

DRAWING NOTES

- GENERAL REQUIREMENTS
1. THE STRUCTURE DESCRIBED BY THESE DOCUMENTS IS INTENDED TO WORK AS A COMPLETED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION INCLUDING TEMPORARY SHORING AND BRACING. THE CONTRACTOR IS ALSO RESPONSIBLE FOR COORDINATION OF HIS OR HER WORK WITH ALL OTHER TRADES, AND FOR PERFORMING ALL WORK IN A SAFE AND SATISFACTORY MANNER.
2. ENGINEER/ARCHITECT'S APPROVAL MUST BE OBTAINED IN WRITING FOR ALL DEVIATIONS AND SUBSTITUTIONS. THE ENGINEER/ARCHITECT IS NOT RESPONSIBLE FOR THE FAILURE OF THE CONTRACTOR TO BUILD THE STRUCTURE ACCORDING TO THE DOCUMENTS.
3. THE CONTRACTOR SHALL VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO DETAILING, FABRICATION AND CONSTRUCTION; AND SHALL NOTIFY THE ENGINEER/ARCHITECT OF ANY DISCREPANCIES.
4. SHOP DRAWINGS: SUBMIT NEWLY PREPARED INFORMATION DRAWN TO SCALE. INDICATE DEVIATIONS FROM CONTRACT DOCUMENTS. DO NOT REPRODUCE CONTRACT DOCUMENTS OR COPY STANDARD INFORMATION. DOCUMENTS REPRODUCED FROM FULLER GROUP, INC. DOCUMENTS WITHOUT WRITTEN PERMISSION, WILL BE REJECTED. COMPLIANCE WITH SPECIFIED REQUIREMENTS REMAINS CONTRACTOR'S RESPONSIBILITY.
5. FOR ADDITIONAL INFORMATION, SEE THE PROJECT SPECIFICATIONS. IN THE CASE OF A DISCREPANCY, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN UNLESS APPROVED OTHERWISE IN WRITING BY THE ENGINEER.
6. OWNER SHALL EMPLOY AND PAY A QUALIFIED INDEPENDENT TESTING AGENCY TO PERFORM TESTS AND INSPECTIONS SPECIFIED IN OTHER SECTIONS, AND THOSE REQUIRED BY AUTHORITIES HAVING JURISDICTION, INCLUDING ALL SPECIAL INSPECTIONS. CONTRACTOR IS RESPONSIBLE FOR SCHEDULING INSPECTIONS AND TESTS. RETESTING: OWNER SHALL PAY FOR RETESTING WHERE RESULTS OF INSPECTIONS AND TESTS PROVE UNSATISFACTORY AND INDICATE NONCOMPLIANCE WITH REQUIREMENTS. THE OWNER RESERVES THE RIGHT TO DEDUCT COSTS OF RETESTING FROM CONSTRUCTION CONTRACT COSTS.
7. SECTIONS SHOWN ON STRUCTURAL DRAWINGS PROVIDE TYPICAL DETAILING INFORMATION THAT SHALL BE APPLIED TO ALL SIMILAR AND LIKE CONDITIONS U.N.O. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH PROJECT REQUIREMENTS.

- EARTHWORK
1. AN INDEPENDENT TESTING AGENCY SHALL BE RETAINED BY THE OWNER TO PERFORM TESTING OF EARTHWORK. ALL FOOTING AND SLAB SUB-GRADES SHALL BE INSPECTED, TESTED IF REQUIRED, BY THE TESTING AGENCY. ALL FILL PLACEMENT AND COMPACTION SHALL BE MONITORED BY THE TESTING AGENCY. ALL BACKFILL MATERIALS SHALL BE APPROVED BY TESTING AGENCY PRIOR TO PLACEMENT
2. FOUNDATIONS HAVE BEEN DESIGNED FOR 1500 P.S.F. ALLOWABLE SOIL BEARING PRESSURE. CAPACITY SHALL BE APPROVED BY THE TESTING AGENCY PRIOR TO CONCRETE PLACEMENT.
3. DRAINAGE FILL UNDER SLABS ON-GRADE SHALL BE NO. 57 STONE, OR APPROVED EQUAL, COMPACTED TO REQUIRED THICKNESS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ALL SERVICE AND UTILITY LINES ON THE SITE.
4. REFER TO PROJECT GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION. IN CASE OF DISCREPANCY, THE GEOTECHNICAL REPORT SHALL GOVERN UNLESS APPROVED OTHERWISE IN WRITING BY THE ENGINEER.

