GENERAL

A. ALL WORK SHALL BE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2014, AS ADOPTED AND SUPPLEMENTED BY LOCAL REGULATION.

B. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ARCHITECT/ENGINEER OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH WORK.

C. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETE DESIGN OF THE STRUCTURE. THEY DO NOT INDICATE THE MEANS AND METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKERS, OR THER PERSONS DURING CONSTRUCTION.

D. OBSERVATION VISITS TO THE SITE BY EOR OR REPRESENTATIVES OF THE EOR MAY BE MADE DURING CONSTRUCTION. ANY SUPPORT SERVICES PERFORMED HEREIN SHALL BE DISTINGUSHED FROM INSPECTION AND,OR TESTING SERVICES PERFORMED BY OTHERS, AND ARE NOT TO BE CONSTRUED AS SUPERVISION AND/OR MANAGEMENT OF CONSTRUCTION.

E. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTRUAL MEMBERS AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT/ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STAILITY OF THE STRUCTURE OR CAUSE DISTRESS WITHIN THE STRUCTURE.

F. CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED IN THE GENERAL NOTES. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS

G. DRAWINGS ARE NOT TO BE SCALED.

H. FOR ACTUAL ELEVATION ON FIRST FLOOR (REF. ELEV. 0'-0" HEREIN). SEE SITPLAN.

J. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO THE DETAILS PRESENTED, SIMILAR DETAILS SHALL BE USED SUBJECT TO THE REVIEW OF ENGINEER OF RECORD.

K. SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS. AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR. SUBSEQUENT DOCUMENTATION/REQUESTS TO BUILDING ENGINEER OF RECORD FROM GENERAL CONTRACTOR SHALL INCLUDE EVALUATION OF DEVIATIONS BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.

L. THE MOST STRINGENT REQUIREMENTS APPLY IN CASE OF CONFLICT BETWEEN SPECIFICATIONS, STANDARDS, CODES AND DRAWINGS.

M. DESIGN DATA

ROOF LIVE LOADS:	20 PSF
ROOF DEAD LOADS:	20 PSF

WIND LOADING

1.	ULTIMATE DESIGN WIND SPEED, V_ULT:	158 MPH
2.	NOMINAL WIND SPEED, V_ASD:	123 MPH
3.	RISK CATEGORY (TABLE 1.5-1, ASCE7):	II
4.	WIND EXPOSURE:	C
5.	ENCLOSURE CLASSIFICATION:	ENCLOSED (+/- 0.18)
6.	COMPONENT AND CLADDING:	SEE CHARTS THIS SHEET

SUBMITTALS

A. SHOP DRAWING REVIEW IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT. CORRECTIONS OR COMMENTS MADE ON THIS REVIEW DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR ERRORS AND OMMISIONS, AND FROM COMPLIANCE WITH THE PLANS AND SPECIFICATIONS. CORRECTIONS OR COMMENTS DO NOT AUTHORIZE AN INCREASE IN THE CONSTRUCTION BUDGET.

B. APPROVAL OF SHOP DRAWINGS DOES NOT INDICATE ACCEPTANCE OF DEVIATIONS FROM CONTRACT DOCUMENTS OR PREVIOUS SHOP DRAWING REVIEW. UNLESS SPECIFICALLY NOTED THEREIN BY ENGINEER OF RECORD.

C. ANY CHANGES TO THE DESIGN CONCEPT SHOWN IN CONTRACT DOCUMENTS SHALL BE SUBMITTED IN WRITING AND APPROVED BY THE ARCHITECT AND ENGINEER PRIOR TO SUBMITTING SHOP DRAWINGS. ALL SUCH CHANGES SHALL BE "BUBBLED" ON THE SHOP DRAWINGS AND REFERENCED TO THE PROPER R.F.I.

D. SUBMITTALS SHALL CONFORM TO THE REQUIREMENTS OF THE CONTRACT DRAWINGS (REFERENCE ITEM C ABOVE FOR EXCEPTION). NON-CONFORMING OR NON-REVIEWED SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.

1. SHOP DRAWINGS SHALL BE "APPROVED", SIGNED AND DATED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO ENGINEER AND ARCHITECT OF RECORD.

2. SHOP DRAWINGS SHALL NOT CONTAIN REPRODUCTIONS OF THE CONTRACT DRAWINGS.

3. SUBMITTAL REQUIREMENTS:

SHOP DRAWINGS/DATA TO BE SUBMITTED FOR APPROVAL

CONCRETE MIX DESIGNS (INDICATE INTENDED USE OF EACH)
REINFORCING STEEL
STRUCTURAL STEEL (INCLUDING JOISTS & JOIST GIRDERS)
METAL DECK

SHOP DRAWINGS/DATA TO BE SUBMITTED FOR APPROVAL SUBMITTALS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LISCENCED IN THE STATE OF THE PROJECT

LIGHT GAUGE TRUSS SYSTEMS
RTU TIE DOWNS

E. PROVIDE COPIES OF MANUFACTURERS LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.

FOUNDATIONS/SITE WORK

A. FOUNDATION DESIGN IS BASED UPON THE FOLLOWING SOILS REPORT:

NO REPORT PROVIDED:
PRIOR TO CONSTRUCTION, CONTRACTOR SHALL RETAIN THE SERVICES OF A QUALIFIED GEOTECHNICAL ENGINEER. THE GENERAL CONTRACTOR HEREIN IS RESPONSIBLE FOR PERFORMING ALL EARTHWORK OPERATIONS IN STRICT ACCORDANCE WITH GEOTECHNICAL ENGINEERING REQUIREMENTS.

