT
MECH	MECHANICAL
MEZZ	MEZZANINE
MFR	MANUFACTURE/MANUFACTURERS
MIN	MINIMUM
MISC	MISCELLANEOUS
MTL	METAL
NIC	NOT IN CONTRACT
NOM	NOMINAL
NTS	NOT TO SCALE
OC	ON CENTER
OPNG	OPENING
OPP	OPPOSITE
PB	PARAPET BEAM (MASONRY)
PCC	PRECAST CONCRETE
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PLY	PLYWOOD
PRE-ENG	PRE-ENGINEERED
PEMB	PRE-ENGINEERED METAL BUILDING
PRE-FAB	PRE-FABRICATED
PRELIM	PRELIMINARY
PROJ	PROJECTION
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	PRESSURE TREATED
PT	POST TENSION(ING) (ED)
PW	PANEL WIDTH
RAD	RADIUS
RB	ROOF BEAM (MASONRY)
RCP	REINFORCED CONCRETE PIPE
RD	ROOF DRAIN
REF	REFERENCE
REINF	REINFORCING
REQ'D	REQUIRED
REV	REVISION
RFI	REQUEST FOR INFORMATION
RTU	ROOF TOP UNIT
RW	RETAINING WALL
SECT	SECTION
SCH/SCHED	SCHEDULE
SIM	SIMILAR
SP/SPC	SPACE(S)
SPECS	SPECIFICATIONS
SQ	SQUARE
SS	STAINLESS STEEL
STD	STANDARD
ST/STL	STEEL
STRUC	STRUCTURAL
SYM	SYMMETRICAL
T & B	TOP AND BOTTOM
T & G	TONGUE AND GROOVE
TE	THICKENED EDGE
THK	THICK
THRD	THREAD(ED)
TB	TIE BEAM
TOC	TOP OF CONCRETE
TOM	TOP OF MASONRY
TOS	TOP OF STEEL
TRIB	TRIBUTARY
TYP	TYPICAL
TS	TUBE STEEL
UNO	UNLESS NOTED OTHERWISE
UON	UNLESS OTHERWISE NOTED
VERT	VERTICAL
VIF	VERIFY IN FIELD
W	WIDE FLANGED
WB	WALL BEAM (TILT)
WC	WALL COLUMN (TILT)
W/	WITH
W/O	WITHOUT
WD	WOOD
WGT	WEIGHT
WH	WEEPHOLE
WP	WORKING POINT
WT	STEEL TEE SECTION
WWF	WELDED WIRE FABRIC
@	AT
&	AND



1 CS1 3D View - Perspective

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PROJECT NAME
WAWA STACKED 8 GAS CANOPY 2017.01 STORE #5254-OKE
OKEECHOBEE RD. & JENKINS RD.
FT. PIERCE, FLORIDA

SHEET TITLE
DESIGN CRITERIA AND NOTES

Revision Schedule

No.	Description	Date
	PERMIT SET	06/24/2016
A	REV 2014.01	APR14
	PRE-BID REVIEW SET	07/26/2017
	BID SET	08/30/2017

PROJECT NO. 2160209
DATE 08-30-2017
DRAWN RC
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CS1

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