- CAST-IN-PLACE CONCRETE
1. SUBMIT CONCRETE MIX DESIGNS AND LABORATORY TEST REPORTS.
2. COMPLY WITH ASTM C 94; ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS"; ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"; AND CRSI'S "MANUAL OF STANDARD PRACTICE."
3. DEFORMED REINFORCING BARS: ASTM A 615, GRADE 60.
4. WELDED STEEL WIRE FABRIC: ASTM A 1064, FLAT SHEETS, NOT ROLLS.
5. PORTLAND CEMENT: ASTM C 150, TYPE I.
6. FLY ASH: ASTM C 618, TYPE F (LIMITED TO 15 PERCENT OF TOTAL CEMENT CONTENT).
7. PROPORTION NORMAL-WEIGHT CONCRETE MIXES TO PROVIDE THE FOLLOWING PROPERTIES:
A. COMPRESSIVE STRENGTH: 3000 PSI AT 28 DAYS FOR BUILDING FOUNDATIONS.
4000 PSI AT 28 DAYS FOR BUILDING SLABS.
B. WATER-CEMENT RATIO: 0.55 MAXIMUM AT POINT OF PLACEMENT FOR SLABS ON GRADE.
C. AIR CONTENT: 5.5 TO 7.0 PERCENT FOR CONCRETE EXPOSED TO FREEZING AND THAWING, 2 TO 4 PERCENT ELSEWHERE.
8. DO NOT ADD WATER TO CONCRETE DURING DELIVERY, AT PROJECT SITE, OR DURING PLACEMENT, UNLESS APPROVED BY ENGINEER.
9. PROTECT CONCRETE FROM PHYSICAL DAMAGE OR REDUCED STRENGTH DUE TO WEATHER EXTREMES DURING MIXING, PLACING, AND CURING.
10. SLAB FINISHES: TROWELED FINISH FOR FLOOR SURFACES TO RECEIVE FLOOR COVERINGS, PAINT, OR OTHER THIN FILM-FINISH COATINGS. SPECIFIED OVERALL VALUES OF FLATNESS, F(f) 35; AND LEVELNESS, F(l) 25; WITH MINIMUM LOCAL VALUES OF FLATNESS, F(f) 24; AND LEVELNESS, F(l) 17. NONSLIP BROOM FINISH TO EXTERIOR CONCRETE PLATFORMS, STEPS, AND RAMPS.
11. FORM 1/8" WIDE CONTRACTION JOINTS WITH POWER SAWS WHEN CUTTING ACTION WILL NOT TEAR, ABRASE OR OTHERWISE DAMAGE SURFACE AND BEFORE CONCRETE DEVELOPS RANDOM CONTRACTION JOINTS. SEE DETAILS FOR ADDITIONAL INFO.
12. BEGIN CURING UNFORMED CONCRETE AFTER FINISHING. KEEP CONCRETE CONTINUOUSLY MOIST FOR AT LEAST 7 DAYS OR APPLY MEMBRANE-FORMING CURING COMPOUND TO CONCRETE. CONTRACTOR SHALL VERIFY COMPOUND IS COMPATIBLE WITH FLOOR COVERING/COATINGS.
13. OWNER SHALL ENGAGE A TESTING AGENCY TO PERFORM TESTS AND TO SUBMIT TEST REPORTS.
14. PROTECT CONCRETE FROM DAMAGE. REPAIR SURFACE DEFECTS IN CONCRETE.
15. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHEN SPECIFIED ON DRAWINGS, UNLESS APPROVED OTHERWISE IN WRITING BY THE ENGINEER OF RECORD. HOLES SHALL BE PREPARED AND ANCHORS SHALL BE INSTALLED FOLLOWING THE MANUFACTURER'S INSTALLATION INSTRUCTIONS. UNLESS SPECIFIED OTHER-WISE, ANCHORS SHALL BE EMBEDDED A MINIMUM OF 8 TIMES THE DIAMETER. U.N.O., ACRYLIC BASED ADHESIVES SHALL BE USED WHEN BASE MATERIAL TEMPERATURES DROP BELOW 40°F AND TRUE EPOXY BASED ADHESIVES ARE PREFERRED FOR USE IN OVERSIZED HOLES. ADHESIVE ANCHORS: ACCEPTABLE PRODUCTS AND MANUFACTURER ARE:
A. EPCON S7 ADHESIVE ANCHORING SYSTEM
B. HILTI HIT-HY 200 ADHESIVE ANCHORING SYSTEM
C. SIMPSON STRONG-TIE SET-XP ADHESIVE ANCHORING SYSTEM