IF THE FOUNDATION RECOMMENDATIONS AN/OR DESIGN VALUES DIFFER FROM THAT ASSUMED, MODIFICATIONS TO THE DRAWINGS MAY BE REQUIRED.

DO NOT PLACE CONCRETE PRIOR TO GEOTECHNICAL APPROVAL.

B. ALLOWABLE SOIL PRESSURE ASSUMED TO BE 2,500 PSF (REF A. ABOVE). SHALLOW FOUNDATION SYSTEM IS ASSUMED TO BE ADEQUATE. ESTIMATED SETTLEMENT ASSUMED TO BE NO GREATER THAN 1" OVERALL AND 1/2" DIFFERENTIAL SETTLEMENT. IT IS ASSUMED THAT ALL PROBLEMATIC SOILS (ORGANIC, EXPANSIVE, ETC.) WILL BE REMOVED AND REPLACED WITH SUITABLE STRUCTURAL FILL.

C. ANY FILL REQUIRED TO BACKFILL EXCAVATED AREA OR ACHIEVE FINISHED GRADE IN STRUCTURAL AREAS SHALL BE AS INDICATED BY GEOTECHNICAL ENGINEER. THE FILL SHALL BE PLACED IN LEVEL LIFTS NOT EXCEED 12 INCHES LOOSE THICKNESS AND COMPACTED TO A MINIMUM OF 95% OF THE SOIL'S MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM SPECIFICATION D-1557.

D. IN-PLACE DENSITY TESTS SHALL BE PERFORMED BY AN EXPERIENCED ENGINEERING TECHNICIAN. TESTS SHALL BE PERFORMED FOR EACH 2,000 SQUARE FEET, IN EVERY COLUMN FOOTING LOCATION AND EACH 50 FEET ALONG WALL FOOTINGS. COPIES OF THE TEST REPORTS SHALL BE FURNISHED TO THE STRUCTURAL ENGINEER.

E. REMOVE FREE WATER FROM EXCAVATIONS BEFORE PLACING CONCRETE.

F. CAUTION SHOULD BE USED WHEN OPERATING VIBRATORY COMPACTING EQUIPMENT NEAR THE EXISTING STRUCTURE TO AVOID THE RISK OF DAMAGE TO THE STRUCTURE.

G. REFER TO ARCHITECTURE DRAWINGS FOR ANY NECESSARY WATERPROOFING REQUIREMENTS.

REINFORCED CONCRETE

A. CONCRETE WORK SHALL CONFORM TO ACI SPECIFICATIONS. CODES AND STANDARDS.

B. CAST-IN-PLACE CONCRETE, UNLESS OTHERWISE NOTED:

PORTLAND CEMENT PER ASTM C150
PROPORTIONED FOR MAX. ALLOWABLE UNIT SHRINKAGE OF 0.03%
25% MAXIMUM FLYASH BY WEIGHT

SLABS
4000 PSI 28-DAY COMPRESSIVE STRENGTH, NORMAL WEIGHT.
MAXIMUM AGGREGATE SIZE OF 1", 0.55 MAX. W/C, 4"-5" SLUMP

FOOTINGS
4000 PSI 28-DAY COMPRESSIVE STRENGTH, NORMAL WEIGHT.
MAXIMUM AGGRETGATE SIZE OF 1", 0.55 MAX. W.C. 3"-4" SLUMP

C. CONCRETE COVER OVER REINFORCEMENT:

FOOTINGS
3"
SLABS, COLUMNS AND BEAMS EXPOSED TO EARTH OR WEATHER
2" (#6 TO #18 BARS), 1 1/2" (3# TO #5 BARS)
SLABS NOT EXPOSED TO EARTH OR WEATHER
3/4" (#3 TO #11 BARS)
BEAMS AND COLUMNS NOT EXPOSED TO EARTH OR WEATHER
1 1/2" (#3 TO #18 BARS)

D. REINFORCING, UNLESS OTHERWISE NOTED:

1. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.

2. WELDED WIRE FABRIC SHALL CONFORM TO ASTMA185. WIRE FABRIC SHALL BE CONTINUOUS, LAPPED ONE CROSS WIRE SPACING PLUS 2" MIN.

3. WHERE CONTINUOUS REINFORCING IS SPECIFIED, SUCH REINFORCING MAY BE SPLICED WHERE APPROVED IN WRITTING BY THE ENGINEER OF RECORD.

4. AT CHANGES IN DIRECTION OF CONCRETE WALLS, BEAMS, AND FOOTINGS, PROVIDE CORNER BARS OF SAME SIZE, QUANTITY AND SPACING AS HORIZONTAL STEEL.

E. CONTRACTOR SHALL EMPLOY A 3rd PARTY TESTING & INSPECTION AGENCY RETAINED BY WAWA. GENERAL CONTRACTOR TO COORDINATE ALL TESTING & INSPECTIONS. AGENCY SHALL PREPARE ONE SET OF FOUR TEST CYLINDERS PER DAY OR ONE SET FOR EACH 50 CUBIC YARDS POURED, MAXIMUM. LABORATORY REPORT SHALL BE FURNISHED TO THE STRUCTURAL ENGINEER SHOWING STRENGTH OF CONCRETE AT 7 (1 CYLINDER) AND 28 (2 CYLINDERS) DAYS. FINAL CYLINDER SHALL BE HELD IN RESERVE.

F. THERE SHALL BE NO HORIZONTAL JOINTS IN ANY CONCRETE POURS UNLESS SHOWN ON THE STRUCTURAL DRAWINGS. THE ENGINEER OF RECORD SHALL APPROVE ALL DEVIATIONS OR ADDITIONAL JOINTS IN WRITING.