- UNIT MASONRY ASSEMBLIES
1. AN INDEPENDENT TESTING AGENCY SHALL BE RETAINED BY THE OWNER TO PERFORM FIELD INSPECTION AND TESTING OF MASONRY CONSTRUCTION. TESTING AGENCY SHALL INSPECT PLACEMENT OF ALL REINFORCEMENT AS SHOWN OR DESCRIBED IN THE CONTRACT DOCUMENTS.
2. ALL CONCRETE MASONRY WORK SHALL CONFORM TO ACI 530-11, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"; AND ACI 530.1-11, "SPECIFICATION FOR MASONRY STRUCTURES".
3. CONCRETE MASONRY UNITS: ASTM C 90; LIGHTWEIGHT. PROVIDE SPECIAL SHAPES FOR UNTELS, CORNERS, JAMBS, SASH, CONTROL JOINTS, AND OTHER SPECIAL CONDITIONS.
4. MORTAR: ASTM C 270, PROPORTION SPECIFICATION, FOR JOB-MIXED MORTAR; AND ASTM C 1142 FOR READY-MIXED MORTAR. ALL MORTAR FOR CMU WORK SHALL BE TYPE S. DO NOT USE CALCIUM CHLORIDE IN MORTAR.
5. GROUT: COMPLY WITH ASTM C476, PROPORTION SPECIFICATION. PLACE VERTICAL REINFORCING IN MASONRY CELLS AND SECURE IN PLACE PRIOR TO PLACEMENT OF GROUT. GROUT POUR HEIGHT SHALL NOT EXCEED 60 INCHES. ALL CELLS CONTAINING REINFORCING BARS SHALL BE FILLED WITH GROUT. WEBS OF HOLLOW UNITS CONTAINING VERTICAL REINFORCING SHALL BE FULLY MORTARED TO CONFINE GROUT DURING PLACEMENT.
6. VERTICAL REINFORCEMENT: SEE SHEET S1.1 FOR VERTICAL REINFORCEMENT IN VERTICAL WALLS SECURE REINFORCEMENT WITH VERTICAL BAR POSITIONERS, "WIRE-BOND CORELOCK" SINGLE AND/OR DOUBLE BARR POSITIONERS, OR EQUAL. LAP BARS AS FOLLOWS: #4 - 25", #5 - 32", #6 - 38".
7. HORIZONTAL REINFORCEMENT: PROVIDE 9 GAGE, LADDER TYPE JOINT REINFORCEMENT FORMED FROM GALVANIZED CARBON-STEEL WIRE, ASTM A 153, CLASS B-2, FOR BOTH INTERIOR AND EXTERIOR WALLS. SPACE REINFORCEMENT AT 8" O.C. BELOW GRADE AND 16" O.C. ABOVE GRADE (LAP 6" MINIMUM). CUT OR INTERRUPT REINFORCING AT CONTROL JOINTS. PROVIDE CONT. BOND BEAMS @ 4'-0" O.C. VERTICALLY.
8. ADDITIONAL REINFORCEMENT: PROVIDE A MINIMUM OF (1) #5 AT CORNERS, WITHIN 16" OF EACH SIDE OF OPENINGS, WITHIN 8" OF EACH SIDE OF CONTROL JOINTS AND WITHIN 8" OF THE ENDS OF WALLS. BOND BEAMS SHALL HAVE (2) #4'S CONTINUOUS AND REINFORCEMENT SHALL BE TURNED AND LAPPED AT ALL CORNERS AND INTERSECTIONS.
9. PLACE BLOCK IN RUNNING BOND WITH 3/8" JOINTS. TOOL EXPOSED JOINTS CONCAVE.
10. PROVIDE VERTICAL CONTROL JOINTS PER NOMA RECOMMENDATIONS: 1.5 TO 1 PANEL SIZE RATIO, OR 24'-8" MAXIMUM. SEE C/S3.0.
11. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHEN SPECIFIED ON DRAWINGS, UNLESS APPROVED OTHERWISE IN WRITING BY THE ENGINEER OF RECORD. HOLES SHALL BE PREPARED AND ANCHORS SHALL BE INSTALLED FOLLOWING THE MANUFACTURER'S INSTALLATION INSTRUCTIONS. ACCEPTABLE PRODUCTS AND MANUFACTURER ARE:
A. EPCON A7 ADHESIVE ANCHORING SYSTEM
B. HILTI HIT-HY 70 ADHESIVE ANCHORING SYSTEM
C. SIMPSON STRONG-TIE AT ADHESIVE ANCHORING SYSTEM

- STRUCTURAL STEEL
1. SUBMITTALS: SHOP DRAWINGS.
2. COMPLY WITH AISI 360-10 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325 OR A 490 BOLTS," AND AWS D1.1 "STRUCTURAL WELDING CODE-STEEL."
3. STRUCTURAL-STEEL WIDE FLANGE SHAPES: ASTM A992, GRADE 50. OTHER SHAPES, PLATES, AND BARS: ASTM A572, GRADE 50, CARBON STEEL.
4. COLD-FORMED STRUCTURAL-STEEL TUBING: ASTM A 500, GRADE B.
5. STEEL PIPE: ASTM A53, TYPE E, GRADE 80.
6. ANCHOR RODS, BOLTS, NUTS: ASTM F1554, GRADE 55, UNHEADED RODS.
7. BOLTS, NUTS, AND WASHERS: ASTM A 325, TYPE 1, HIGH-STRENGTH HEAVY HEX STEEL. STRUCTURAL BOLTS, HEAVY HEX CARBON-STEEL NUTS, AND HARDENED CARBON-STEEL WASHERS, UNCOATED.
8. WELDS: E70XX PER AWS.
9. PRIMER: LEAD- AND CHROMATE-FREE, NONSAPHALTIC, RESIN-INHIBITING PRIMER.
10. GROUT: ASTM C 1107, NONMETALLIC, SHRINKAGE RESISTANT, REMOVED.
11. FABRICATE STRUCTURAL STEEL ACCORDING TO ALSO SPECIFYING AN TOLERANCE LIMITS OF RCSC'S "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" FOR STRUCTURAL STEEL.
12. SHOP PRIMING: PREPARE SURFACES ACCORDING TO SSPC-SP 2 OR SSPC-SP 3. SHOP PRIME STEEL TO A DRY FILM THICKNESS OF AT LEAST 2.0 MILS. DO NOT PRIME SURFACES TO BE EMBEDDED IN CONCRETE OR MORTAR OR TO BE FIELD WELDED.
13. ERECT STRUCTURAL STEEL ACCORDING TO AISC'S "SPECIFICATIONS AND TOLERANCES FOR ERECTION" AND AISC'S "CODE OF STANDARD PRACTICE FOR STRUCTURAL STEEL BUILDINGS AND BRIDGES."
14. SET BASE AND BEARING PLATE WITH WEDGES, SHIMS, OR ANCHOR BOLTS. TIGHTEN ANCHOR BOLTS, CUT OFF WEDGES OR SHIMS FLUSH WITH FACE OF PLATE. CHECK GROUT SOLIDLY BETWEEN BEARING SURFACES AND PLATES.
15. BOLTED CONNECTIONS: SALES TO HIGH-STRENGTH BOLTS ACCORDING TO RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325 OR A 490 BOLTS."
16. UNLESS SPECIFICALLY NOTED, ALL BOLTED CONNECTIONS SHALL BE NON-SLIP CRITICAL USING 3/4" DIAMETER A325-N BOLTS WITH THREE PLATES INCLUDED IN THE SHEAR PLANE. ALL CONNECTIONS SHALL BE CITED FROM TABLE 10-1, OR TABLE 10-2, AISC STEEL CONNECTION MANUAL, UNLESS NOTED. CONNECTIONS SHALL DEVELOP AT LEAST ONE-HALF OF THE MAXIMUM TOTAL UNIFORM LOAD CAPACITY TABULATED IN THE MANUAL FOR THE GIVEN SHAPE AND SPAN OF THE BEAM. AS A MINIMUM, ALL SHEAR CONNECTIONS SHALL DEVELOP AT LEAST THE NUMBER OF ROWS OF 3/4" DIAMETER A325-N BOLTS (AT 3" PITCH) AS CAN BE FIT IN A CONNECTION. ALL BOLTED CONNECTIONS SHALL BE NON-SLIP CRITICAL USING 3/4" DIAMETER A325-N BOLTS (AT 3" PITCH) AS CAN BE FIT IN A CONNECTION. ALL BOLTED CONNECTIONS SHALL BE NON-SLIP CRITICAL USING 3/4" DIAMETER A325-N BOLTS (AT 3" PITCH) AS CAN BE FIT IN A CONNECTION.
17. ALL DECK EDGES SHALL HAVE CONTINUOUS SUPPORT. PROVIDE L4x4x1/2 CONTINUOUS SUPPORT ANGLE UNLESS NOTED OTHERWISE. WELD ANGLE TO EACH SUPPORT BEAM OR JOIST WITH 1/2" x 2" FILLET WELDS, U.N.O. WHERE ANGLE ABUTS MASONRY WALL, ATTACH ANGLE TO WALL WITH 1/2" DIA. SLEEVE ANCHORS AT 24" O.C., U.N.O.