G. SAW CUT CONTROL JOINTS SHALL BE LOCATED @ 12' O.C. MAXIMUM, UNLESS OTHERWISE NOTED WITH LENGTH TO WIDTH RATIO NOT TO EXCEED 1.5:1 (INDICATED ON PLAN) JOINTS IN TERRAZO FINISH FLOOR MUST ALIGN WITH CONCRETE CONTROL JOINTS.

H. FINISH CONCRETE SLAB ON GRADE SURFACE TO THE FOLLOWING TOLERANCES. ACCORDING TO ASTM E 1155: SPECIFIED OVERALL VALUES OF FLOOR FLATNESS, F(F) 20, MINIMUM LOCAL VALUES OF FLOOR FLATNESS, F(F) 15, SPECIFIED OVERALL VALUES OF FLOOR LEVELNESS, F(L) 15. MINIMUM LOCAL VALUES OF FLOOR LEVELNESS, F(L) 10.

I. LIQUID MEMBRANE FORMING CURING COMPOUNDS ARE PROHIBITED FROM USE ON THE BUILDING SLAB ON GRADE AS THEY MAY ADVERSELY AFFECT THE PERFORMANCE OF THE CONCRETE TERRAZO TOPPING. PROVIDE FOR CURING OF THE SLAB VIA ALTERNATE OR TRADITIONAL METHODS. REFER TO ACI 308.1 FOR ADDITIONAL INFORMATION.

J. ALL EPOXY ANCHORS USED AT REINFORCED CONCRETE SHALL BE HILTI HIT HY 150 OR HIT HY 200, UNLESS OTHERWISE NOTED.

K. POWDER ACTUATED FASTNERS (PAFS) NOT PERMITTED AT CONCRETE.

CONCRETE MASONRY UNITS (CMU)

A. ALL MASONRY WORK SHALL CONFORM TO ACI SPECIFICATIONS FORMASONRY STRUCTURES (ACI 530).

B. CONCRETE BLOCK SHALL CONFORM TO ASTM C90. TYPE II, WITH A MIN. NET COMPRESSIVE STRENGTH AT 28-DAYS OF 1900 PSI (F'M = 1500 PSI). NORMAL WEIGHT UNITS UNLESS OTHERWISE NOTED.

C. BELOW GRADE: MORTAR SHALL BE TYPE M. ABOVE GRADE, M OR S. GROUT TO CONFORM TO ASTM C270.

D. ALL MASONRY LOCATED BELOW GRADE SHALL BE GROUTED SOLID.

E. GROUT FOR FILLED CELLS SHALL CONFORM TO ASTM C476, AND SHALL HAVE A SLUMP OF BETWEEN 8" AND 10". GROUT COMPRESSIVE STRENGTH SHALL BE 3000 PSI AT 28 DAYS. ALL GROUTING SHALL BE LOW LIFT. MAXIMUN GROUT POUR HEIGHT = 5'-0".

F. PLACE ALL MASONRY IN RUNNING BOND WITH 3/8" MORTAR JOINTS. PROVIDE COMPLETE COVERAGE FACE SHELL MORTAR BEDDING, HORIZONTAL AND VERTICAL. PROVIDE FULL MORTAR WEBS IN ALL COURSE OF GROUTED CELLS AND CELLS ADJACENT TO GROUTED CELLS.

G. PROVIDE 8" DEEP PRECAST REINFORCED CONCRETE LINTELS OVER ALL MASONRY OPENINGS NOT SHOWN TO HAVE A STRUCTURAL BEAM. MINIMUM END BEARING SHALL BE 8". LINTEL WIDTH TO MATCH MASONRY WIDTH.

H. ALL EPOXY ANCHORS USED IN MASONRY SHALL OCCUR AT GROUTED. REINFORCED CELLS.

J. REINFORCING. UNLESS OTHERWISE NOTED IN PLANS:

1. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.

2. ALL OPENINGS IN CMU SHALL BE REINFORCED WITH AT LEAST (1) #5 BAR AT THE TOP, BOTTOM AND BOTH SIDES OF THE OPENING

3. BOND BEAM AT ROOF LEVEL SHALL BE DOUBLE COURSE K.O. / LINTEL BLOCK WITH (2) #5 CONT. BARS EACH COURSE, AND BOND BEAM AT PARAPET LEVEL SHALL BE SINGLE COURSE K.O. / LINTEL BLOCK WITH (2) #5 CONT. BARS.

4. LAP SPLICES SHALL BE AS FOLLOWS:
48 X BAR DIAMETERS. UNLESS OTHERWISE NOTED.

5. HORIZONTAL WALL REINFORCING SHALL BE 9 GA. GALVANIZED LADDER TYPE @ 16" O.C. VERT.

6. ALL REINFORCING HOOKS AND BENDS SHALL BE STANDARD ACI TYPE.

7. ALL WALL DOWELS SHALL MATCH REINFORCING SIZE AND QTY.

L. POWER ACTUATED FASTENERS (PAFS) NOT PERMITTED AT MASONRY.

LAP SPLICES (INCHES)

REINF. SIZE	CONCRETE STRENGTH (PSI) 3000	3500	4000	4500	5000
#3	21	20	18	17	16
#4	30	27	24	23	22
#5	36	33	30	29	27
#6	46	43	40	38	36
#7	63	59	54	51	49
#8	82	77	71	68	64
#9	104	97	90	86	81
#10	132	123	114	108	102
#11	162	151	140	133	126

REF. CRSI, CLASS B, CAT. 3, FY=60 KSI, UNCOATED BARS, NORMAL WEIGHT CONC.