- STEEL JOISTS
1. SUBMITTALS: SHOP DRAWINGS.
2. COMPLY WITH RECOMMENDATIONS OF SJA'S "STANDARD SPECIFICATIONS LOAD TABLES AND WEIGHT TABLES FOR STEEL JOISTS AND JOIST GRIDDERS" AND AWS D1.1 "STRUCTURAL WELDING CODE-STEEL," AND WITH OSHA REGULATION 29 CFR, SUBPART R.
3. BOLTS, NUTS, AND WASHERS: ASTM A 325, TYPE 1.
4. PRIMER: MANUFACTURER'S STANDARD SHOP PRIMER.
5. MANUFACTURE JOISTS ACCORDING TO SJA'S SPECIFICATIONS WITH STEEL ANGLE TOP AND BOTTOM CHORD MEMBERS.
6. PROVIDE BRIDGING ANCHORS AND HORIZONTAL OR DIAGONAL BRIDGING.
7. SHOP PRIMING: PREPARE SURFACES ACCORDING TO SSPC-SP 2 OR SSPC-SP 3. SHOP PRIME STEEL JOISTS TO A DRY FILM THICKNESS AT LEAST 1 MIL.
8. INSTALL JOISTS AND ACCESSORIES PLUMB, SQUARE, AND TRUE TO LINE; SECURELY FASTEN TO SUPPORTING CONSTRUCTION ACCORDING TO SJA'S SPECIFICATIONS AND OSHA REGULATIONS.
9. PROVIDE JOIST REINFORCEMENT IN ALL LOCATIONS WHERE A CONCENTRATED LOAD GREATER THAN 50 POUNDS IS APPLIED TO A JOIST BETWEEN PANEL POINTS. DO NOT APPLY GREATER THAN 200 POUNDS TOTAL CONCENTRATED LOADS TO ANY SINGLE JOIST UNLESS NOTED OTHERWISE ON DRAWINGS.
10. JOIST FABRICATOR TO DESIGN ROOF JOISTS FOR A NET UPLIFT OF 16 PSF. IF ADDITIONAL BRIDGING IS REQUIRED, IT SHALL BE CLEARLY NOTED ON THE ERECTION DRAWINGS.

- STEEL DECK
1. SUBMITTALS: SHOP DRAWINGS.
2. COMPLY WITH STEEL DECK INSTITUTE (SDI) DESIGN MANUAL, LATEST EDITION, AND AMERICAN WELDING SOCIETY (AWS) D1.3 STRUCTURAL WELDING CODE/SHEET METAL.
3. CERTIFY THAT ALL WELDERS HAVE SATISFACTORILY PASSED AWS QUALIFICATION TESTS FOR THE WELDING PROCESSES INVOLVED AND THAT ALL CERTIFICATIONS ARE CURRENT.
4. THE DECK MANUFACTURER SHALL FURNISH RIDGE AND VALLEY PLATES, SUMP PANS, FINISH STRIPS, COVER PLATES, END CLOSURES AND REINFORCING CHANNELS AS REQUIRED TO PROVIDE A FINISHED SURFACE FOR APPLICATION OF ROOFING, AND POUR STOPS, GIRDER FILLERS AND DECK CLOSURES AS REQUIRED BY THE STEEL DECK INSTITUTE. MECHANICAL FASTENERS OR WELDS ARE ACCEPTABLE FOR ACCESSORY ATTACHMENTS.
5. STEEL ROOF DECK SHALL BE WIDE RIB (TYPE B), 1-1/2" IN DEPTH WITH A DESIGN THICKNESS OF 22 GAGE.
6. SHEET STEEL FOR PRIMER PAINTED ROOF DECK AND/OR ACCESSORIES SHALL CONFORM TO ASTM A611 WITH A MINIMUM YIELD STRENGTH OF 33 KSI.
7. ANCHOR DECK UNITS, WITH 5/8" DIAMETER PUDDLE WELDS, AT 6" O.C. TO INTERIOR STEEL SUPPORTING MEMBERS AND AT 6" O.C. AROUND PERIMETER EDGES U.N.O. FASTEN SIDE LAPS OF DECK UNITS WITH A MINIMUM OF (4) #10 SIDELAP SCREWS

BRICK UNTEL SCHEDULE
Table with 3 columns: CLEAR OPENING SIZE, STEEL ANGLE SIZE, MINIMUM BEARING LENGTH. Rows include sizes for openings up to 10'-0".

- NOTES:
1. HOT DIP GALVANIZE ALL UNTELS IN EXTERIOR WALLS.
2. PROVIDE (1) ANGLE FOR EACH 4" OF WALL WIDTH IN MULTI-WYTHE BRICK WALLS.
3. DO NOT PLACE VERTICAL CONTROL JOINTS ABOVE OR WITHIN 24" OF EITHER SIDE OF OPENINGS.
4. PROVIDE ROLLED UNTELS AT ARCHED BRICK LOCATIONS SHOWN ON ARCHITECTURAL ELEVATIONS.

8" CMU MASONRY UNTEL SCHEDULE
Table with 4 columns: CLEAR OPENING, DEPTH, MIN. BRG., REINFORCING. Rows include sizes for openings from 1'-4" to 6'-0".

- EXTEND REINFORCING TO END OF BEARING ON EACH SIDE. GROUT MASONRY SOLID BELOW BEARING TO FOUNDATION BELOW. THIS SCHEDULE DOES NOT APPLY TO OPENINGS OVER 4'-0" SUPPORTING ROOF LOADS. SPECIAL DESIGN REQUIRED FOR THIS CONDITION.

DESIGN LOAD CRITERIA

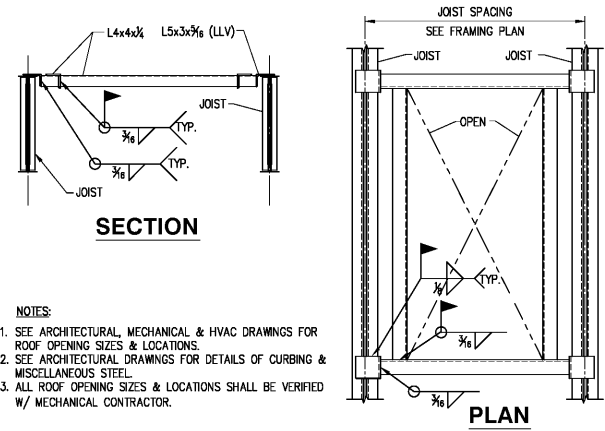
ROOF DESIGN LOADS
Table with 2 columns: Load Type, Value. Includes Dead Load, Live Load, Total Load, Net Uplift.