STRUCTURAL STEEL

A. ALL WORK SHALL CONFORM TO AISC SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS (AISC 360).

B. MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED:

W SECTIONS	ASTM A992
STRUCTURAL STEEL PLATES, ANGLES, CHANNELS	ASTM A36
THREADED ROD ANCHOR BOLTS	ASTM F1554 GRADE 55-S1
HEADED ANCHOR BOLTS	ASTM F1554 GRADE 55-S1
PIPE SECTIONS	ASTM A53, GRADE B.
TUBE SECTIONS	ASTM A500, GRADE B.

C. WELDING, UNLESS OTHERWISE NOTED:

1. ALL SHOP AND FIELD WELDING SHALL CONFORM TO AWS STRUCTURAL WELDING CODE, ANSI/AWS D1.1

2. ALL WELDING IN THE SHOP AND IN THE FIELD SHALL BE PERFORMED BY CERTIFIED WELDERS ONLY.

3. WELDING ELECTRODES SHALL BE E70XX LOW HYDROGEN.

D. CONNECTIONS, UNLESS OTHERWISE NOTED:

1. TIGHTEN BOLTS BY THE "SNUG-TIGHT" METHOD.

2. FIELD CONNECTIONS SHALL BE MADE WITH [illegible] MIN. HIGH-STRENGTH [illegible] TYPE BOLTS (A325N) WITH THREADS ASSUMED TO BE [illegible] IN SHEAR PLANES.

3. ALL BEAM TO BEAM AND BEAM TO COLUMN CONNECTIONS [illegible] INCORPORATE THE MAXIMUM PERMISSIBLE 3/4" A325N BOLTS IN A SINGLE ROW [illegible] ON AVAILABLE WEB DEPTH.

4. CONNECTIONS SHALL BE SINGLE PLATE TYPE. UNLESS OTHERWISE [illegible] ANGLE AND EXTENDED PLATE CONNECTIONS SHALL NOT BE PERMITTED UNLESS SPECIFICALLY DETAILED AS SUCH HEREIN.

E. SHOP DRAWINGS (SEE SUBMITTALS) SHALL INCLUDE COMPLETE DETAILING OF STRUCTURAL STEEL MEMBERS AND CONNECTIONS AS REQUIRED TO FABRICATE AND ERECT ALL STRUCTURAL STEEL FRAMING.

F. GROUT UNDER BEARING PLATES SHALL BE NON-METALLIC. NON-SHRINK TYPWITH A COMPRESSIVE STRENGTH OF AT LEAST 6000 PSI WHEN BEARING ON 3000 PSI CONCRETE AND 8000 PSI WHEN BEARING ON 4000 PSI CONCRETE.

G. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED OR OTHERWISE PROTECTED.

H. ALL STEEL EXPOSED TO SOIL SHALL BE ENCASED IN CONCRETE OR COATED PER ASPHALIC BASED CORROSION NOTE.

STEEL JOISTS AND STEEL GIRDERS

A. ALL STEEL JOISTS, JOIST GIRDERS AND ASSOCIATED PARTS SHALL CONFORM TO THE STANDARDS OF THE STEEL JOIST INSTITUTE.

B. ALL JOISTS AND GIRDERS SHOWN IN THE PLANS ARE MINIMUM SIZES. DEPTH CANNOT BE DECREASED OR INCREASED WITHOUT WRITTEN APPROVAL BY ARCHITECT/ENGINEER.

C. CONTRACTOR TO FOLLOW SJI REQUIREMENTS AS A MINIMUM.

D. PROVIDE BRIDGING AND X-BRACING AS PER THE REQUIREMENTS OF SJI. SHOULD PLANS INDICATE BRIDGING AND X-BRACING IN EXCESS OF THAT REQUIRED BY SJI, PLANS WILL GOVERN.

E. PROVIDE SEATS, HEADER CHORD EXTENTIONS, OR OTHER ELEMENTS AND FEATURES SHOWN IN THE CONTRACT DOCUMENTS AND THOSE WHICH ARE REQUIRED FOR PROPER ERECTION AND PERFORMANCE OF STEEL JOISTS

F. STEEL JOISTS, JOIST GIRDERS, BRIDGING AND X-BRACING SHALL BE DESIGNED FOR NET UPLIFT. NET UPLIFT, NOT PRESENTED IN PLANS SHALL BE BASED ON THE CLADDING AND COMPONENTS WIND LOADS INDICATED IN THE PLANS PER APPLICABLE ZONE AND TRIB. AREA. LOAD COMBINATIONS SHALL CONFORM TO THE CODES SPECIFIED IN THE PROJECT DOCUMENTS.

G. REF. PLANS FOR CONCENTRATED LOADS AND/OR UNUSUAL LOAD PATTERNS.

H. AT A MINIMUM, PROVIDE BOTTOM CHORD BRACING EVERY OTHER JOIST AT JOIST-GIRDERS AND SUPPORTING W BEAMS. PROVIDE ADDITIONAL BRACING AS SPECIFIED HEREIN AS TYPICAL AND AS INDICATED IN PLANS.

I. ***MINIMUM WELDS, UNLESS OTHERWISE NOTED:***

1. BAR JOIST ENDS	***3/16" FILLET, 2" LONG, EACH SIDE***
2. JOIST GIRDER ENDS	***1/4" FILLET, 3" LONG, EACH SIDE***

J. DIAGONAL BRIDGING / BRACING SHALL BE BETWEEN ADJACENT JOISTS WHENEVER BOTTOM CHORD HORIZONTAL BRIDGING IS DISCONTINUOUS.