SNOW LOADS
Table with 2 columns: Load Type, Value. Includes Ground Snow Load, Exposure Factor, Thermal Factor, Importance Factor, Snow Storage, Flat Roof Snow Load, Minimum Load for Gable Roof, Pm.

WIND LOADS
Table with 2 columns: Wind Speed, Category. Includes Basic Wind Speed, Risk Category, Height & Exposure Adjustment, Wind Directionality Factor, Topographic Factor, Kzt.

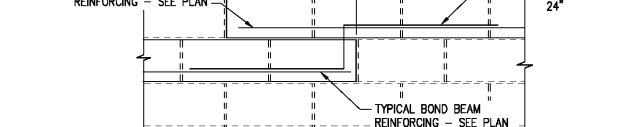
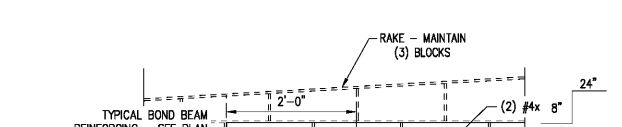
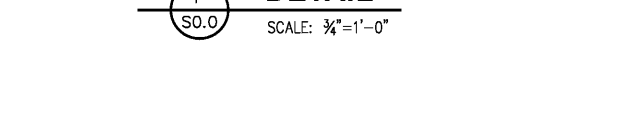
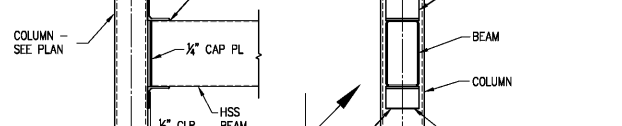
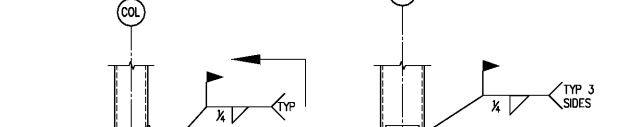
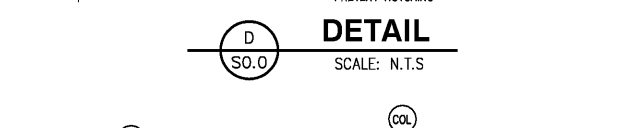
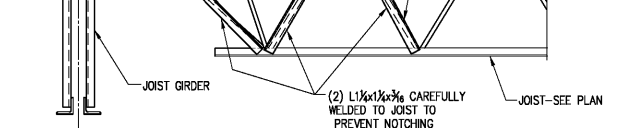
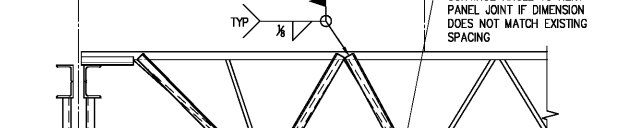
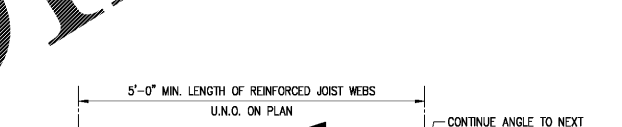
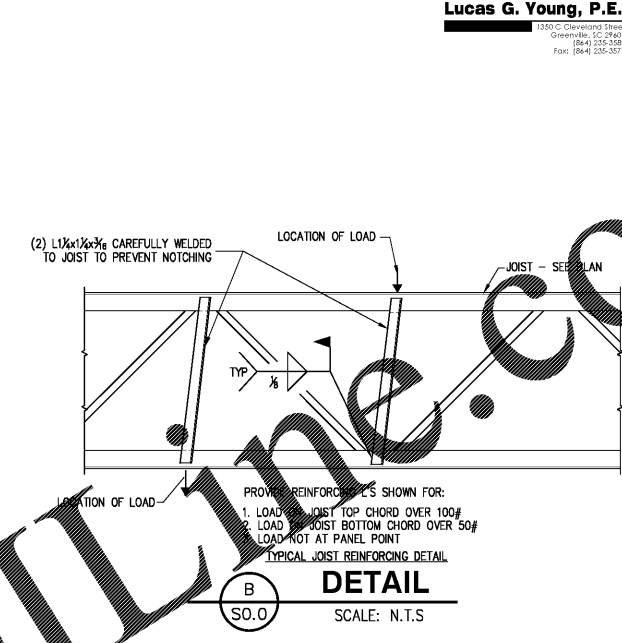
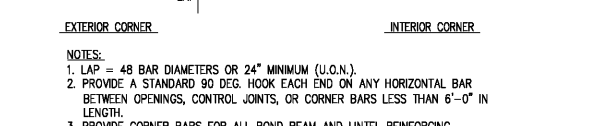
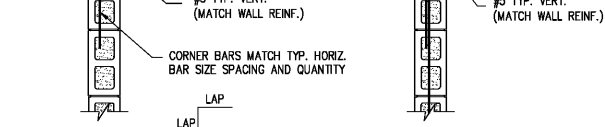
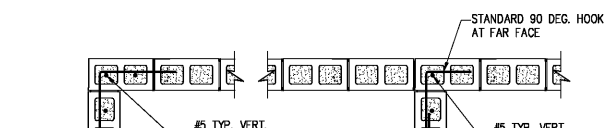
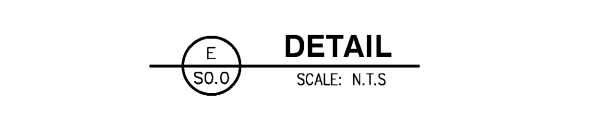
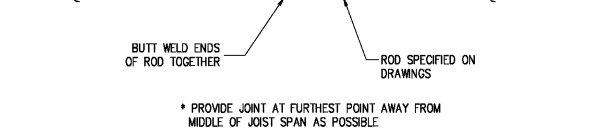