STEEL DECK NOTES

A. DECK SHALL CONFORM TO SDI SPECIFICATIONS.

B. SPECIFICATION AND FASTENING REQUIREMENTS, REF. PLANS

C. WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL. END LAPS AND SIDE JOINTS SHALL BE COMPLETELY FUSED TO THE SUPPORTING MEMBERS. USE PROPER SIZE WELDING ELECTRODE AND WELDING MACHINE CURRENT, SO THAT WELDS ARE ADEQUATE PENETRATION AND FREE FROM "BURN-THROUGH".

D. METAL DECK SHALL BE ERECTED CONTINUOUS FOR A MINIMUM OF THREE (3) SPANS AND SHALL LAP AT THE CENTERLINE OF SUPPORTS A MINIMUM OF TWO (2") INCHES. PROVIDE A MINIMUM END BEARING OF 2" OVER SUPPORTS.

E. PROVIDE SUPPORTS FOR METAL DECK AS REQUIRED WHERE METAL DECK IS CUT OUT. WELD DECK TO SUPPORTS SAME AS EDGE CONDITION.

F. DO NOT HANG OR ATTACH DUCTWORK, CONDUIT, PIPING, EQUIPMENT, CEILINGS, ETC. FROM METAL DECKING.

G. PROVIDE STL. SHIMS AND EMBEDS AS REQUIRED TO SUPPORT DECK ON "TYPICAL" BAY SPACING.

ASPHALTIC BASED CORROSION RESISTANCE COATING

A. REFER TO SPECIFICATIONS FOR GENERAL REQUIREMENTS, PRODUCTS, AND EXECUTION OF WORK.

B. ASPHALTIC BASED CORROSION RESISTANCE COATING SHALL BE MADE WITH AN ASPHALT-BASE EMULSION COATING SYSTEM PER ASTM D 1187.

C. APPLY A MINIMUM 1/16-INCH THICK COATING IN TWO APPLICATIONS.

D. COATING TO BE APPLIED ON ALL SIDES OF MEMBERS – TO INCLUDE STEEL TO BE ADJACENT TO CAST-IN-PLACE CONCRETE.

E. COAT STRUCTURAL, ANCHOR BOLTS, WELDS, AND ALL COMPONENTS IN THE AFFECTED AND DEFINED AREA.

F. ALL STEEL AND STEEL COMPONENTS (I.E., BASE PLATES AND ANCHOR BOLTS) EXPOSED TO SOIL SHALL BE COATED FROM THE FOUNDATION (LOWEST LEVEL) UP TO 6-INCHES ABOVE THE SLAB OR FINAL GRADE – WHICHEVER IS GREATER.

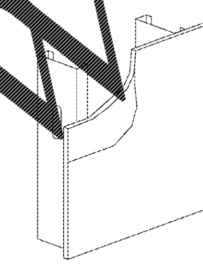
STRUCTURAL PLYWOOD / CDX, 5/6" MIN. THICKNESS & DENSGLASS [illegible]

A. ZONES 1, 2, AND 4 (REF. S1) #10 BUGLEHEAD SCREWS @ 4" O.C. [illegible] AND 6" FIELD

B. ZONES 3, AND 5 (REF. S1) #10 BUGLEHEAD [illegible] @ 4" O.C. EDGES AND FIELD

C. ALL PANEL EDGES SHOULD BE SUPPORTED BY [illegible] FLAT STRAP BRACING [illegible] SOLID BLOCKING. REF. SHEET S7 & DETAIL 10/S10 FOR MORE INFO.

D. PLYWOOD TO BE PLACED PERPENDICULAR TO THE FRAMING.



Sheathing Fastener Requirements

LIGHT GAUGE FRAMING, TRUSSES & SYSTEMS

A. ALL LIGHT GAUGE STEEL FRAMING SHALL CONFORM TO THE "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STRUCTURAL STEEL MEMBERS," AISI."

B. ALL STUDS SHALL BE EQUAL TO A MINIMUM OF 6-INCH 54 MIL THICK, UNLESS OTHERWISE NOTED.

C. ALL STUDS SHALL BE "C" SHAPE, UNLESS OTHERWISE NOTED.

D. MATERIALS, UNLESS OTHERWISE NOTED:

54, 68 AND 97 MIL STUDS	50 KSI
27, 33, AND 43 MIL STUDS	33 KSI
RUNNERS AND ACCESSORIES	33 KSI

E. RIGID CLIP ANGLES SHALL BE COLD FORMED TO SHAPE FROM SHEET STEEL IN COMPLIANCE WITH THE REQUIREMENTS OF ASTM A653/ A653M WITHSTRUCTURAL STEEL (SS) GRADE 50 CLASS 1 FOR MEMBERS WITH A DESIGN THICKNESS OF 0.0566 (16G) OR GREATER.

F. ALL PARTS AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING MEETING THE REQUIREMENTS OF ASTM A525 / ASTM A653.

G. ALL FRAMING SCREWS SHALL BE A MINIMUM OF #10-16 DIAMETER EXCEPT WHERE LARGER SCREWS ARE NOTED AND SHALL BE SELF-DRILLING TAPPING SCREWS CONFORMING TO SAE J78. A MINIMUM OF THREE EXPOSED THREADS SHALL EXTEND THROUGH THE STEEL BEYOND ALL FRAMING COMPONENTS CONNECTED. SCREWS SHALL PENETRATE THE INDIVIDUAL FRAMING COMPONENTS OR CONNECTION MATERIAL WITHOUT CAUSING PERMANENT SEPARATION BETWEEN COMPONENTS. SCREWS SHALL BE INSTALLED SUCH THAT NEITHER THREADS NOR HOLES ARE STRIPPED. ALL FRAMING SCREWS SHALL BE GRABBER HEX HEAD SELF DRILLING TAPING SCREWS OR ITW BUILDEX TEKS SELF DRILLING TAPING SCREWS. HILTI KWIK-PRO SELF DRILLING SCREWS ARE ALSO ACCEPTABLE. SCREW DRILLING POINT ACCORDING TO MANUFACTURER SPECIFICATIONS. UNLESS OTHERWISE NOTED ON DRAWINGS. ALL SCREWS SHALL HAVE A CORROSION RESISTANT COATING ACCORDING TO THE PROJECT SPECIFICATION.

H. WELDING OF LIGHT-GAUGE STEEL MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.3: STRUCTURAL WELDING CODE - SHEET STEEL. WELDERS AND WELDING PROCEDURES SHALL BE QUALIFIED AS SPECIFIED IN AWS D1.3. ALL WELDS SHALL BE TOUCHED-UP USING A ZINC-ENRICHED PAINT. WELDING ELECTRODES SHALL BE E60XX UNLESS OTERWISE NOTED. DO NOT BURN THROUGH MEMBERS. MEMBERS WITH DAMAGE RELATED TO WELDING SHALL BE REPLACED AND NOT RE-USED, UNLESS OTHERWISE APPROVED IN WRITING BY ENGINEER OF RECORD.

I. ALL STUDS SHEATHED TO BOTH SIDES SHALL HAVE LATERAL BRACING AT 4'-0" O.C. (VERT.) MAXIMUM. REFER TO 10/S9. ALL STUDS SHEATHED ONLY ON ONE SIDE OR LESS SHALL HAVE FULL DEPTH BLOCKING AT 4'-0" O.C.

J. SPLICING OF STUDS, TRACKS AND OTHERS STRUCTURAL MEMBERS SHALL NOT BE PERFORMED WITHOUT AN APPROVED SPLICE DESIGN AND DETAIL.

K. COPING, CUTTING OR NOTCHING OF FLANGES AND EDGE STIFFENERS IS NOT PERMITTED FOR STRUCTURAL MEMBERS WITHOUT AN APPROVED DESIGN AND DETAIL.

L. TRUSS DELIVERY, HANDLING, BRACING AND INSTALLATION [illegible] S214-07.

M. TRUSS SYSTEM

TOP CHORD DEAD LOAD:	15 PSF
BOTTOM CHORD DEAD LOAD:	5 PSF

N. LIGHT GAUGE FRAMING SYSTEMS INCLUDING TRUSSES [illegible] SHALL [illegible] ALL NECESSARY PARTS AND ACCESSORIES, TEMPORARY AND PERMANENT [illegible] REQUIRED [illegible] A COMPLETE SYSTEM, INCLUDING [illegible] BRIDGES TO SUPPORT STRUCTURE.

STRUCTURAL INSPECTION

A. THE CONTRACTOR OR OWNER SHALL EMPLOY A QUALIFIED INDEPENDENT INSPECTION AGENCY. THIS AGENCY (REFERRED TO AS INSPECTOR HEREIN) SHALL BE RESPONSIBLE FOR VERIFYING THAT PROJECT STRUCTURAL WORK IS ACCOMPLISHED IN CONFORMANCE WITH THE CONTACT DOCUMENTS.

B. DURATION AND FREQUENCY OF JOB [illegible] SHALL BE SUFFICIENT FOR THE INSPECTOR TO STATE AT THE COMPLETION OF THE PROJECT THAT THE STRUCTURAL WORK IS ACCOMPLISHED, AND ITS RELATED ELEMENTS HAVE BEEN ERECTED IN ACCORDANCE WITH THE CONTACT DOCUMENTS. SPECIFIC SYSTEMS TO BE INSPECTED INCLUDE:

FOUNDATION
CONCRETE (GENERAL, SLAB ON GRADE)
STRUCTURAL STEEL (INCLUDING JOISTS & JOIST GIRDERS)
METAL DECK
LIGHT GAUGE FRAMING (INCLUDING TRUSS SYSTEMS)

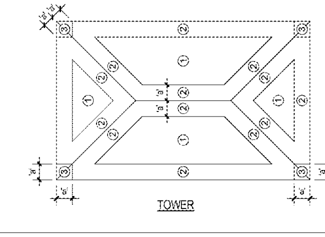
C. THE FOREGOING LIST IS NOT INTENDED TO BE ALL INCLUSIVE. THE INSPECTOR SHALL USE HIS PROFESSIONAL JUDGEMENT AND HIS KNOWLEDGE OF THE JOB SITE CONDITIONS AND THE OFFICIAL CONTRACT DOCUMENTS. THE INSPECTOR WILL NOT REPLACE THE QUALITY CONTROL PERSONNEL OF THE CONTRACTOR.

D. THE INSPECTOR DOES NOT RELIEVE THE CONTRACTORS CONTRACTUAL OR STATUTORY OBLIGATIONS. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE OFFICAL CONTRACT DOCUMENTS. THE INSPECTOR WILL NOT REPLACE THE QUALITY CONTROL PERSONNEL OF THE CONTRACTOR.