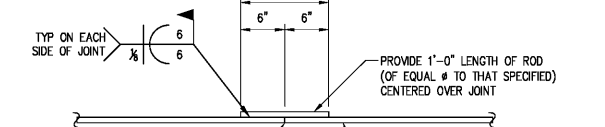
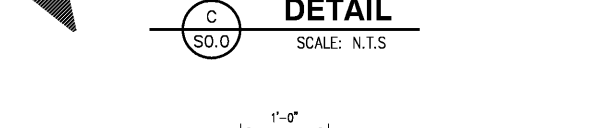
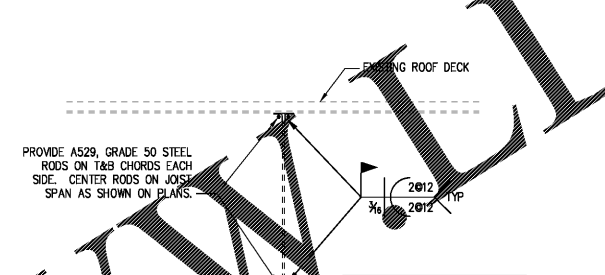
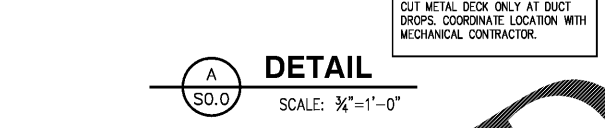
WIND PRESSURES
Table with 4 columns: Zone, Wind Speed, Pressure, Negative Pressure. Includes Zone 1, Zone 3, Zone 4, Zone 5.

SEISMIC LOADS
Table with 2 columns: Parameter, Value. Includes Importance Factor, Risk Category, Site Class, Soil Class, S1, Sds, Sd1, Design Category, Basic System, Resisting System, Response Coefficient, Response Modification Factor, Base Shear, Analysis Procedure.



TYP. RTU SUPPORT & ROOF OPN'G FRAMING

NOTE: PROVIDE SUPPORT FRAMING FOR ROOF OPENINGS 12'x12' AND LARGER



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Project Name: GERMANTOWN2
Drawn By: SJF

Revisions table with columns: No., DATE, DESCRIPTION.

Project Title:

RETAIL SPACE

Poplar Avenue & Exeter Road

Location: Germantown, TN

Sheet Contents: General Notes & Design Criteria

Date: November 04, 2016

Sheet Number: S0.0