E. ALL INSPECTION REPORTS SHALL BE FORWARD BY THE INSPECTOR TO THE ENGINEER AND ARCHITECT OF RECORD.

LIST OF STRUCTURAL ABBREVIATIONS

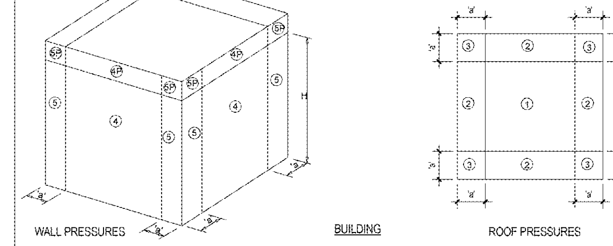
AB	ANCHOR BOLTS	MATL	MATERIAL
ADDL	ADDITIONAL	MAX	MAXIMUM
ALT	ALTERNATE	MB	MASONRY BEAM
ANCH	ANCHOR	MC	MOMENT CONNECTION
APPROX	APPROXIMATE	MCU	MASONRY COLUMN UNIT
ARCH	ARCHITECT/ARCHITECTURAL	MCJ	MASONRY CONTROL JOINT
BB	BOND BEAM	MECH	MECHANICAL
BLDG	BUILDING	MEZZ	MEZZANINE
BM	BEAM	MFR	MANUFACTURE/
BO	BOTTOM OF		MANUFACTURERS
BOT/BTM	BOTTOM	MIN	MINIMUM
BP	BASE PLATE/BEARING PLATE	MISC	MISCELLANEOUS
BRG	BEARING	MTL	METAL
BTW	BETWEEN	NIC	NOT IN CONTRACT
CANT	CANTILEVER	NOM	NOMINAL
CB	CONCRETE BEAM	NTS	NOT TO SCALE
CC	CONCRETE COLUMN	OC	ON CENTER
CIP	CAST IN PLACE	OPNG	OPENING
CJ	CONSTRUCTION JOINT	OPP	OPPOSITE
CL	CENTERLINE	PB	PARAPET BEAM (MASONRY)
CLR	CLEAR/CLEARANCE	PCC	PRECAST CONCRETE
COL	COLUMN	PL	PLATE
CONC	CONCRETE	PLF	POUNDS PER LINEAR FOOT
CONNX	CONNECTION	PLY	PLYWOOD
CONSTR	CONSTRUCTION	PRE-ENG	PRE-ENGINEERED
CONT	CONTINUOUS	PEMB	PRE-ENGINEERED METAL
CORR	CORRUGATED		BUILDING
CMU	CONCRETE MASONRY UNIT	PRE-FAB	PRE-FABRICATED
DET	DETAIL	PRELIM	PRELIMINARY
DIA/DIAM	DIAMETER	PROJ	PROJECTION
DIM	DIMENSION	[illegible]	POUNDS PER SQUARE FOOT
DIST	DISTANCE	[illegible]	POUNDS PER SQUARE INCH
DN	DOWN	[illegible]	PRESSURE TREATED
DR	DRAIN	[illegible]	POST TENSION(ING) (ED)
DWG	DRAWING	PW	PANEL WIDTH
DWL	DOWEL	RAD	RADIUS
EA	[illegible]	RB	ROOF BEAM (MASONRY)
[illegible]	EACH END	RCP	REINFORCED CONCRETE PIPE
EF	EACH FACE	[illegible]	ROOF DRAIN
EJ	EXPANSION JOINT	[illegible]	REFERENCE
EL/ELEV	ELEVATION	REINF	REINFORCING
ELECT	ELECTRICAL	REQ'D	REQUIRED
EMB	EMBEDMENT	REV	REVISION
ENGR	ENGINEER	RTU	ROOF TOP UNIT
EQ	[illegible]	RW	RETAINING WALL
EQUIP	[illegible]	SECT	SECTION
EQUIV	EQUIVALENT	SCH/SCHED	SCHEDULE
ES	EACH SIDE	SIM	SIMILAR
EXIST	EXISTING	SP/SPC	SPACE(S)
EXP	EXPANSION	SPECS	SPECIFICATIONS
EXT	EXTERIOR	SQ	SQUARE
EW	EACH WAY	SS	STAINLESS STEEL
FAB	FABRICATE	STD	STANDARD
[illegible]	FLOOR DRAIN	ST/STL	STEEL
FDN	FOUNDATION	STRUC	STRUCTURAL
FF	FINISHED FLOOR	SYM	SYMMETRICAL
FIN	FINISH(ED)	T & B	TOP AND BOTTOM
FL/FLR	FLOOR	T & G	TONGUE AND GROOVE
FTG	FOOTING	TE	THICKENED EDGE
GA	GAGE/GAUGE	THK	THICK
GALV/GV	GALVANIZED	THRD	THREAD(ED)
GC	GENERAL CONTRACTOR	TB	TIE BEAM
HAS	HEADED ANCHOR STUD	TOC	TOP OF CONCRETE
HB	HIGH BEAM (MASONRY)	TOM	TOP OF MASONRY
HORIZ	HORIZONTAL	TOS	TOP OF STEEL
HSB	HIGH STRENGTH BOLTS	TYP	TYPICAL
HSS	HOLLOW STEEL SECTION	TS	TUBE STEEL
HT	HEIGHT	UNO	UNLESS NOTED OTHERWISE
INT	INTERIOR	UON	UNLESS OTHERWISE NOTED
JST	JOIST	VERT	VERTICAL
JT	JOINT	VIF	VERIFY IN FIELD
K	KIPS	W	WIDE FLANGED
KO	KNOCK OUT	WB	WALL BEAM (TILT)
KSF	KIPS PER SQUARE FOOT	WC	WALL COLUMN (TILT)
KSI	KIPS PER SQUARE INCH	W/	WITH
L	ANGLE	W/O	WITHOUT
LB	LOW BEAM (MASONRY)	WD	WOOD
LBS	POUNDS	WGT	WEIGHT
LD	DEVELOPMENT LENGTH	WH	WEEPHOLE
LDH	LONG DIMENSION HORIZONTAL	WP	WORKING POINT
LDV	LONG DIMENSION VERTICAL	WT	STEEL TEE SECTION
LGTH	LENGTH	WWF	WELDED WIRE FABRIC
LLBB	LONG LEG BACK TO BACK	@	AT
LLH	LONG LEG HORIZONTAL	&	AND
LLV	LONG LEG VERTICAL		



(160 MPH) ALLOWABLE (ASD) WIND PRESSURES (ASCE 7-10) FOR COMPONENTS AND CLADDING

EFFECTIVE AREA (SQFT)	ZONE 1 POS. (PSF)	ZONE 1 NEG. (PSF)	ZONE 2 POS. (PSF)	ZONE 2 NEG. (PSF)	ZONE 3 POS. (PSF)	ZONE 3 NEG. (PSF)	ZONE 4 POS. (PSF)	ZONE 4 NEG. (PSF)	ZONE 5 POS. (PSF)	ZONE 5 NEG. (PSF)
10	22.4	-35.6	22.4	-61.8	22.4	-91.4	38.8	-42.1	38.8	-52.0
20	20.4	-34.5	20.4	-56.9	20.4	-85.5	37.1	-40.4	37.1	-48.5
50	17.8	-33.2	17.8	-50.4	17.8	-77.6	34.8	-38.1	34.8	-43.8
A ≥ 100	15.8	-32.3	15.8	-45.4	15.8	-71.7	33.0	-36.3	33.0	-40.4

NOTES:
1. ROOF OVERHANG PRESSURES:
a. ZONE 2 OH: +16 PSF AND -100.6 PSF
b. ZONE 3 OH: +16 PSF AND -130.2 PSF
2. "A" DENOTES EDGE STRIP = 3' - 0"
3. POSITIVE & NEGATIVE SIGNS SIGNIFY PRESSURES ACTING TOWARDS AND AWAY FROM THE BUILDING SURFACES, RESPECTIVELY (FULL HEIGHT, U.N.O.).
4. WIND PRESSURES INDICATED ABOVE ARE ALLOWABLE STRESS DESIGN WIND PRESSURES. DIVIDE THE PRESSURES BY A FACTOR OF 0.6 TO OBTAIN STRENGTH DESIGN WIND PRESSURES.



(160 MPH) ALLOWABLE (ASD) WIND PRESSURES FOR COMPONENTS & CLADDING (ASCE 7-10)

EFFECTIVE AREA (SQ FT)	ZONE 1 PRESSURE	ZONE 1 SUCTION	ZONE 2 PRESSURE	ZONE 2 SUCTION	ZONE 3 PRESSURE	ZONE 3 SUCTION	ZONE 4 PRESSURE	ZONE 4 SUCTION	ZONE 4 PARAPET CASE A	ZONE 4 PARAPET CASE B	ZONE 5 PRESSURE	ZONE 5 SUCTION	ZONE 5 PARAPET CASE A	ZONE 5 PARAPET CASE B
10	14.1 PSF	-34.7 PSF	31.8 PSF	-58.3 PSF	31.8 PSF	-58.3 PSF	31.8 PSF	-34.5 PSF	95.4 PSF	70.1 PSF	31.8 PSF	-42.4 PSF	95.4 PSF	78.6 PSF
20	13.3 PSF	-33.9 PSF	30.4 PSF	-52.1 PSF	30.4 PSF	-52.1 PSF	30.4 PSF	-33.0 PSF			30.4 PSF	-39.6 PSF		
50	12.1 PSF	-32.7 PSF	28.5 PSF	-43.9 PSF	28.5 PSF	-43.9 PSF	28.5 PSF	-31.2 PSF			28.5 PSF	-35.9 PSF		
A ≥ 100	11.2 PSF	-31.8 PSF	27.2 PSF	-37.7 PSF	27.2 PSF	-37.7 PSF	27.2 PSF	-29.8 PSF			27.2 PSF	-33.0 PSF		

NOTES:
1. PARAPET WIND PRESSURES:
CASE A = PRESSURES TOWARD THE BUILDING.
CASE B = PRESSURES AWAY FROM THE BUILDING.
2. 'P' DENOTES PARAPET PRESSURE VALUES.
3. 'a' DENOTES EDGE STRIP = 6'-0"
4. POSITIVE & NEGATIVE SIGNS SIGNIFY PRESSURES ACTING TOWARDS AND AWAY FROM THE BUILDING SURFACES, RESPECTIVELY (FULL HEIGHT, UNLESS NOTED).
5. WIND PRESSURES INDICATED ABOVE ARE ALLOWABLE STRESS DESIGN WIND PRESSURES. DIVIDE THE PRESSURES BY A FACTOR OF 0.6 TO OBTAIN STRENGTH DESIGN WIND PRESSURES

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SHEET TITLE
DESIGN CRITERIA AND NOTES

Revision Schedule

No.	Description	Date
	PERMIT SET	06/24/2016
B	REV 2017.01	07/26/2017
	PRE-BID REVIEW SET	07/26/2017
	BID SET	08/30/2017

PROJECT NO. 2160209
DATE 08-30-2017
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